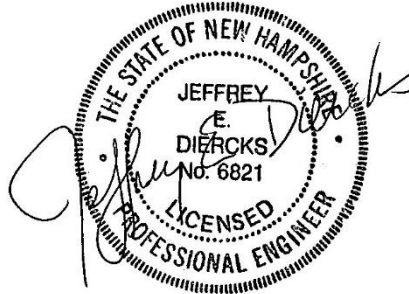


## **ADDENDUM NO. 2**

### **MODIFICATIONS TO THE WATER MAIN DRAWINGS AND SPECIFICATIONS AND MODIFICATIONS TO APPENDICES**

**January 29, 2018**

**Prepared by: CDM Smith Inc., Manchester, New Hampshire**



29-JAN-2018

#### **MODIFICATIONS TO THE WATER MAIN DRAWINGS**

1. DELETE Note 5 on Sheets W-1 through W-5, and INSERT INSTEAD this new Note 5:

“5. ALL WATER MAIN JOINTS TO BE RESTRAINED. ALL FITTINGS TO BE OF THE RESTRAINED MECHANICAL JOINT TYPE.”

2. On the north side of Main Street, west of Broadway, Sheet W-5 shows an unlabeled water main. To that water main, add the label “Abandoned”.
3. On the lower strip of Sheet W-5, INSERT a new boxed note which reads “REMOVE AND DISPOSE OF EXISTING 10” AND 12” CAST IRON MAIN WHERE NECESSARY FOR CONSTRUCTION OF NEW 12” DI MAIN.”
4. In the lower strip of Sheet W-5, DELETE the boxed note which reads, “CROSS OVER EXISTING DRAIN AND ENCASE WITH CONCRETE, SEE NOTE 10”. INSERT INSTEAD a boxed note which reads, “REDUCE COVER DEPTH TO 4.3 FT TO CROSS OVER EXISTING DRAIN, SEE NOTE 10”.
5. In the “Water Main – Sewer and Drain Crossings” table on Sheet W-6, DELETE the entry in the righthand column which reads “<5.0”, and INSERT INSTEAD “4.3”.
6. On Sheet W-6, DELETE the TYPICAL CULVERT CROSSING DETAIL and the detail titled STEEL SLEEVE FOR WATER MAIN AT CULVERT CROSSINGS. INSERT INSTEAD the two culvert crossing details which follow the Addendum text.

## **MODIFICATIONS TO THE WATER MAIN SPECIFICATIONS**

1. In Section W-01025, DELETE paragraph 1.05. INSERT INSTEAD the following new paragraph 1.05:

“1.05 Temporary Water Service (ITEMS W-5a THROUGH W-5d)

### **A. Measurement**

1. Temporary water service pipe of the sizes specified in the Bid Form (Items 5a and 5b) will be measured in place on a linear foot basis.
2. Temporary hydrants (Item 5c) will be measured as the number actually installed in the temporary water system and accepted by the Engineer.
3. Type B temporary service connections (Item 5d) will be measured as the number actually installed in the temporary water system and accepted by the Engineer.

### **B. Payment**

1. Payment for furnishing, installing and removing temporary service pipe of the sizes specified will be made for the respective quantities as above determined, at the applicable price per linear foot bid under Items 5a and 5b in the Bid Form. Such price and payment shall be full compensation for all labor, equipment, materials, and incidentals for submitting the plan for piping; furnishing and installing the temporary service pipe; connecting the temporary pipe to the existing water system; furnishing and installing Type A hose connections; furnishing and installing temporary valves; disinfecting all pipe and hose as specified; excavation and pavement restoration for street crossings; protection of driveways and of the temporary pipe; bagging out-of-service hydrants; maintaining the temporary piping system; temporary piping system removal; and surface restoration.
2. Payment for furnishing, installing and removing temporary hydrants will be made at the applicable unit price bid under Item 5c in the Bid Form. Such price and payment shall be full compensation for all labor, equipment, materials and incidentals for furnishing, installing, maintaining and removing temporary hydrants at locations approved by Engineer and Owner.
3. Payment for furnishing, installing and removing Type B service connections will be made at the applicable unit price bid under Item 5d in the Bid Form. Said price shall apply to 1-inch or 2-inch temporary service connections, connected to 1-inch or 2-inch existing services, at locations approved by Engineer and Owner. Such price and payment shall be full compensation for all labor, equipment, materials

and incidentals for excavation; tapping or otherwise connecting the existing service connection to the temporary piping system; temporary shutoff valve if needed; backfill; disinfection; maintaining the temporary service connection; removing the temporary service connection; repairing the permanent service connection as needed; and restoration of ground surface or pavement.”

2. In Section W-01025, paragraph 1.06, DELETE “24-inch steel sleeve” wherever it appears and INSERT INSTEAD “20-inch steel sleeve”.
3. At the end of Section 01170, INSERT the following new paragraph 1.11:

“1.11 STEEL SLEEVE FOR CULVERT CROSSINGS

- A. Steel casing pipe shall be of leakproof construction and conform to the requirements of ASTM A53. Steel casing pipe shall have a minimum yield strength of 35,000 psi, and shall be designed to withstand H-20 traffic loading. Steel casing shall be of nominal 20-inch diameter with a 0.50-inch wall thickness. The steel casing pipe shall be galvanized in accordance with AASHTO M 111. All hardware shall be A325 H.S. galvanized steel.
- B. Steel pipe shall be furnished in lengths of the Contractor’s choice. The casing shall have beveled ends with a single or double v-groove and shall be field joined by full-penetration butt welding all around. Alternatively, Permalok connectors may be used.
- C. Casing spacers shall be Model CCS as manufactured by Cascade Waterworks Mfg. Co. of Yorkville, IL, or approved equal. Casing spacer shall be a two-piece shell per carrier pipe and made from T304 stainless steel of a minimum 14-gauge thickness. Each shell section shall be lined with a 0.090” thick ribbed PVC extrusion with a retaining section that overlaps the edges of the shell and prevents slippage. PVC liner shall have a hardness of 85-90 durometer. Bearing surfaces (runners) shall be ultra-high molecular weight polyethylene to provide abrasion resistance and a low coefficient of friction (0.12). The runners shall be attached to support structures (risers) at appropriate positions to properly support the carrier pipe within the casing pipe and to ease installation. The runners shall be mechanically bolted to the spacer. Risers shall be MIG welded to the shell, where applicable. Risers shall be made of T304 stainless steel of a maximum 10-gauge with bolt heads welded to the inside of the risers for strength. Bottom risers 6” and over in height shall be reinforced. All reinforcing plates shall be 10 gauge T304 stainless steel and shall be MIG welded to mating parts.

- D. Casing spacers shall employ centered and restrained positioning within the casing pipe, and shall be installed in accordance with the manufacturer's instructions. Number and position of spacers shall be in accordance with the manufacturer's instructions.
- E. Casing spacer end seals shall be a pull-over type construction and made from 0.090" (3/32") thick Neoprene with T304 stainless steel bands for securing the ends of the end seal to the casing pipe and carrier pipe. Casing spacer end seals shall be Model CCES as manufactured by Cascade Waterworks Mfg. Co. of Yorkville, IL, or approved equal. Casing spacer end seals shall be installed in accordance with manufacturer's instructions."
4. In Section W-02616, DELETE paragraph 2.03C and INSERT INSTEAD the following new paragraph 2.03C:
- "C. Contractor shall have at hand at the beginning of the work Viton gaskets of all needed sizes to be incorporated into restrained joints. The number of Viton gaskets of each size to be kept at hand shall be enough to assure the continued progress of Contractor's work. OWNER or ENGINEER may order the use of Viton gaskets based on conditions encountered in the field. Whenever any Viton gaskets are utilized, Contractor shall restore the stock in such a manner as to assure the continued progress of the work."

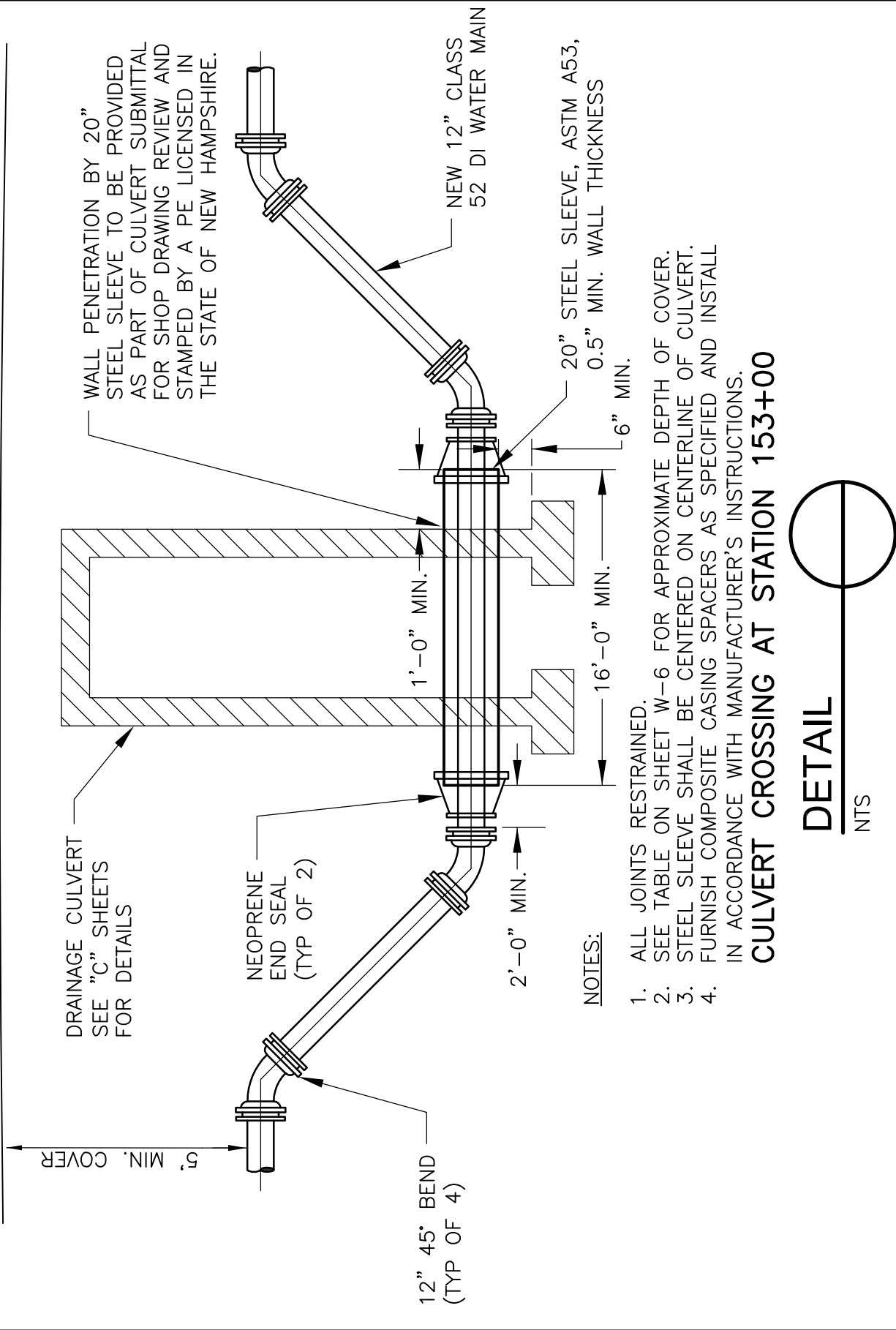
#### **MODIFICATIONS TO THE APPENDICES**

1. DELETE "Appendix A – Geotech (Subsurface) Information", and INSERT INSTEAD the attached "Appendix A – Geotechnical (Subsurface) Information".
2. INSERT the attached new "Appendix G – Environmental Testing Results and Monitoring Well Water Levels".
3. INSERT the attached new "Appendix H – Summary of Meeting with GLSD".

#### **ATTACHMENTS**

- Two details for culvert crossings
- Replacement "Appendix A – Geotechnical (Subsurface) Information"
- New "Appendix G – Environmental Testing Results and Monitoring Well Water Levels"
- New "Appendix H – Summary of Meeting with GLSD"

**END OF ADDENDUM**



CULVERT CROSSING AT STATION 153+00

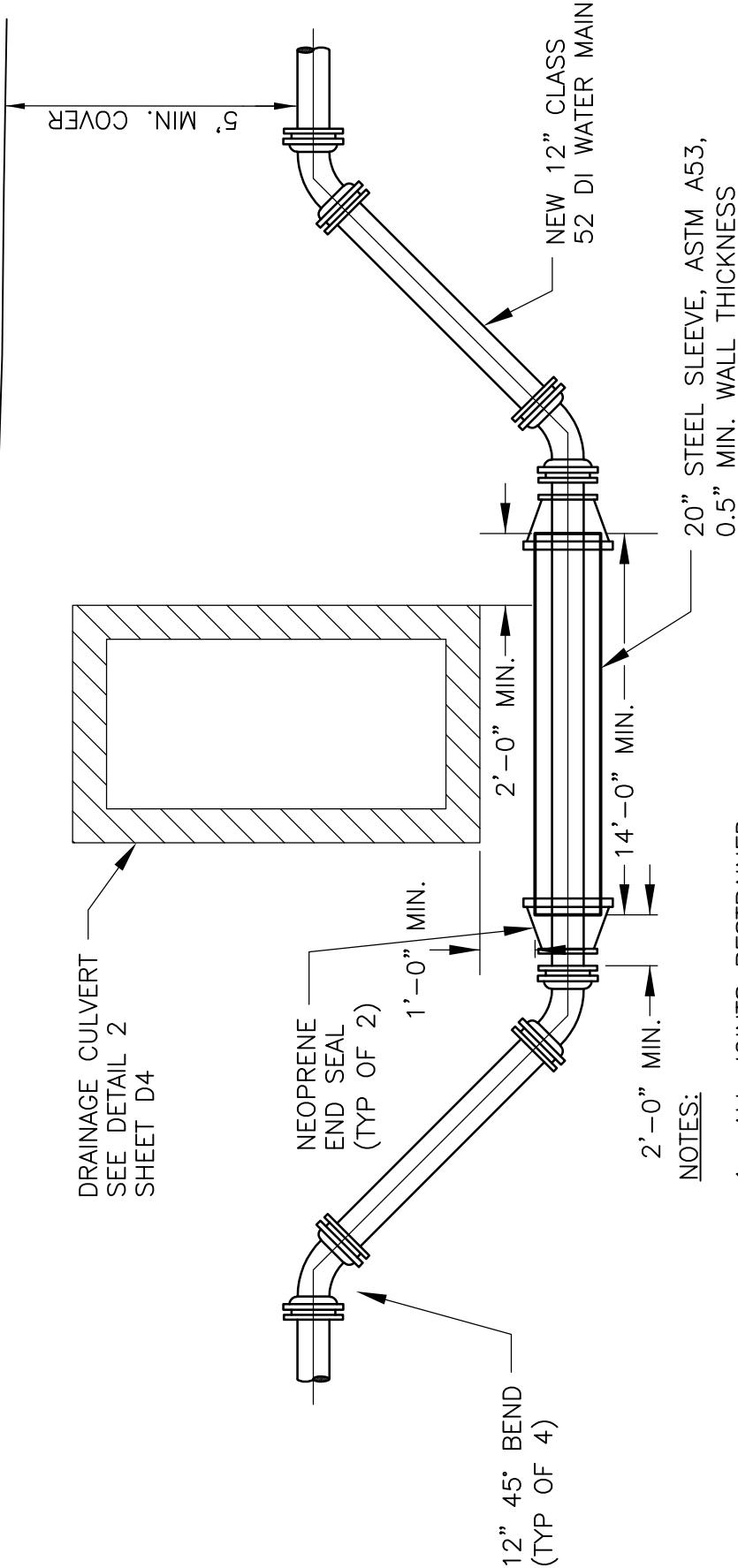
DETAIL

NTS

NOTES:

1. ALL JOINTS RESTRAINED.
2. SEE TABLE ON SHEET W-6 FOR APPROXIMATE DEPTH OF COVER.
3. STEEL SLEEVE SHALL BE CENTERED ON CENTERLINE OF CULVERT.
4. FURNISH COMPOSITE CASING SPACERS AS SPECIFIED AND INSTALL IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS.

<div> <div>CDM</div> <div>Smith</div> </div>	<div>DATE</div> <div>JANUARY 2018</div>
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- NOTES:
1. ALL JOINTS RESTRAINED.
  2. SEE TABLE ON SHEET W-6 FOR APPROXIMATE DEPTH OF COVER.
  3. STEEL SLEEVE SHALL BE CENTERED ON CENTERLINE OF CULVERT.
  4. FURNISH COMPOSITE CASING SPACERS AS SPECIFIED AND INSTALL IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS.

CULVERT CROSSING AT STATION 170+15

DETAIL

NTS

SHEET NO.		LOCATION	

## **APPENDIX A**

### **GEOTECH (SUBSURFACE) INFORMATION**



## Memorandum

*To: Jeffrey Diercks, P.E.*

*From: Jennifer Todd  
Tyler Dunn, P.E.*

*Reviewed By: Kathleen M. Murtagh, P.E.*

*Date: January 9, 2018*

*Subject: Summary of Geotechnical Field Exploration Program and Geotechnical  
Engineering Recommendations  
South Broadway Watermain and Sewer Replacement  
Salem, New Hampshire*

## Introduction

This memorandum summarizes CDM Smith's field exploration and laboratory testing program, and provides geotechnical engineering recommendations for the design and construction for the South Broadway Watermain and Sewer Replacement, located in the Town of Salem, New Hampshire.

Elevations noted herein are in feet (ft.) and are referenced to North American Vertical Datum of 1988 (NAVD88).

## Existing Conditions

The project area includes the paved road South Broadway (Rte. 28) from Main Street (Rte. 97) to south of Cluff Crossing Road, also including some of Main Street and North Broadway around their intersection with South Broadway in Salem, New Hampshire.

South Broadway is a four-lane state highway with the Salem Rail Trail (former rail line) to the west and commercial areas to the east and west along most of the length from Main Street to Cluff Crossing Road. The area between Angelas Drive and Brookwood Drive is mainly residential, containing a mobile home park. The project area covers approximately 7,900 ft of South Broadway, and approximately 1,650 ft of Main Street, from Pleasant Street to Millville Street.

South Broadway runs generally north-south from Main Street in the north to Cluff Crossing Road in the south. The roadway elevation of South Broadway ranges from approximately El. 136 to approximately El. 123. Main Street runs generally east-west from Pleasant Street to Millville Street.

Utilities are present above and below ground in the project area. Subsurface utilities include water mains, gas mains, telecommunications conduits, sanitary sewers, and storm drains. The water line



and gas line are connected via services to structures on the east side of the street. Catch basins are present adjacent to the edge of the road on both sides of the street.

## **Proposed Construction**

The proposed construction includes installing approximately 7000 feet of watermain pipe and about 7,700 feet of sanitary sewer pipe within the project area. The proposed gravity sanitary sewer designed by Underwood Engineers, Portsmouth, NH consists of PVC pipe ranging in size from 15- to 24-inch diameter. The smaller diameter pipe begins at the northern end near Main Street and incrementally increases in size toward the south. The proposed depth of the sewer pipe invert will range from about 10 to 16 feet below existing road grade. The proposed watermain consists of 12-inch-diameter ductile iron pipe. The watermain pipe will be installed with a minimum 5-foot cover below the existing road surface grade. Associated sewer manholes are anticipated to be prefabricated, but may be cast-in-place if site conditions warrant.

## **Purpose and Scope**

The purpose of this study was to investigate the subsurface conditions at the project site and to provide geotechnical engineering recommendations and construction considerations as required for the project.

Specifically, the scope of work included the following:

- Review available existing information;
- Conduct twenty-eight (28) test borings to investigate subsurface conditions and to obtain soil samples for geotechnical laboratory testing;
- Conduct one (1) additional test boring for another project, far to the south along South Broadway to investigate subsurface conditions and to obtain soil samples for geotechnical laboratory testing;
- Install five (5) monitoring wells to investigate the subsurface water level;
- Conduct geotechnical laboratory tests on select soil samples to assist with the classification of soils encountered and development of engineering soil properties;
- Develop geotechnical engineering recommendations for design and construction of the proposed sewer line; and
- Prepare this memorandum presenting CDM Smith's recommendations, including data collected as part of the investigation.

## **Subsurface Exploration Program**

Twenty-nine (29) test borings (B-1 through B-29) were performed as part of the subsurface exploration program.

The test borings were drilled by New England Boring Contractors of Derry, NH in two phases. Phase I was conducted from April 10 through April 13, 2017 using a track-mounted drill rig and Phase II was conducted from September 11 through October 4, 2017 using a truck-mounted drill rig. Test borings were advanced using drive and wash drilling techniques, except for test boring B-28 which was advanced using a 4-inch hollow stem auger (HSA). The test borings were advanced to depths that ranged from 7 ft. to 33 ft. below ground surface (bgs).

Soil sampling was conducted continuously from ground surface to approximately 10 ft. bgs at each boring location, and then at approximately 5-foot intervals thereafter. All sampling was conducted using a split spoon sampler in accordance with ASTM D1586 (2-inch diameter sampler driven 24 inches by blows from a 140-pound hammer falling freely for a 30-inch drop). A manual hammer was used during split spoon sampling. The number of blows required to drive the sampler each 6-inch increment was recorded and the Standard Penetration Resistance (N-value) was calculated as the sum of the blows over the second and third 6-inch intervals. Split spoon refusal was defined as less than 6 inches of penetration for 100 blows from a 140-pound hammer.

Rock coring was conducted at two boring locations (B-13A and B-20). Rock coring was conducted using an NX-size diamond bit, double-core barrel, system in general accordance with ASTM D-2113. NX-size rock core samples have a nominal diameter of 2-inches. Samples were classified in the field, the core recovery and RQD was determined as well. RQD is defined as the sum of all rock core pieces measuring over 4 inches in length divided by the total length of the core run and is expressed as a percentage.

When possible, groundwater levels at the test boring locations were estimated from the condition of the samples obtained and by the observed water levels within the test borings during and immediately following drilling activities. All test borings were backfilled with drill cuttings and the surface repaired with asphalt patch.

The test borings were located in the field by CDM Smith by tape measure and line-of-sight. The as-drilled locations of the test borings are shown on **Figure 1**. Logs of the test borings, prepared by CDM Smith, are included in **Attachment A**.

## **Monitoring Wells**

Five (5) monitoring wells were installed at the site at test boring locations B-2, B-3, B-4, B-11, and B-15. The installed monitoring wells are open-standpipe wells. The standpipe monitoring wells were constructed using 2-inch diameter, Schedule 40 PVC pipe with machine-slotted screens. The screen interval was 15 feet in length in all wells. Screen slot size was 10-slot (0.010 inch). Prior to placement of the well screens, the boreholes were flushed with clean water. A threaded cap was attached to the bottom of the screen, which was then lowered down the borehole attached to lengths of solid 2-inch PVC riser pipe. Clean quartz sand was then poured slowly around the PVC to extend the filter pack approximately 0.5 to 1 foot above the top of the screen. A 1-foot layer of bentonite chips was used to seal off the filter pack. At each test boring locations, the PVC pipe was

cut off approximately 3 inches below the ground surface, the boreholes were grouted to the surface and were covered with a protective flush road box.

The monitoring well installation logs and respective monitoring well reports, prepared by CDM Smith, are included in **Attachment B**.

### **Subsurface Conditions**

In general, subsurface conditions encountered during the recent test boring program consisted of Asphalt, Subbase, Concrete, Fill, Clayey Silt, Gravelly Sand, Silty Sand, Sand, Weathered Rock, and Bedrock. A summary of the subsurface conditions encountered is included in **Table 1**.

#### *Asphalt*

Asphalt was encountered at all but three (3) locations (B-2, B-3, and B-28). The asphalt was between 3 and 10 inches thick at the test boring locations.

#### *Subbase*

A discrete subbase was encountered four (4) locations (B-5, B-10, B-22, and B-26). The subbase was between 2.5 and 6 inches thick at the test boring locations.

#### *Concrete*

Concrete was encountered at thirteen (13) locations (B-6 through B-8, B-13, B-14, B-16, B-19 through B-21, and B-23 through B-26). The concrete was between 4 and 7 inches thick at the test boring locations.

#### *Fill*

Fill was encountered at two (2) test boring locations (B-5 and B-14). Where encountered, this layer was approximately 10 inches to 5 ft. thick, and typically consists of moist, loose to very dense, brown, fine SAND, little silt, little fine to coarse gravel. SPT N-values in the fill stratum ranged from 9 blows per foot (bl/ft) to 54 bl/ft, with an average of 31 bl/ft at the test boring locations.

#### *Clayey Silt*

A Clayey Silt stratum was encountered at three (3) test boring locations (B-3, B-9, and B-16A). The Clayey Silt stratum was fully penetrated at boring location B-5; at this location the layer was approximately 11 ft. thick. At the other boring locations, where this stratum was encountered, but not fully penetrated, the penetration depth ranged from approximately 8 ft. to 17 ft. The clayey silt stratum typically consists of moist, stiff to hard, SILT, with varying amounts of fine sand. SPT N-values in the clayey silt stratum ranged from 10 bl/ft to 34 bl/ft, with an average of 18 bl/ft at the test boring locations.

#### *Gravelly Sand*

A Gravelly Sand stratum was encountered at ten (10) test boring locations (B-3 (MW), B-4 (MW), B-13A, B-14, B-16, B-19 through B-20A, B-24, and B-29). The Gravelly Sand stratum was fully penetrated at boring locations B-3 (MW), B-4 (MW), B-13A, B-16, and B-19; at these locations the

layer was approximately 1.5 ft. to 12 ft. thick. At the other boring locations, where this stratum was encountered, but not fully penetrated, the penetration depth ranged from approximately 2 ft. to 13 ft. The Gravelly Sand stratum typically consists of moist, loose to very dense, fine to coarse SAND, some fine to coarse gravel, trace silt. SPT N-values in the gravelly sand stratum ranged from 6 bl/ft to over 100 bl/ft, with an average of 63 bl/ft at the test boring locations.

#### *Silty Sand*

A Silty Sand stratum was encountered at seventeen (17) test boring locations (B-1 through B-5, B-8, B-9, B-16, B-17 through B-20, B-21, and B-23 through B-26). At one location, B-2 (MW), organic material (wood, roots) was encountered in a surface Silty Sand stratum. The Silty Sand stratum was fully penetrated at boring locations B-1, B-9, B-19, B-20, B-23, and B-24; at these locations the layer was approximately 6 ft. to 22.5 ft. thick. At the other boring locations, where this stratum was encountered, but not fully penetrated, the penetration depth ranged from approximately 3 ft to 25 ft. The Silty Sand stratum typically consists of moist, very loose to very dense, fine SAND and SILT, trace fine gravel. SPT N-values in the silty sand stratum ranged from 2 bl/ft to over 100 bl/ft, with an average of 22 bl/ft at the test boring locations.

#### *Sand*

A Sand stratum was encountered at all but eleven (11) test boring locations. The eleven locations where the Sand stratum was not encountered include B-3 (MW) through B-5, B-16 through B-17, B-19 through B-20A, B-26, and B-29. The Sand stratum was fully penetrated at boring locations B-2 (MW), B-8, B-9, B-14, B-18, B-22, B-24, and B-25; at these locations the layer was approximately 1.8 ft. to 16.5 ft. thick. At the other boring locations, where this stratum was encountered, but not fully penetrated, the penetration depth ranged from approximately 1.8 ft. to approximately 24.2 ft. The Sand stratum typically consists of moist, very loose to very dense, fine to coarse SAND, little silt, trace fine gravel. SPT N-values in the sand stratum ranged from 2 bl/ft to over 100 bl/ft, with an average of 26 bl/ft at the test boring locations.

#### *Weathered Rock*

Weathered Rock was encountered at test boring location B-22. This layer was penetrated approximately 5.5 ft., and no material was recovered. The strata classification was based on the material that came up in the wash water and rig chatter during and speed of roller-bit advance. SPT N-values in the Weathered Rock stratum were both greater than 100 bl/ft at the test boring location.

#### *Bedrock*

Bedrock was encountered at test boring location CDM-13A. This layer was penetrated approximately 5 ft. and consists of hard, fresh, dark grey, fine- to medium-grained GRANITE; primary joint set horizontal, smooth, planar, fresh, tight. Depth to top of rock was approximately 28 ft. bgs at the exploration location. The percent recovery for the rock core was 57% and the RQD was 38% at the exploration location.

## **Groundwater Conditions**

Groundwater level measurements were recorded during drilling and at the conclusion of drilling at each boring location. Additional measurements were taken at the monitoring well locations. All measurements were taken from the ground surface.

A groundwater measurement was taken at every boring location that was advanced using the drive and wash technique, which introduces fluid into the soil. The measured groundwater levels at these locations ranged from approximately 3.5 ft. to approximately 8.8 ft. bgs. A summary of groundwater levels encountered during the drilling program is presented in **Table 1**.

At the monitoring well locations, water levels were measured at five different times from April 13 to October 4, 2017. The measured groundwater levels at these locations measured during this period ranged from approximately 3.3 ft. bgs to 6.5 ft. bgs. A summary of water level depths measured at the monitoring wells is presented in **Table 2**.

## **Variation in Subsurface Conditions**

Interpretation of general subsurface conditions presented herein is based on soil and groundwater conditions observed during the subsurface exploration program. However, subsurface conditions may vary between exploration locations. If conditions are found to be different than assumed, recommendations contained in this report should be reevaluated by CDM Smith and confirmed in writing.

Water levels in the explorations may not be representative of stabilized groundwater levels. In addition, water levels are expected to fluctuate with time due to season, temperature, climate, and construction in the area as well as other factors. Therefore, groundwater conditions at the time of construction may be different from those observed at the time of the explorations.

## **Geotechnical Laboratory Testing**

Geotechnical laboratory tests were performed on select split spoon samples obtained from the test borings. Grain size analyses were performed on 59 samples in accordance with ASTM D6913, ASTM D7928, and ASTM D1140. The purpose of these tests was to assist with soil classification and evaluate engineering properties of the soil. Geotechnical testing was conducted at the CDM Smith Geotechnical Laboratory in Somerville, Massachusetts.

Results of the geotechnical laboratory testing are summarized in **Table 3**. The complete geotechnical laboratory test results are included in **Attachment C**.

## **Geotechnical Design Recommendations**

### **Geotechnical Engineering Evaluation**

Geotechnical engineering recommendations have been made related to the current understanding of the proposed utility upgrades within the proposed project area in Salem, New Hampshire. In general, these evaluations have been made and criteria established based on the results of the

subsurface investigation and geotechnical laboratory testing programs conducted for this study, and based on published correlations with soil properties and the minimum requirements of the 2015 New Hampshire Building Code and subsequent amendments (the Code). In addition, recommended design criteria are based on performance tolerances, such as allowable settlement, as understood to relate to similar structures.

### **Structure Foundations**

Sewer manholes and associated structures should be constructed on mat foundations bearing on suitable soil that is prepared and protected in accordance with the recommendations herein, or on compacted structural fill placed after the removal of unsuitable soils. The foundations may be designed for a maximum net bearing pressure of 2 tons per square foot (tsf), provided the structures bear on suitable soil or structural fill placed after the removal of unsuitable soils.

Suitable soils include the sand, silty sand, gravelly sand, weathered rock or bedrock strata. Unsuitable soils include the fill strata, and any other organic, soft, loose or disturbed soil present at the foundation subgrade level. If boulders or bedrock are encountered above the subgrade level, the rock should be removed at least 12 inches below the bottom of the structure and replaced with compacted structural fill or screened gravel wrapped in filter fabric.

Where portions of the structures are founded on structural fill, the fill should extend at least 2 feet beyond the edge of the foundation, then outward and downward at a slope of one horizontal to one vertical (1H:1V) to the lowest subgrade elevation or the limit of steel sheet piling that is left in place.

### **Depth of Foundations**

In accordance with Section 1809.5 of the New Hampshire State Building Code, all foundations supported on soil should bear at least 4 ft below any adjacent ground surface exposed to freezing.

### **Estimated Settlement**

Settlement of the proposed structure, designed as recommended herein, is expected to be less than 1 inch with no more than 1/2 inch of differential settlement.

### **Design Groundwater Elevation**

Groundwater levels measured during both the subsurface exploration program and period of monitoring well readings ranged between approximately El 115 and El 131. However, for the purposes of design, the groundwater level should be taken to be at ground surface.

### **Resistance to Buoyancy**

Below grade structures that extend below the design groundwater elevation should be appropriately waterproofed and designed to resist hydrostatic uplift pressure (buoyancy).

The dead weight of the structure plus weight for earth placed directly over extensions to the structure foundation may be used to resist buoyancy. For the purpose of design against uplift, the soil used as backfill should be assumed to have a total unit weight, in place, of 120 pounds per cubic foot (pcf).

### **Lateral Pressure on Below-Grade Walls**

Below-grade walls that are backfilled on one side and restrained against rotation or movement at the top should be designed for lateral pressures from soil and groundwater based on an equivalent fluid unit weight of 60 pcf above the design groundwater level and 90 pcf below the design groundwater level.

A lateral pressure equal to 0.5 times the surface vertical surcharge loads from building foundations, slabs or other loads should be applied over the full height of all walls. Earthquake induced pressures in accordance with the Code should be included in the design of all below-grade walls.

Earthquake-induced pressures should be included as applicable per the Code.

### **Earthquake Considerations**

Based on the Section 1613.5 of the Code the site should be considered as Site Class "D".

Based on the subsurface investigation, the soils encountered below the proposed structure foundations at the site are not considered susceptible to liquefaction.

### **Piping**

The proposed sanitary sewer piping and watermain piping are anticipated to be installed by cut and cover methods in excavated trenches.

#### *Pipe Subgrade*

The sand, gravelly sand, silty sand, and bedrock strata encountered at the proposed pipeline subgrades are considered suitable for support of the proposed piping, provided the subgrade remains undisturbed, firm, and stable. If soft clayey silt, loose sand or gravel, organic soils, or otherwise unsuitable soils are encountered at subgrade level, these soils should be excavated to the top of a suitable stratum or a maximum of 4 feet below the pipe invert and replaced with compacted structural fill.

#### *Pipe Bedding*

The pipes should be placed on a bedding of at least 6 inches of screened gravel or compacted structural fill on a soil subgrade, or 12 inches of screened gravel or compacted structural fill on a rock subgrade. The screened gravel should be wrapped in filter fabric to prevent migration of fine-grained soils into the pipe bedding material.

### *Trench Backfill*

The pipe should be backfilled with screened gravel or structural fill to at least the level of the pipe crown. A minimum of 12 inches of compacted sand is recommended above the crown of the pipe, with filter fabric separating the sand layer and the screened gravel layer.

In paved areas, the remainder of the trench backfill, to the underside of the pavement section, should consist of select common fill and be placed and compacted to at least 95 percent of the maximum dry density as determined by laboratory test ASTM D1557.

## **Construction Considerations**

### **Primary Construction Considerations**

The primary geotechnical considerations related to the construction of the proposed utility piping and associated manholes include, but are not limited to:

- Proximity of the proposed piping and associated manholes to existing commercial properties and residential structures, utilities, and roadways.

### **Excavation**

The anticipated excavation depth for the construction of the new sewer piping and associated manholes is expected to extend up to approximately 20 ft bgs. It is anticipated that the majority of the pipeline excavation can be made using conventional earthmoving equipment. However, it is possible that occasional boulders or weathered bedrock may be encountered above the sewer subgrade which may require rock excavation techniques such as hoe-rams, rock splitting, wedging or chemical expanders. Based on the results of the subsurface exploration program, rock removal utilizing blasting methods is not anticipated and is not recommended due to the proximity of existing commercial and residential structures and utilities in service..

It is anticipated that an excavation support system may be required for the excavation due to the excavation depth to limit excavation quantities and protect adjacent facilities and utilities from disturbance. Any excavation support system should be selected, designed and installed by the Contractor. Specific recommendations pertaining to excavation support systems are discussed herein. Undermining of existing foundations must not occur. Where open excavations are feasible, the side slopes should be designed and sloped in accordance with OSHA regulations.

Excavations within the zone of influence of existing structures or utilities should not be permitted unless the design of the excavation support includes an analysis of the stability of the structure foundation or utility and as necessary, incorporates required bracing. The zone of influence is defined as a line extending at least 2 feet beyond the bottom edge of the foundation or springline of the utility, then outward and downward at a slope of 1 horizontal to 1 vertical. Excavations within a line extending beyond the foundation or springline of the utility, then outward and downward at a slope of 1 horizontal to 2 vertical may require restraining or monitoring of the existing utility or structure.



## **Excavation Support Systems**

The excavation for the construction of the sewer manholes may require the use of an excavation support system to limit excavation quantities, assist in the control of groundwater and to protect adjacent existing residential structures, utilities and roadways.

The design of the excavation support systems should be performed by a professional engineer registered in the State of New Hampshire employed by the Contractor. The design of the excavation support systems should be performed in conjunction with the design of the dewatering systems. The selection of the type of excavation support system will be performed by the Contractor. Selection of the temporary excavation support and dewatering system should consider the groundwater level, the susceptibility of the excavation subgrade to disturbance, and instability due to heaving and seepage gradients. If the subgrade becomes disturbed due to seepage inflow into the excavation, the disturbed material should be over-excavated and replaced with compacted structural fill.

In order to minimize construction impacts on nearby structures and utilities, support systems installed within 25 ft of existing structures or utilities that are to remain in service during or after construction shall be installed using non-vibratory methods.

Any vertical excavation support members installed within the zone of influence of any existing or new structures, utilities or pipelines should be left in place to avoid disturbing bearing soils. Any sheeting or soldier piles left in place should be cut off at least 5 feet below the adjacent finished grade.

## **Dewatering**

Excavations for construction of the proposed structures are expected to extend up to approximately 20 ft below the existing ground surface. The Contractor will be required to design and implement a dewatering system that maintains a dry, undisturbed and stable subgrade. The dewatering system should be designed by a professional engineer registered in the State of New Hampshire employed by the Contractor. To avoid disturbance to the subgrade, the dewatering system should be designed to maintain a factor of safety of 1.5 against upward seepage gradients and against heaving of the bottom of the excavation and be capable of maintaining the water elevation at least 2 feet below the subgrade level during the entire period of excavation. In addition, the dewatering system should be designed to not adversely impact adjacent structures, utilities or other site features.

All dewatering, handling and disposal of pumped water and any special testing should be conducted in accordance with local regulations, permits and specified requirements.

If wet weather is encountered during construction, the Contractor should take care to schedule excavations to limit the duration of open cuts, slope the bottoms of the excavations to facilitate drainage and provide berms to limit runoff into the excavation. In addition, excavated material to

be reused as fill should be stockpiled in such a manner that promotes runoff and limits saturation of the materials.

### **Preparation and Protection of Foundation Subgrades**

Care should be taken to avoid excess traffic on the excavated subgrades prior to placement of structural fill, or concrete foundations. Any unsuitable material present at the subgrade level should be removed and replaced with compacted structural fill. The exposed subgrade should be appropriately dewatered and protected against precipitation and freezing.

Granular soil subgrades should be proof rolled with at least four coverages of a vibratory compactor prior to placement of fill or concrete foundations.

Proof rolling should not be conducted where the subgrade consists of cohesive soil, however, a smooth edge bucket should be used for final excavation in such soil.

### **Protection of Existing Structures**

Protection of existing utilities and structures is the responsibility of the Contractor. The Contractor will need to take adequate measures to protect existing structures, roadways and utilities from movement and/or damage.

#### *Preconstruction Survey*

Prior to start of excavation, installation of excavation support and start of dewatering work, a pre-construction survey of existing structures, utilities and roadways should be performed within 75 feet of the work to establish baseline conditions.

#### *Settlement Monitoring*

We recommend that settlement and deformation monitoring points be established on any existing structures and in-service utilities located within a horizontal distance from construction activities of 50 ft. at a minimum. Monitoring points should also be located along the exterior of excavation support systems.

Monitoring points on existing structures and utilities should be installed and baseline surveys taken prior to the start of the installation of the excavation support system. The points should be monitored during installation of any excavation support system, excavation dewatering, backfilling, and vibratory construction activities. Monitoring points on new utilities should be installed and baseline surveys taken immediately following the installation of the structure. Monitoring points on the excavation support should be installed and baseline surveys taken prior to the start of excavation. All monitoring points should be surveyed daily during excavation, and then twice weekly thereafter until all backfilling is complete.

The Contractor should be prepared to alter the excavation methods if settlements or deformations measured exceed any of the below threshold values. If settlements or deformations exceed any of

the limiting values, the Contractor should stop all construction activities, stabilize the excavation, and revise excavation methods to prevent additional settlement or deformation. These criteria may be altered by the Engineer as necessary on a case by case basis.

<u>Monitoring Target Location</u>	<u>Threshold Value</u>	<u>Limiting Value</u>
Structures	0.25 inch	0.5 inch
Utilities	0.5 inch	1.0 inch
Excavation Support System	1.0 inch	2.0 inch

### **Vibration Criteria**

Ground vibrations due to construction can cause damage to adjacent structures, utilities and other facilities. To avoid or mitigate this potential damage, limits on ground vibrations in the form of ground displacement, velocity, or acceleration at given frequencies are typically established. The Bureau of Mines has established criteria to limit ground vibrations using the peak particle velocity (PPV) and frequency parameters. These limits have been established using the cracking of plaster walls in a residential house as a model.

The maximum peak particle velocities associated with vibration-inducing construction methods and at the ground surface at existing adjacent structures and utilities should be as follows:

Frequency (Hz)	Max. Peak Particle Velocity (in. per sec.)	
	<u>Threshold</u>	<u>Limiting</u>
Over 40	1.0	2.0
30 to 40	0.75	1.5
20 to 30	0.5	1.0
Less than 20	0.35	0.5

In no case should the maximum peak particle velocities caused by the use of vibratory or driving hammers exceed 2.0 inches per second at the closest facility (structure or utility) to the work. If the induced vibration criteria cannot be met at any existing structure, other means or non-vibration construction methods will be required (e.g. line drilling, chemical expansion, drill and socket soldier piles, etc.).

At least one seismograph should be located at adjacent/nearby structures and utilities during all vibratory construction activities to confirm compliance with the recommendations herein and record actual impact vibrations.

## **Backfill Materials and Compaction Requirements**

### *Screened Gravel*

Screened Gravel should be hard, durable, rounded, or subangular particles of proper size and gradation, and should be free from sand, loam, clay, excess fines, and other deleterious materials. The material should conform to the following gradation:

U.S. Standard Sieve Size	Percent Passing by Weight
5/8 in.	100
1/2 in.	40-100
3/8 in.	15-45
No. 10	0-5

Material conforming to Standard Stone Size #67, as defined in Table 703.1 of the New Hampshire DOT Standard Specifications for Road and Bridge Construction is also acceptable. Screened gravel shall be wrapped in filter fabric in order to prevent migration of fines into the material. Filter fabric shall be non-woven geotextile, Mirafi 140N or approved equivalent, and should be installed as required in the Specifications.

### *Structural Fill*

Granular fill used as structural fill below footings, mat foundations, slabs-on-grade, and road pavement sections should consist of a mineral soil free of organic material, loam, debris, frozen soil or other deleterious material which may be compressible or which cannot be properly compacted. Structural fill should conform to the following gradation requirements:

U.S. Standard Sieve Size	Percent Passing by Weight
3 inches	100
No. 4	20-70
No. 40	5-35
No. 200	0-10

Structural fill should be placed in layers no thicker than 8 inches, as placed, and compacted with suitable compaction equipment to at least 95 percent of maximum dry density as determined by ASTM D1557. Lift thickness should be reduced to 4 inches in confined areas accessible only to hand guided compaction equipment.

### *Common Fill*

Common fill used as backfill around structures where passive pressure is not relied on, in parking areas, and landscaped areas should consist of granular soil free from organic material, debris, frozen soil, or other deleterious material. It should contain stones no larger than 6 inches and have no more than 30 percent of material passing the No. 200 sieve.

Common fill should be placed in layers not to exceed 12 inches, as placed, and compacted with suitable vibratory compaction equipment to at least 92 percent of maximum dry density as determined by ASTM D1557. Lift thickness should be reduced to 6 inches in confined areas accessible only to hand guided compaction equipment.

We anticipate that the some of the excavated soil may be suitable for reuse as common fill. Care should be taken to separate layers of topsoil encountered during the excavation.

#### *Select Common Fill*

Select common fill should be the same as common fill except that it should not contain stone larger than 2 inches. Select common fill should consist of soil, free from organic material, loam, debris, frozen soils or other deleterious material which may be compressible or which cannot be compacted properly.

Select common fill should be placed in layers no thicker than 8 inches, as placed, and compacted with suitable compaction equipment to at least 95 percent of maximum dry density as determined by ASTM D1557. Lift thickness should be reduced to 4 inches in confined areas accessible only to hand guided compaction equipment.

#### **Construction Monitoring**

It is recommended that a qualified Geotechnical Engineer or an experienced Resident Engineer be present during construction to confirm that the Contractor complies with the intent of these recommendations. Specifically, the field representative would undertake the following responsibilities:

- Confirm removal of unsuitable materials as recommended herein;
- Monitor the excavation and installation of excavation support system;
- Confirm that appropriate dewatering and surface water control methods are employed;
- Confirm that the subgrade conditions encountered are suitable for support of the proposed structures as required by the Code;
- Confirm that the subgrades are appropriately prepared and protected prior to placement of concrete or fill;
- Observe, test and document placement and compaction of fill and backfill material; and
- Observe the installation of geotechnical instrumentation and review site monitoring data collected.

In addition, the field representative would be present to identify and provide response should conditions encountered differ from those assumed during preparation of this report.

## Closing

These recommendations have been prepared for the design and construction for the South Broadway Water and Sewer Improvements, located in the Town of Salem, New Hampshire as understood at this time and described in this memorandum. These recommendations have been prepared in accordance with generally accepted engineering practices. No other warranty, express or implied, is made. In the event that changes in the design or location of the alignment occur, the conclusions and recommendations contained herein should not be considered valid unless verified in writing by CDM Smith.

### *Figures*

**Figures 1 through 8** – Subsurface Exploration Location Plan

### *Tables*

**Table 1** – Summary of Subsurface Exploration Program

**Table 2** – Summary of Water Level Readings in Monitoring Wells

**Table 3** – Summary of Geotechnical Laboratory Test Results

### *Attachments*

**Attachment A** – Test Boring Logs

**Attachment B** – Monitoring Well Installation Logs and Monitoring Well Reports

**Attachment C** – Geotechnical Laboratory Test Results

## Figures

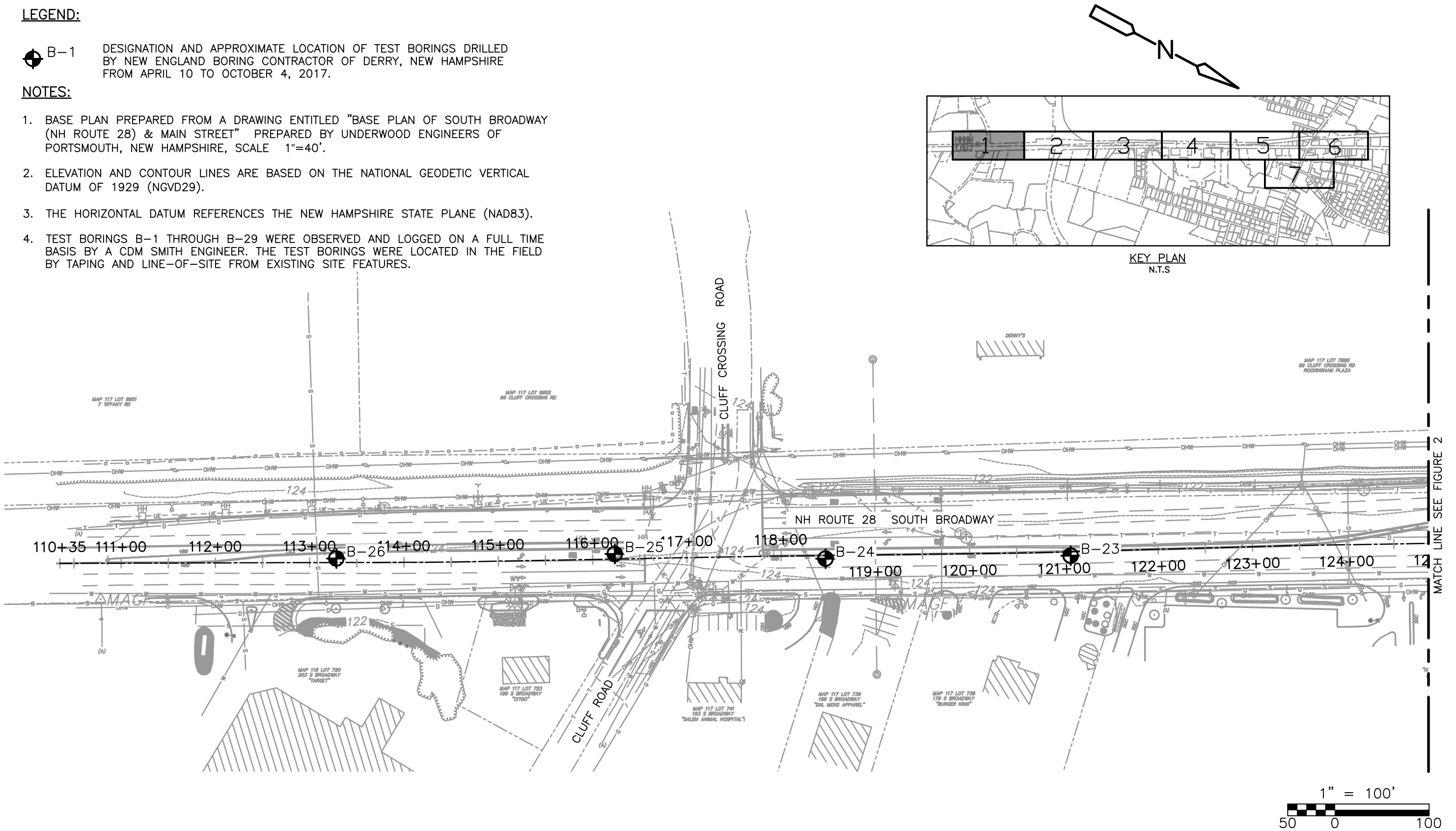
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LEGEND:

B-1 DESIGNATION AND APPROXIMATE LOCATION OF TEST BORINGS DRILLED BY NEW ENGLAND BORING CONTRACTOR OF DERRY, NEW HAMPSHIRE FROM APRIL 10 TO OCTOBER 4, 2017.

NOTES:

1. BASE PLAN PREPARED FROM A DRAWING ENTITLED "BASE PLAN OF SOUTH BROADWAY (NH ROUTE 28) & MAIN STREET" PREPARED BY UNDERWOOD ENGINEERS OF PORTSMOUTH, NEW HAMPSHIRE, SCALE 1"=40'.
2. ELEVATION AND CONTOUR LINES ARE BASED ON THE NATIONAL GEODETIC VERTICAL DATUM OF 1929 (NGVD29).
3. THE HORIZONTAL DATUM REFERENCES THE NEW HAMPSHIRE STATE PLANE (NAD83).
4. TEST BORINGS B-1 THROUGH B-29 WERE OBSERVED AND LOGGED ON A FULL TIME BASIS BY A CDM SMITH ENGINEER. THE TEST BORINGS WERE LOCATED IN THE FIELD BY TAPING AND LINE-OF-SITE FROM EXISTING SITE FEATURES.



TOWN OF SALEM  
SOUTH BROADWAY WATERMAIN REPLACEMENT  
SALEM, NEW HAMPSHIRE

FIGURE 1  
SUBSURFACE EXPLORATION LOCATION PLAN  
DECEMBER 2017





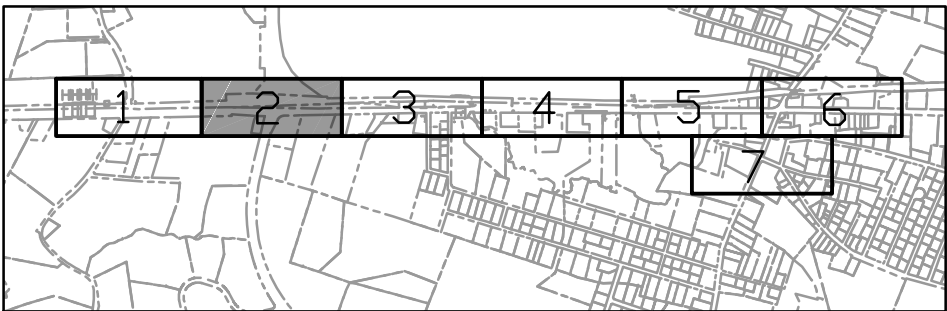
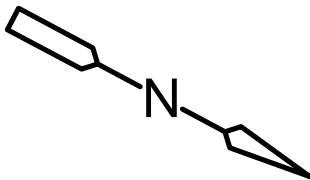
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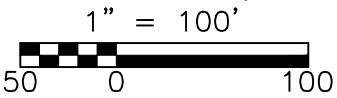
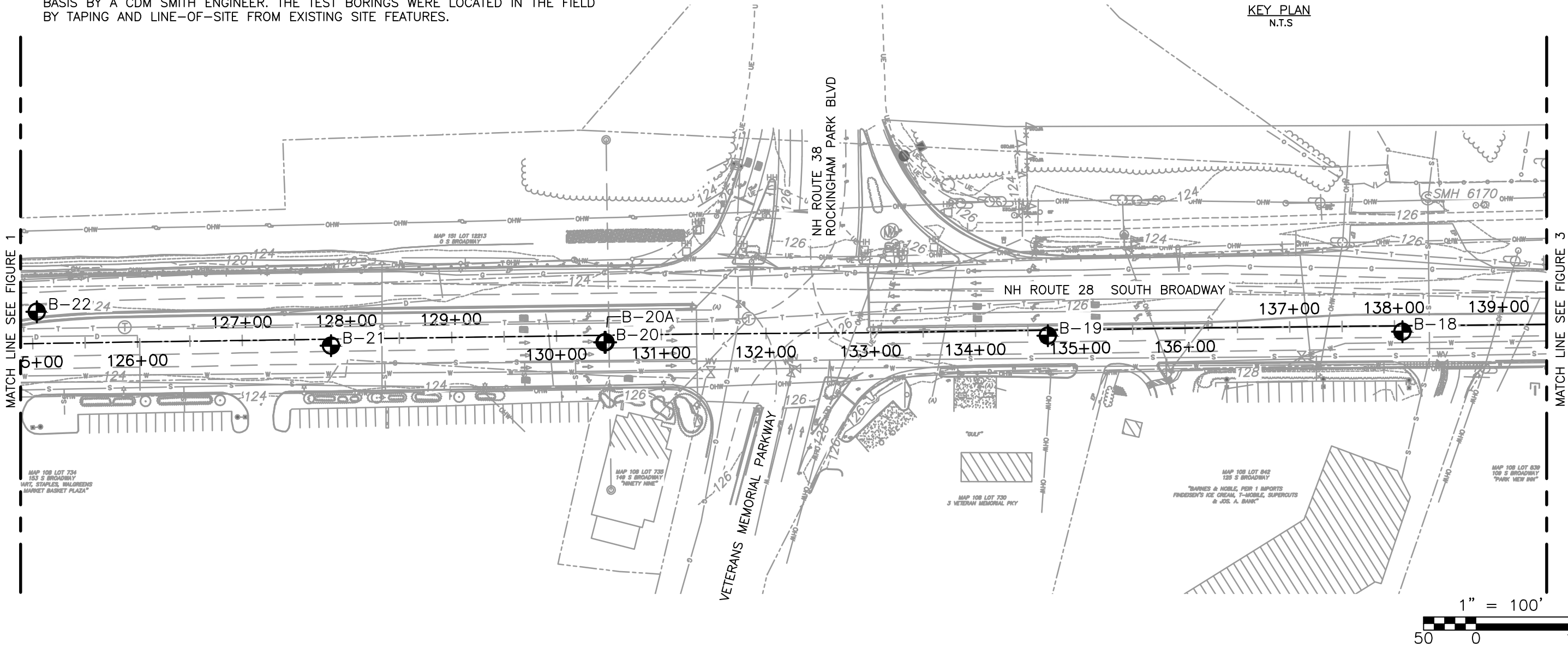
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FIGURE 2  
SUBSURFACE EXPLORATION LOCATION PLAN  
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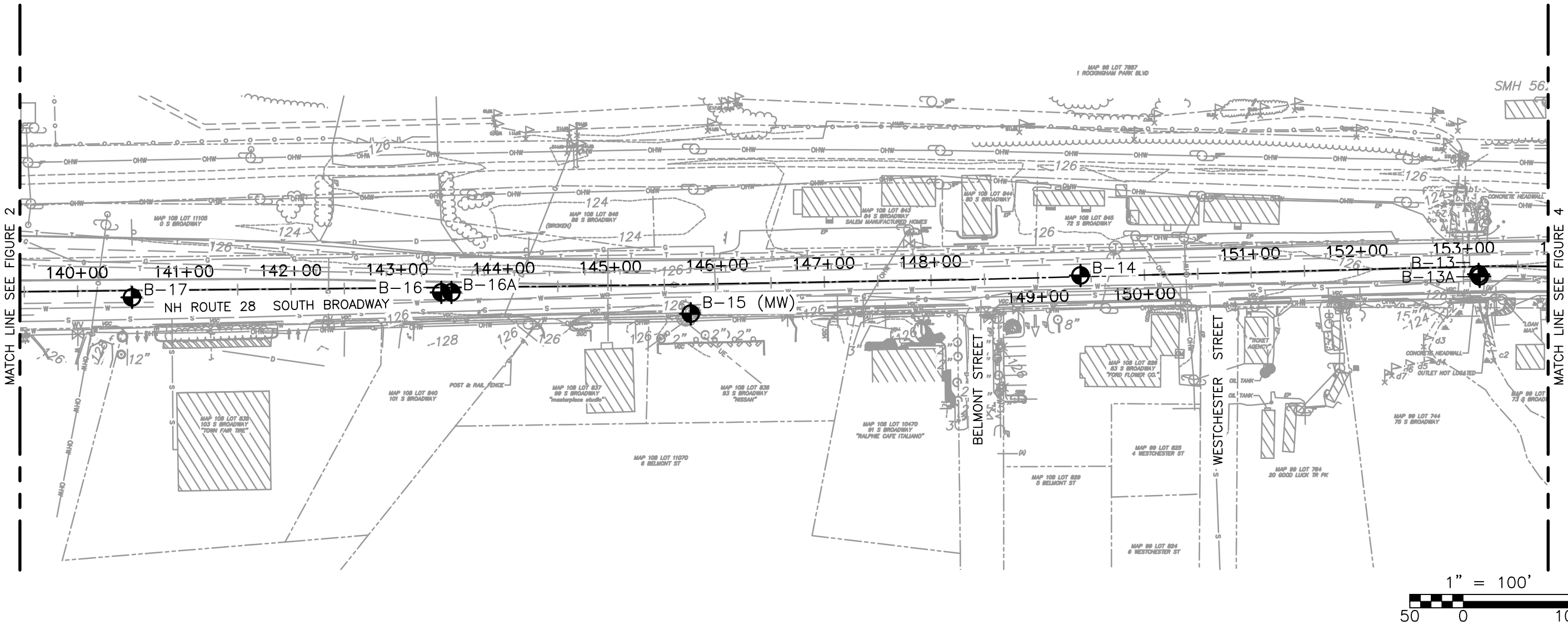
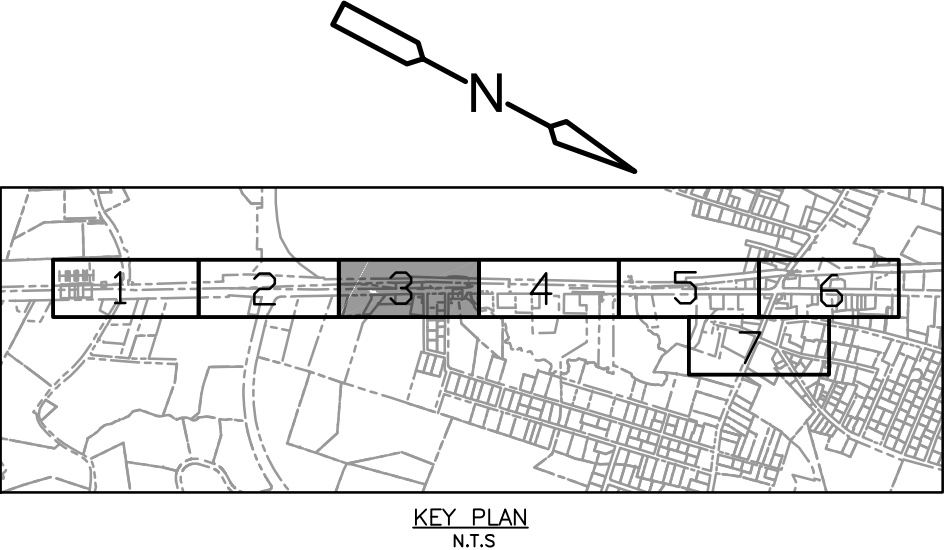
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SOUTH BROADWAY WATERMAIN REPLACEMENT  
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FIGURE 3  
SUBSURFACE EXPLORATION LOCATION PLAN  
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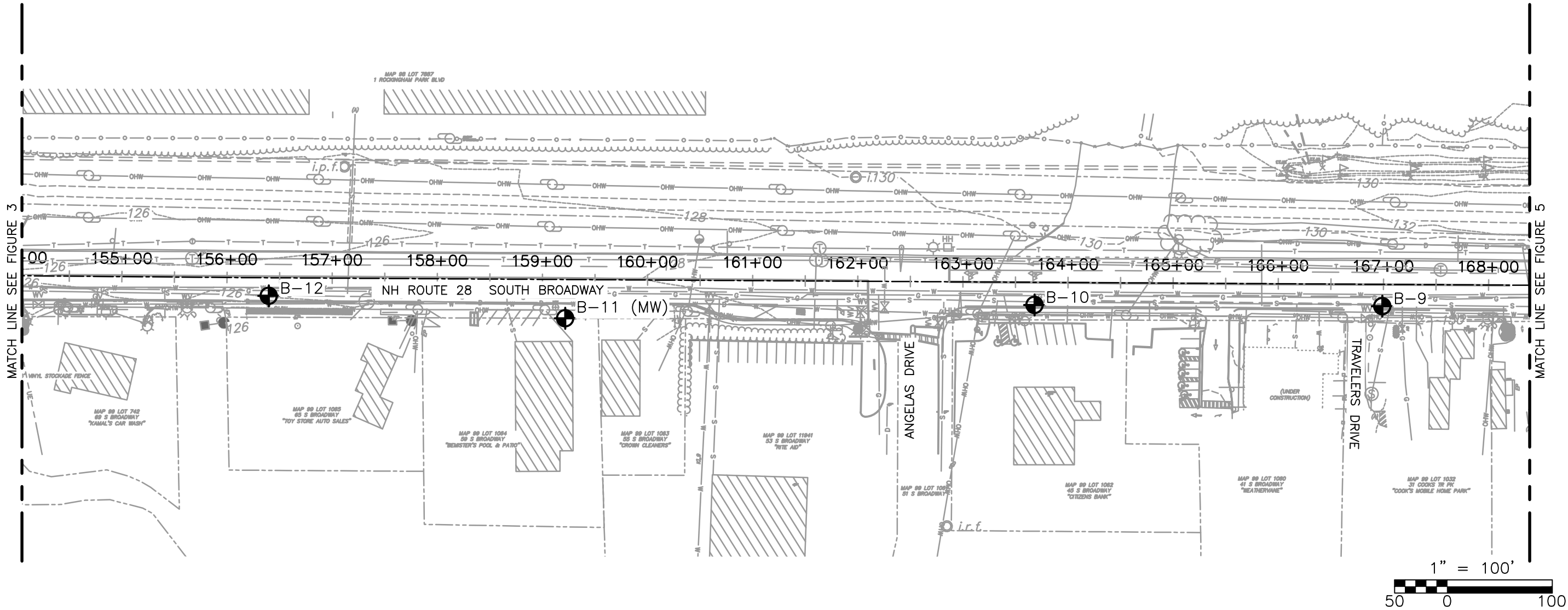
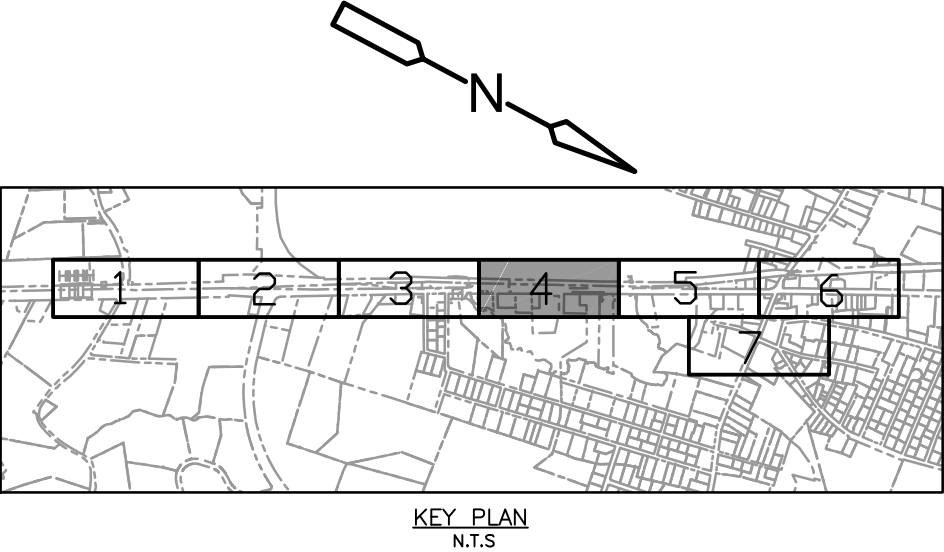
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FIGURE 4  
SUBSURFACE EXPLORATION LOCATION PLAN  
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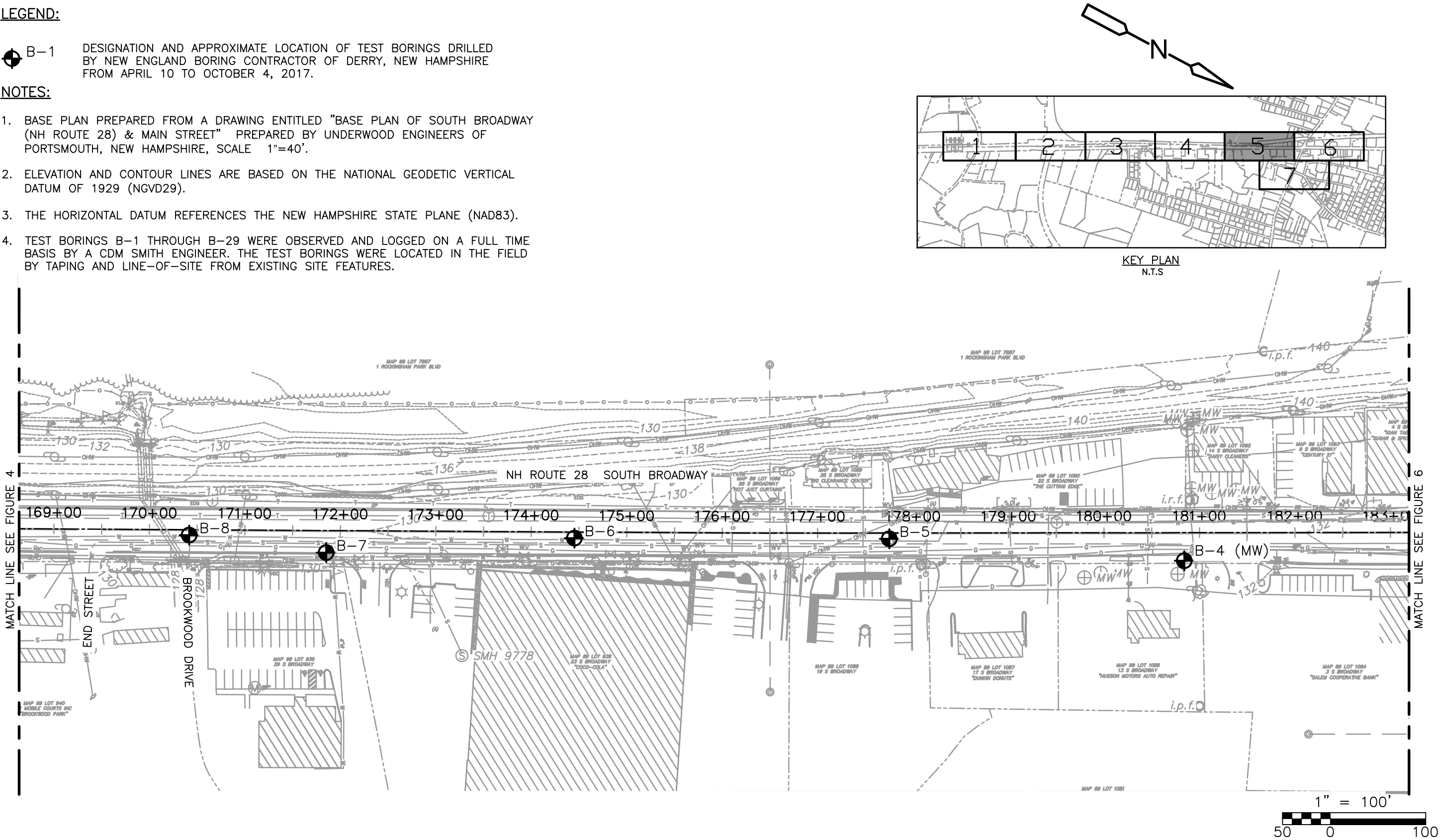
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FIGURE 5  
SUBSURFACE EXPLORATION LOCATION PLAN  
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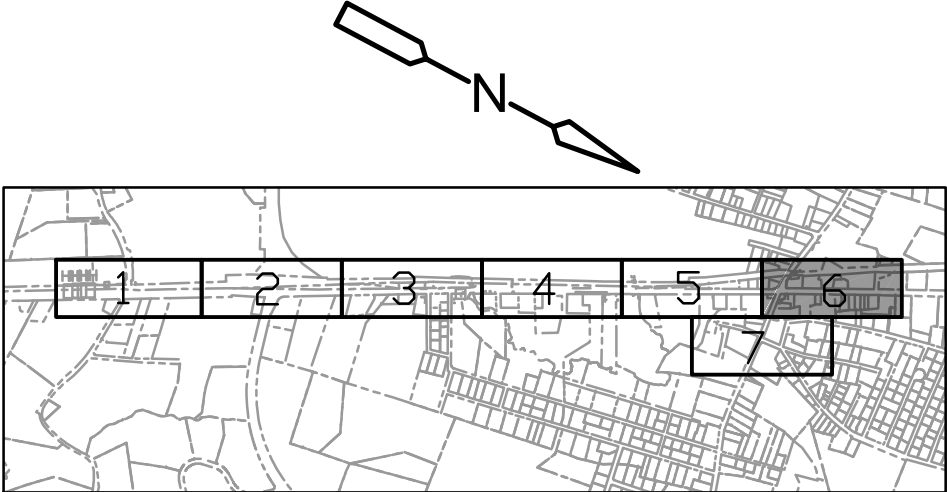
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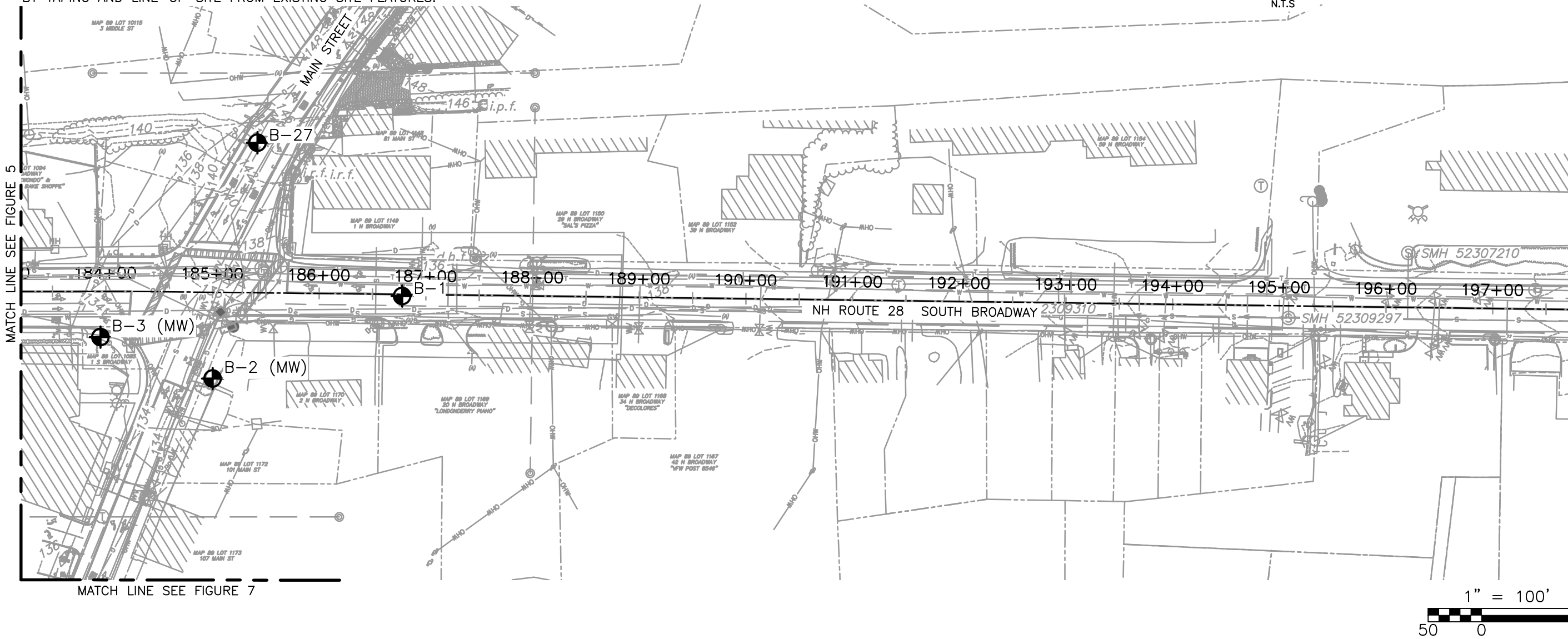
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FIGURE 6  
SUBSURFACE EXPLORATION LOCATION PLAN  
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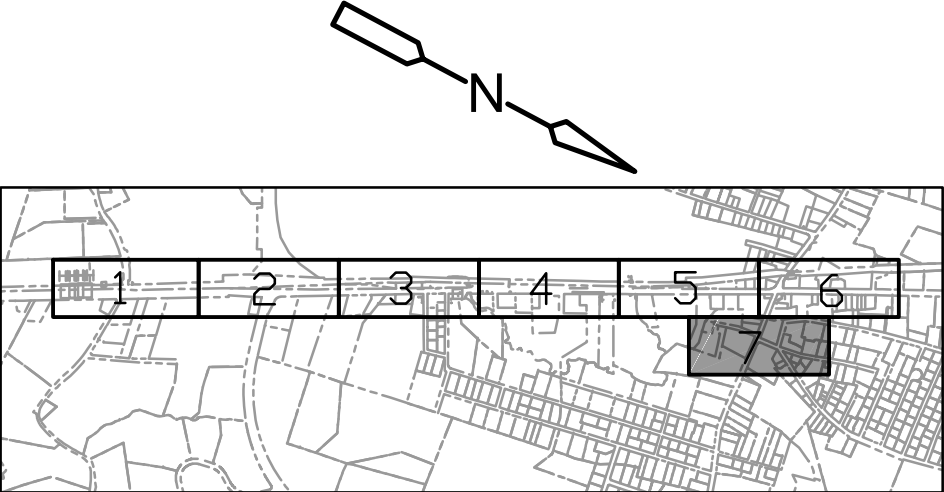
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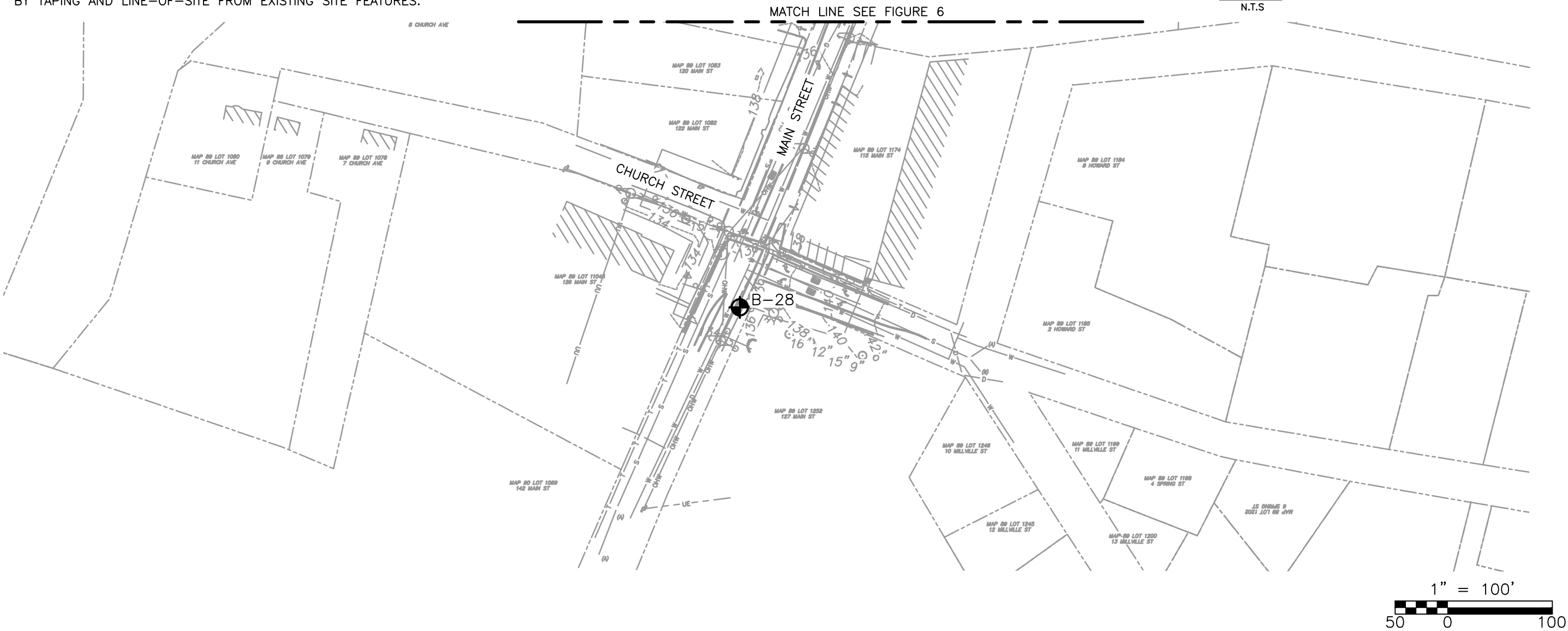
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TOWN OF SALEM  
SOUTH BROADWAY WATERMAIN REPLACEMENT  
SALEM, NEW HAMPSHIRE

FIGURE 7  
SUBSURFACE EXPLORATION LOCATION PLAN  
DECEMBER 2017





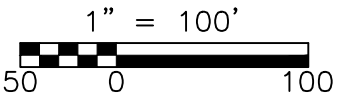
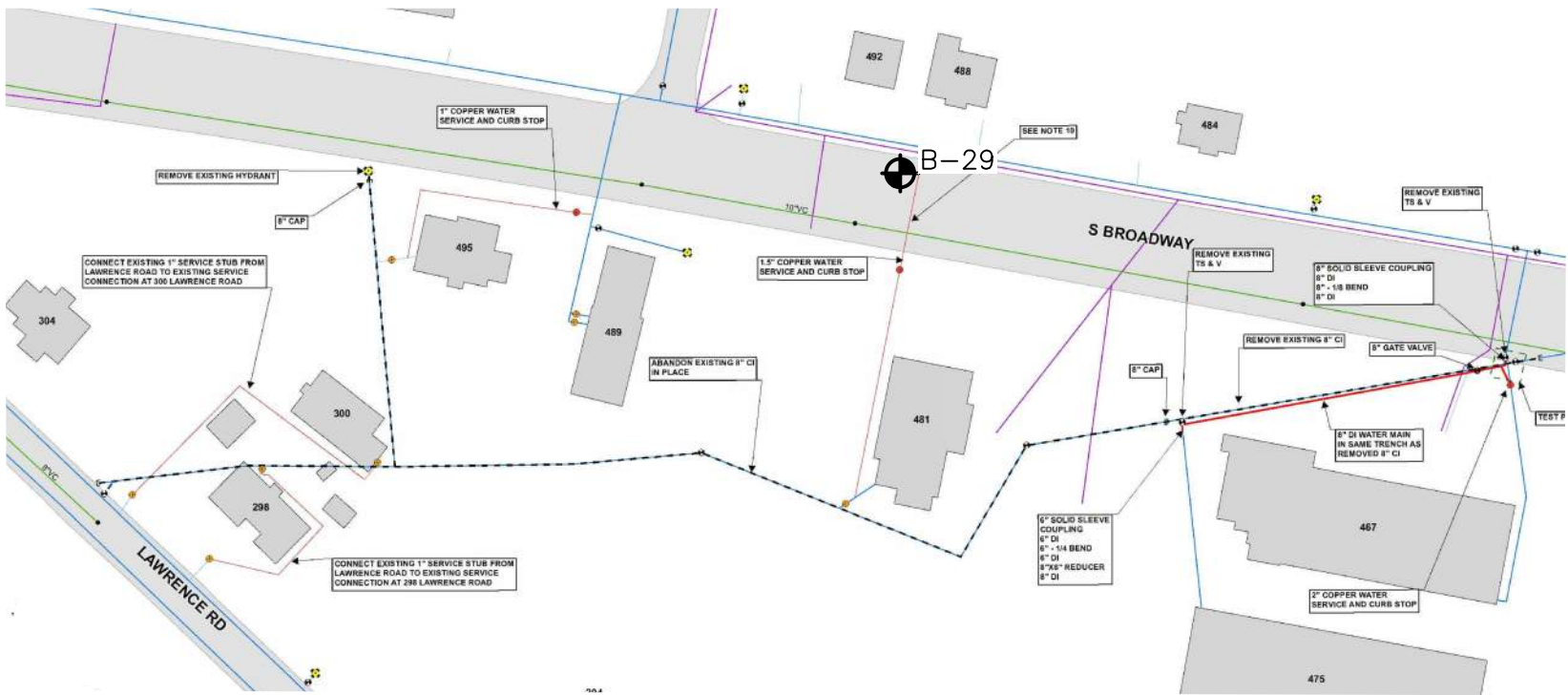
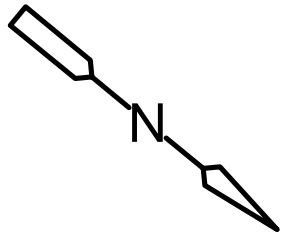
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SOUTH BROADWAY WATERMAIN REPLACEMENT  
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FIGURE 8  
SUBSURFACE EXPLORATION LOCATION PLAN  
DECEMBER 2017



## Tables



**Town of Salem**  
**South Broadway Watermain and Sewer Replacement**  
**Salem, NH**

**Table 1**  
**Summary of Recent Subsurface Exploration Program**

Exploration Number	Approximate Ground Surface Elevation (ft) <sup>(1)</sup>	Exploration Depth (ft)	Stratum Thickness (ft) <sup>(2)</sup>										Depth to Top of Bedrock (ft)	Approximate Top of Bedrock Elevation (ft)	Depth to Groundwater (ft) <sup>(3)</sup>	Approximate Groundwater Elevation (ft)	
			Asphalt	Subbase	Concrete	Fill	Clayey Silt	Gravelly Sand	Silty Sand	Sand	Weathered Rock	Bedrock					
B-1	135.3	20.0	0.4	NE	NE	NE	NE	NE	6.0	5.6, > 8	NE	NE	NE	NE	6.1	129.2	
B-2 (MW)	135.0	21.0	NE	NE	NE	NE	NE	NE	g <sup>(4)</sup> , > 9.0	4.0	NE	NE	NE	NE	7.3	127.8	
B-3 (MW)	133.3	22.0	NE	NE	NE	NE	NE	12.0	> 10.0	NE	NE	NE	NE	NE	5.0	128.3	
B-4 (MW)	131.2	21.0	0.4	NE	NE	NE	NE	7.6	> 13.0	NE	NE	NE	NE	NE	7.5	123.7	
B-5	130.7	20.0	0.5	0.5	NE	5.0	11.0	NE	> 3.0	NE	NE	NE	NE	NE	6.0	124.7	
B-6	130.1	21.0	0.4	NE	0.6	NE	NE	NE	NE	> 20	NE	NE	NE	NE	6.4	123.7	
B-7	129.2	20.0	0.6	NE	0.4	NE	NE	NE	NE	> 19	NE	NE	NE	NE	5.7	123.5	
B-8	128.1	40.0	0.6	NE	0.4	NE	NE	NE	> 22.5	16.5	NE	NE	NE	NE	7.0	121.1	
B-9	129.1	20.0	0.7	NE	NE	NE	> 8.0	NE	8.0	3.3	NE	NE	NE	NE	4.9	124.2	
B-10	126.1	8.0	0.7	0.3	NE	NE	NE	NE	NE	> 7.0	NE	NE	NE	NE	3.5	122.6	
B-11 (MW)	126.4	19.0	0.5	NE	NE	NE	NE	NE	NE	> 18.5	NE	NE	NE	NE	3.1	123.3	
B-12	126.1	25.0	0.8	NE	NE	NE	NE	NE	NE	> 24.2	NE	NE	NE	NE	5.0	121.1	
B-13	125.2	28.0	0.6	NE	0.4	NE	NE	1.5, 8.0	NE	11.5, > 6.0	NE	NE	NE	NE	4.0	121.2	
B-13A	125.2	33.0	Drilled directly to 28 ft to continue exploration from B-13.									NE	> 5.0	28.0	97.2	8.0	117.2
B-14	125.9	25.0	0.5	NE	0.5	0.8	NE	> 13.0	NE	10.2	NE	NE	NE	NE	5.0	120.9	
B-15 (MW)	125.9	19.3	0.3	NE	NE	NE	NE	NE	NE	>19.0	NE	NE	NE	NE	5.0	120.9	
B-16	125.5	8.0	0.5	NE	0.3	NE	NE	2.2	5.8 <sup>(5)</sup>	NE	NE	NE	NE	NE	NE	NE	
B-16A	125.5	25.0	NR	NR	NR	NE	> 17.0	NE	NE	NE	NE	NE	NE	NE	4.1	121.4	
B-17	124.2	25.0	0.8	NE	NE	NE	NE	NE	> 24.2	NE	NE	NE	NE	NE	7.8	116.4	
B-18	125.1	21.0	0.7	NE	NE	NE	NE	NE	> 22.5	1.8	NE	NE	NE	NE	8.2	116.9	
B-19	125.3	25.0	0.6	NE	0.4	NE	NE	1.5	22.5	NE	NE	NE	NE	NE	5.8	119.5	
B-20	123.2	13.0	0.4	NE	0.5	NE	NE	1.6, > 2.0	8.5	NE	NE	NE	NE	NE	4.2	119.0	
B-20A	123.2	16.5	Drilled directly to 14 ft to continue exploration from B-20.						> 2.0	NE	NE	NE	NE	NE	NR	NR	
B-21	124.4	25.0	0.4	NE	0.4	NE	NE	NE	11.2	> 13.0	NE	NE	NE	NE	7.8	116.6	
B-22	124.3	23.0	0.7	0.3	NE	NE	NE	NE	NE	16.5	> 5.5	NE	NE	NE	4.0	120.3	
B-23	124.3	24.0	0.6	NE	0.4	NE	NE	NE	6.0	5.0, > 12.0	NE	NE	NE	NE	7.0	117.3	
B-24	124.4	25.0	0.6	NE	0.4	NE	NE	> 13.0	6.0	5.0	NE	NE	NE	NE	7.2	117.2	
B-25	123.5	26.0	0.5	NE	0.5	NE	NE	NE	> 18.0	7.0	NE	NE	NE	NE	7.4	116.1	
B-26	123.5	26.0	0.4	0.2	0.4	NE	NE	NE	> 25.0	NE	NE	NE	NE	NE	8.8	114.7	
B-27	143.1	10.0	0.4	NE	NE	NE	NE	NE	NE	> 9.6	NE	NE	NE	NE	NE	NE	
B-28	136.2	10.0	NE	NE	NE	NE	NE	NE	NE	> 10.0	NE	NE	NE	NE	NE	NE	
B-29	NR	7.0	0.3	NE	NE	NE	NE	> 6.7	NE	NE	NE	NE	NE	NE	NE	NE	

**Notes:**

1. Elevations are referenced to North American Vertical Datum of 1988 (NAVD88) and are estimated from a topographic survey.
2. Commas indicate that a strata was encountered twice; the second value is the thickness of the lower strata
3. Groundwater depths were measured upon completion of test borings.
4. Results of Organic Content tests run on the samples in the upper Silty Sand strata in this boring were greater than 5%
5. An obstruction was encountered at 8 ft bgs.

**Abbreviations:**

> Indicates Strata not fully penetrated  
NE indicates not encountered  
NR indicates value not recorded  
MW indicates a monitoring well

**Town of Salem**  
**South Broadway Watermain and Sewer Replacement**  
**Salem, NH**

**Table 2**  
**Summary of Water Level Readings in Monitoring Wells**

Exploration Number	Ground Surface Elevation (ft)	Depth Below Ground Surface (ft)				
		4/13/2017	6/8/2017	6/29/2017	7/24/2017	10/4/2017
B-2 (MW)	135.0	3.90	5.68	6.20	6.09	6.50
B-3 (MW)	133.30	4.60	4.53	5.10	5.14	5.80
B-4 (MW)	131.20	3.80	4.08	4.60	4.62	5.30
B-11 (MW)	126.40	3.30	4.00	4.79	5.07	6.20
B-15 (MW)	125.90	5.00	3.21	4.30	4.61	5.30

**Notes:**

See Subsurface Exploration Location Plans for the locations of the monitoring wells.

**Town of Salem**  
**South Broadway Watermain and Sewer Replacement**  
**Salem, NH**

**Table 3**  
**Summary of Geotechnical Laboratory Test Results**

Exploration Number	Sample Number	Sample Depth (ft)	Stratum	USCS Classification <sup>(1)</sup>	Grain Size Analysis <sup>(2)</sup>						Moisture Content (%) <sup>(3)</sup>	Organic Content (%) <sup>(4)</sup>	Atterberg Limits <sup>(5)</sup>			
					Gravel (%)		Sand (%)			Fines (%)			LL(%)	PL(%)	PI(%)	
					Coarse	Fine	Coarse	Medium	Fine	Silt	Clay					
B-1	S-5	8 - 10	Silty Sand	ML	0.0	0.0	0.0	1.4	14.7	56.3	27.6	22.0	--	--	--	--
B-1	S-6	14 - 16	Sand	SP-SM	3.5	13.8	8.0	25.3	37.9	11.5		12.9	--	--	--	--
B-2	S-4	6 - 8	Organic Silty Sand	--	--	--	--	--	--	--		96.9	14.5	--	--	--
B-2	S-5	8 - 10		SP-SM	6.0	4.6	3.9	29.2	46.8	9.5		20.0	--	--	--	--
B-2	S-6	14 - 16	Silty Sand	ML	0.0	0.0	0.0	0.1	10.1	58.3	31.5	26.5	--	--	--	--
B-3	S-4	6 - 8	Gravelly Sand	OL/ML	0.0	0.0	0.4	14.0	22.8	46.8	16.0	92.3	13.0	--	--	--
B-4	S-5	8 - 10	Silty Sand	ML	0.0	0.0	0.0	1.4	12.1	68.3	18.2	24.0	--	--	--	--
B-5	S-5	8 - 10	Clayey Silt	ML	--	--	--	--	--	--		25.7	--	31	25	6
B-5	S-6	14 - 16	Clayey Silt	ML	0.0	0.0	0.0	0.6	8.7	90.7		21.1	--	NV	NP	NP
B-6	S-3	4 - 6	Sand	SP	0.0	2.8	2.5	35.6	54.3	4.8		18.9	--	--	--	--
B-6	S-5	8 - 10	Sand	SP-SM	0.0	0.4	0.1	13.5	80.5	5.5		23.0	--	--	--	--
B-6	S-6	14 - 16	Sand	SM	0.0	0.0	0.1	0.1	82.0	17.8		28.4	--	--	--	--
B-7	S-5	8 - 10	Sand	SP-SM	0.0	0.0	0.3	8.2	85.2	6.3		24.3	--	--	--	--
B-7	S-6	14 - 16	Sand	SP-SM	0.0	0.0	0.2	2.0	89.8	8.0		25.8	--	--	--	--
B-8	S-3	4 - 6	Sand	SP-SM	0.0	0.0	0.3	25.3	67.9	6.5		18.3	--	--	--	--
B-8	S-5	8 - 10	Sand	SM	0.0	0.0	0.2	8.9	59.0	31.9		23.8	--	--	--	--
B-8	S-8	24 - 26	Silty Sand	ML	0.0	0.0	0.0	0.2	32.3	62.6	4.9	26.3	--	--	--	--
B-9	S-1A	1 - 2.5	Sand	SM	0.0	5.2	3.4	17.2	40.3	33.9		18.4	--	--	--	--
B-9	S-5	8 - 10	Silty Sand	ML	0.0	0.0	1.7	3.2	32.3	62.8		31.0	--	--	--	--
B-9	S-6	14 - 16	Clayey Silt	ML	--	--	--	--	--	--		32.2	--	NV	NP	NP
B-10	BAG (Subbase)	0 - 1	Subbase	GP	87.6	7.1	1.1	2.2	1.3	0.7		2.8	--	--	--	--
B-10		S-3	4 - 6	Sand	--	--	--	--	--	--		27.9	4.2	--	--	--
B-11		S-5	8 - 10	Sand	SP	11.4	4.0	6.2	25.8	48.9	3.7	14.3	--	--	--	--
B-12		S-5	8 - 10	Sand	SP-SM	0.0	0.0	1.3	5.8	85.4	7.5	25.0	--	--	--	--
B-12		S-6	14 - 16	Sand	SM	0.0	0.0	0.2	2.8	77.3	19.7	24.4	--	--	--	--
B-13	S-4	6 - 8	Sand	--	--	--	--	--	--	--		23.6	0.8	--	--	--
B-13	S-5	8 - 10	Sand	SP	0.0	0.1	0.3	21.0	76.5	2.1		23.1	--	--	--	--
B-14	S-5	8 - 10	Sand	SM	0.0	1.7	0.0	0.2	80.2	17.9		25.1	--	--	--	--
B-14	S-7	19 - 21	Clayey Silt	ML	0.0	0.0	0.1	0.2	3.4	96.3		31.5	--	30	27	3
B-15	S-1	5 - 7	Silty Sand	ML	0.0	0.0	0.0	0.7	35.4	48.2	15.7	23.2	--	--	--	--
B-15	S-3	9 - 11	Silty Sand	SM	0.0	10.5	2.0	2.5	47.5	37.5		21.0	--	--	--	--
B-15	S-4	14 - 16	Silty Sand	SM	11.7	25.3	5.3	6.4	26.0	25.3		12.7	--	--	--	--
B-16A	S-1	8 - 10	Clayey Silt	ML	0.0	0.0	0.0	0.6	4.9	84.3	10.2	28.1	--	--	--	--
B-16A	S-4	23 - 25	Clayey Silt	CL-ML	--	--	--	--	--	--		29.0	--	27	22	5
B-17	S-2	2.5 - 4	Silty Sand	--	--	--	--	--	--	--		25.5	2.0	--	--	--

**Town of Salem**  
**South Broadway Watermain and Sewer Replacement**  
**Salem, NH**

**Table 3**  
**Summary of Geotechnical Laboratory Test Results**

Exploration Number	Sample Number	Sample Depth (ft)	Stratum	USCS Classification <sup>(1)</sup>	Grain Size Analysis <sup>(2)</sup>						Moisture Content (%) <sup>(3)</sup>	Organic Content (%) <sup>(4)</sup>	Atterberg Limits <sup>(5)</sup>			
					Gravel (%)		Sand (%)			Fines (%)			LL(%)	PL(%)	PI(%)	
					Coarse	Fine	Coarse	Medium	Fine	Silt	Clay					
B-17	S-5	8 - 10	Silty Sand	SM	0.0	0.0	0.0	0.2	79.5	19.0	1.3	25.0	--	--	--	--
B-17	S-7	19 - 21	Silty Sand	ML	0.0	0.0	0.0	0.5	7.2	82.4	9.9	29.7	--	--	--	--
B-18	S-4	6 - 8	Silty Sand	ML	0.0	0.0	0.0	0.1	17.9	78.5	3.5	24.5	--	--	--	--
B-18	S-6	14 - 16	Silty Sand	SM	0.0	0.0	0.0	0.1	66.6	32.1	1.2	25.8	--	--	--	--
B-19	S-2	4 - 6	Silty Sand	SM	0.0	0.0	0.9	5.1	55.7	38.3		25.5	--	--	--	--
B-19	S-5	14 - 16	Silty Sand	ML	0.0	0.0	0.0	0.4	2.6	80.3	16.7	27.1	--	--	--	--
B-21	S-3	4 - 6	Silty Sand	SM	0.0	0.0	1.2	14.5	71.0	13.3		30.1	4.4	--	--	--
B-21	S-5	8 - 10	Silty Sand	ML	0.0	0.0	0.0	0.1	4.1	66.6	29.2	26.8	--	NV	23	NP
B-21	S-6	14 - 16	Sand	SW-SM	0.0	6.6	12.8	35.7	37.4	7.5		17.3	--	--	--	--
B-21	S-7	19 - 21	Sand	SM	26.0	15.7	8.9	13.8	23.0	12.6		8.0	--	--	--	--
B-22	BAG (Subbase)	0 - 1	Sand	SM	0.0	26.0	12.7	26.9	18.4	16.0		11.8	--	--	--	--
B-22	S-5	8 - 10	Sand	SM	4.0	5.3	4.5	16.1	38.6	28.1	3.4	16.6	--	--	--	--
B-22	S-6	14 - 16	Sand	SM/SC	3.7	33.2	11.4	17.3	17.4	17.0		11.4	--	--	--	--
B-23	S-5	8 - 10	Silty Sand	ML	--	--	--	--	--	--		32.5	--	32	26	6
B-23	S-6	14 - 16	Sand	SM	0.0	16.6	7.9	16.1	34.3	22.5	2.6	13.7	--	--	--	--
B-24	S-5	8 - 10	Silty Sand	SM	0.0	0.0	1.4	12.2	43.5	32.6	10.3	22.3	--	--	--	--
B-24	S-6	14 - 16	Gravelly Sand	SM	22.6	16.4	8.1	13.0	20.4	19.5		8.7	--	--	--	--
B-24	S-7	19 - 21	Gravelly Sand	GP-GM	25.2	31.6	12.5	8.3	10.9	11.5		7.0	--	--	--	--
B-25	S-3	4 - 6	Sand	SP-SM	0.0	10.5	2.7	8.5	70.1	8.2		17.6	2.4	--	--	--
B-25	S-6	14 - 16	Silty Sand	ML	0.0	0.0	0.0	0.3	2.6	89.3	7.8	28.8	--	--	--	--
B-25	S-7	19 - 21	Silty Sand	ML	0.0	0.0	0.0	0.2	7.2	67.3	25.3	29.1	--	--	--	--
B-26	S-5	8 - 10	Silty Sand	SP-SM	0.0	0.0	0.1	0.1	94.6	5.2		24.8	--	--	--	--
B-26	S-6	14 - 16	Silty Sand	SM	0.0	0.0	0.0	0.1	79.7	20.2		27.5	--	--	--	--
B-26	S-7	19 - 21	Silty Sand	ML	0.0	0.0	0.0	0.1	5.9	94.0		26.5	--	--	--	--

**Notes:**

- USCS classifications were performed in accordance with ASTM D2488.
- Grain size analysis tests performed in accordance with ASTM D6913, ASTM D7928, and ASTM D1140.
- Moisture content analysis performed in accordance with ASTM D2216.
- Organic content analysis performed in accordance with ASTM D2974.
- Atterberg Limit analysis performed in accordance with ASTM D4318.

**Abbreviations:**

ML: Silt	CL-ML: Low Plasticity Silty Clay
SP-SM: Poorly Graded Sand with Silt	SW-SM: Well-Graded Sand with Silt
OL: Low Plasticity Organic Silt	GP-GM: Poorly Graded Gravel with Silt
SP: Poorly Graded Sand	SC: Clayey Sand
SM: Silty Sand	-- : Test not performed
GP: Poorly Graded Gravel	

## **Attachment A – Test Boring Logs**



# Boring Number: B-1

**Client:** Town of Salem  
**Project Location:** Salem, NH

**Project Name:** S. Broadway Watermain Replacement  
**Project Number:** 25972-215427

**Drilling Contractor/Driller:** New England Boring Contractors / P. Schofield

**Surface Elevation (ft.):** 135.3

**Drilling Method/Casing/Core Barrel Size:** Drive and Wash / 4 in / NA

**Total Depth (ft.):** 20

**Hammer Weight/Drop Height/ Spoon Size:** 140 lb / 30 in / 2 in O.D.

**Depth to Initial Water Level (ft):**

**Bore Hole Location:** See boring location plan

**Depth Date Time**

**Drilling Date: Start:** 9/25/2017 **End:** 9/25/2017

6.1 9/25/2017 10:20 am

**N:** 103227.01 **E:** 1101521.71

**Abandonment Method:** Backfilled with soil cuttings.

**Logged By:** A. Smith

Elev. Depth (ft)	Sample Type	Sample Number	Sample Length (in)	Blows per 6 inches	Sample Recovery (in)	N-Value	Organic Vapor Reading (ppm)	Graphic Log	Strata	Material Description	Remarks
135.3											
0										5" Asphalt	
	SS	S-1	18	16 17 14	11	31	0.0			Moist, dense, brown, fine to coarse SAND, some fine to coarse gravel, little silt	
	SS	S-2	24	6 5 4 3	12	9	0.0			Moist, loose, brown, fine to coarse SAND, some fine to coarse gravel, little silt	
										No Recovery	
130.3	SS	S-3	24	5 5 5 3	0	10	0.0				
5											
	SS	S-4	24	5 1 1 7	14	2	0.0			Top 12": Wet, soft, brown, SILT, trace fine sand, trace fine gravel, trace organics Bottom 2": Wet, medium dense, gray, fine SAND, some silt	
	SS	S-5	24	27 18 9 9	12	27	0.0			Wet, very stiff, gray, Clayey SILT, little fine to medium sand	
125.3											
10											
	SS	S-6	24	11 16 13 7	14	29	0.0			Wet, medium dense, brown, fine to coarse SAND, little fine to coarse gravel, little silt	
120.3											
15											
	SS	S-7	24	10 12 11 14	16	23	0.0			Wet, medium dense, gray, fine SAND, little silt, little fine gravel	
115.3										End of exploration at 20 ft bgs.	

Sample Types				Consistency vs Blowcount/Foot				Burmister Classification	
AS - Auger/Grab Sample	HP - Hydro Punch	Granular (Sand):		Fine Grained (Clay):				and	35-50%
CS - California Sampler	SS - Split Spoon	V. Loose: 0-4	Dense: 30-50	V. Soft: <2	Stiff: 8-15			some	20-35%
BQ - 1.5" Rock Core	ST - Shelby Tube	Loose: 4-10	V. Dense: >50	Soft: 2-4	V. Stiff: 15-30			little	10-20%
NX - 2" Rock Core	WS - Wash Sample	M. Dense: 10-30		M. Stiff: 4-8	Hard: >30			trace	<10%
	GP - Geoprobe							moisture, density, color	

**Reviewed by:** T. Dunn

**Date:** 11/15/2017

**Boring Number:** B-1



# Boring Number: B-2 (MW)

**Client:** Town of Salem  
**Project Location:** Salem, NH

**Project Name:** S. Broadway Watermain Replacement  
**Project Number:** 25972-215427

**Drilling Contractor/Driller:** New England Boring Contractors / B. Raiche

**Surface Elevation (ft.):** 135.0

**Drilling Method/Casing/Core Barrel Size:** Drive and Wash / 4 in / NA

**Total Depth (ft.):** 21

**Hammer Weight/Drop Height/ Spoon Size:** 140 lb / 30 in / 2 in O.D.

**Depth to Initial Water Level (ft):**

**Bore Hole Location:** See boring location plan

**Depth Date Time**

**Drilling Date: Start:** 4/11/2017 **End:** 4/11/2017

7.25 4/11/2017 3:00 pm

**N:** 103106.52 **E:** 1101673.3

**Abandonment Method:** Monitoring Well installed.

**Logged By:** J. Todd

Elev. Depth (ft)	Sample Type	Sample Number	Sample Length (in)	Blows per 6 inches	Sample Recovery (in)	N-Value	Organic Vapor Reading (ppm)	Graphic Log	Strata	Material Description	Remarks
135.0											
0	SS	S-1	24	3 8 5 6	18	13	0.0		Organic Silty Sand	Grass at surface Moist, medium dense, light brown, fine to medium SAND, some silt, trace fine gravel	Composite analytical sample taken from 0 ft to 10 ft bgs.
	SS	S-2	24	6 5 1 1	13	6	0.0			Moist, loose, brown, fine to medium SAND, some silt, trace organic material (wood/roots)	
130.0	SS	S-3	24	1 1 1 1	14	2	0.0			Moist, very loose, dark brown, fine SAND and SILT, trace organic material (wood/roots)	
5	SS	S-4	24	1 1 1 1	15	2	0.0			Moist, very loose, dark brown, fine Organic SAND and SILT	
	SS	S-5	24	3 6 22 14	18	28	0.0		Sand	Moist, medium dense, dark brown, fine to medium SAND, little fine to coarse gravel, trace silt	
125.0											
10									Silty Sand		
	SS	S-6	24	6 12 17 24	16	29	0.0			Moist, medium dense, tan, Clayey SILT, little fine sand	
120.0											
15	SS	S-7	24	10 31	13	52	0.0			Moist, very dense, brown, fine to medium SAND, some silt, trace fine gravel	
115.0											

Sample Types				Consistency vs Blowcount/Foot				Burmister Classification	
AS - Auger/Grab Sample	HP - Hydro Punch	Granular (Sand):		Fine Grained (Clay):				and	35-50%
CS - California Sampler	SS - Split Spoon	V. Loose: 0-4 Dense: 30-50		V. Soft: <2 Stiff: 8-15				some	20-35%
BQ - 1.5" Rock Core	ST - Shelby Tube	Loose: 4-10 V. Dense: >50		Soft: 2-4 V. Stiff: 15-30				little	10-20%
NX - 2" Rock Core	WS - Wash Sample	M. Dense: 10-30		M. Stiff: 4-8 Hard: >30				trace	<10%
	GP - Geoprobe							moisture, density, color	

**Reviewed by:** T. Dunn

**Date:** 11/15/2017

**Boring Number:** B-2 (MW)



# Boring Number: B-2 (MW)

Client: Town of Salem

Project Name: S. Broadway Watermain Replacement

Project Location: Salem, NH

Project Number: 25972-215427

Elev. Depth (ft)	Sample Type	Sample Number	Sample Length (in)	Blows per 6 inches	Sample Recovery (in)	N-Value	Organic Vapor Reading (ppm)	Graphic Log	Strata	Material Description	Remarks
115.0 20	SS	S-7	24	21 18	13	52	0.0				
										End of exploration at 21 ft bgs.	
110.0 25											
105.0 30											
100.0 35											
95.0 40											
90.0 45											

BL S. BROADWAY SALEM NH - BORING LOGS.GPJ - 12/15/17

Boring Number: B-2 (MW)





# Boring Number: B-3 (MW)

**Client:** Town of Salem  
**Project Location:** Salem, NH

**Project Name:** S. Broadway Watermain Replacement  
**Project Number:** 25972-215427

**Drilling Contractor/Driller:** New England Boring Contractors / B. Raiche

**Surface Elevation (ft.):** 133.3

**Drilling Method/Casing/Core Barrel Size:** Drive and Wash / 4 in / NA

**Total Depth (ft.):** 22

**Hammer Weight/Drop Height/ Spoon Size:** 140 lb / 30 in / 2 in O.D.

**Depth to Initial Water Level (ft):**

**Bore Hole Location:** See boring location plan

Depth	Date	Time
5	4/11/2017	11:00 am

**Drilling Date: Start:** 4/11/2017 **End:** 4/11/2017

**Abandonment Method:** Monitoring Well installed.

**N:** 102995.41 **E:** 1101688.44

**Logged By:** J. Todd

Elev. Depth (ft)	Sample Type	Sample Number	Sample Length (in)	Blows per 6 inches	Sample Recovery (in)	N-Value	Organic Vapor Reading (ppm)	Graphic Log	Strata	Material Description	Remarks
133.3											
0	SS	S-1	24	3 6 19 39	7	25	0.0		Gravelly Sand	Moist, medium dense, brown, fine to coarse SAND and fine to coarse GRAVEL, little silt, trace organic material (grass/roots)	Composite analytical sample taken from 0 ft to 10 ft bgs.
	SS	S-2	24	15 11 4 15	2	15	0.0			Dry, medium dense, gray, fine GRAVEL, trace fine to medium sand, trace silt	
128.3	SS	S-3	24	17 8 5 3	15	13	0.0			Moist, medium dense, gray-brown, fine SAND and SILT, trace fine gravel	
5	SS	S-4	24	3 2 4 4	14	6	0.0			Moist, medium stiff, dark gray, Organic Clayey SILT and fine to medium SAND	
	SS	S-5	24	6 7 6 8	<1	13	0.0			Wet, medium dense, dark gray, fine GRAVEL, trace silt	
123.3											
10											
118.3	SS	S-6	24	7 8 14 19	18	22	0.0		Silty Sand	Moist, medium dense, tan, fine SAND, some silt	
15											
113.3											

**Sample Types**

AS - Auger/Grab Sample  
CS - California Sampler  
BQ - 1.5" Rock Core  
NX - 2" Rock Core  
HP - Hydro Punch  
SS - Split Spoon  
ST - Shelby Tube  
WS - Wash Sample  
GP - Geoprobe

**Consistency vs Blowcount/Foot**

Granular (Sand):	
V. Loose: 0-4	Dense: 30-50
Loose: 4-10	V. Dense: >50
M. Dense: 10-30	

Fine Grained (Clay):	
V. Soft: <2	Stiff: 8-15
Soft: 2-4	V. Stiff: 15-30
M. Stiff: 4-8	Hard: >30

**Burmister Classification**

and some	35-50%
little	20-35%
trace	10-20%
moisture, density, color	<10%

**Reviewed by:** T. Dunn

**Date:** 11/15/2017

**Boring Number:** B-3 (MW)



# Boring Number: B-3 (MW)

Client: Town of Salem

Project Name: S. Broadway Watermain Replacement

Project Location: Salem, NH

Project Number: 25972-215427

Elev. Depth (ft)	Sample Type	Sample Number	Sample Length (in)	Blows per 6 inches	Sample Recovery (in)	N-Value	Organic Vapor Reading (ppm)	Graphic Log	Strata	Material Description	Remarks
113.3 20	SS	S-7	24	30 26 11 9	5	37	0.0		Silty Sand	Moist, dense, light brown, fine to medium SAND, little silt	
										End of exploration at 22 ft bgs.	
108.3 25											
103.3 30											
98.3 35											
93.3 40											
88.3 45											

BL S. BROADWAY SALEM NH - BORING LOGS.GPJ - 12/15/17

Boring Number: B-3 (MW)



# Boring Number: B-4 (MW)

**Client:** Town of Salem  
**Project Location:** Salem, NH

**Project Name:** S. Broadway Watermain Replacement  
**Project Number:** 25972-215427

**Drilling Contractor/Driller:** New England Boring Contractors / B. Raiche  
**Drilling Method/Casing/Core Barrel Size:** Drive and Wash / 4 in / NA  
**Hammer Weight/Drop Height/ Spoon Size:** 140 lb / 30 in / 2 in O.D.  
**Bore Hole Location:** See boring location plan  
**Drilling Date: Start:** 4/10/2017 **End:** 4/10/2017  
**N:** 102714.69 **E:** 1101822.78

**Surface Elevation (ft.):** 131.2  
**Total Depth (ft.):** 21  
**Depth to Initial Water Level (ft):**  
**Depth Date Time**  
7.5 4/10/2017 2:30 pm  
**Abandonment Method:** Monitoring Well installed.  
**Logged By:** J. Todd

Elev. Depth (ft)	Sample Type	Sample Number	Sample Length (in)	Blows per 6 inches	Sample Recovery (in)	N-Value	Organic Vapor Reading (ppm)	Graphic Log	Strata	Material Description	Remarks
131.2										5" Asphalt Sidewalk	Composite
0	SS	S-1	18	8 10 12	8	22	0.0		Gravelly Sand	Moist, medium dense, brown, fine to medium SAND, little fine to coarse gravel, little silt	analytical sample taken from 0 ft to 10 ft bgs.
	SS	S-2	13	33 85 100/1"	4	>185	0.0			Moist, very dense, brown, fine to medium SAND, some fine to coarse gravel, trace silt	
126.2 5	SS	S-3	9	29 100/3"	3	>100	0.0			Moist, very dense, brown, fine to coarse GRAVEL, trace medium to coarse sand, trace silt	
	SS	S-4	24	3 26 40 36	5	66	0.0			Moist, very dense, dark gray and tan, fine to coarse SAND, some fine to coarse gravel, trace silt	
121.2 10	SS	S-5	24	8 13 15 14	14	28	0.0		Silty Sand	Moist, very stiff, tan, Clayey SILT, little fine to medium sand	
116.2 15	SS	S-6	24	24 29 27 28	5	56	0.0			Moist, very dense, tan, fine to medium SAND, some silt, little fine gravel	
111.2	SS	S-7	24	25 29	7	55	0.0			Moist, very dense, tan, fine to medium SAND, some silt, trace fine gravel	

Sample Types				Consistency vs Blowcount/Foot				Burmister Classification	
AS - Auger/Grab Sample	HP - Hydro Punch			Granular (Sand):		Fine Grained (Clay):		and	35-50%
CS - California Sampler	SS - Split Spoon			V. Loose: 0-4	Dense: 30-50	V. Soft: <2	Stiff: 8-15	some	20-35%
BQ - 1.5" Rock Core	ST - Shelby Tube			Loose: 4-10	V. Dense: >50	Soft: 2-4	V. Stiff: 15-30	little	10-20%
NX - 2" Rock Core	WS - Wash Sample			M. Dense: 10-30		M. Stiff: 4-8	Hard: >30	trace	<10%
	GP - Geoprobe							moisture, density, color	

**Reviewed by:** T. Dunn

**Date:** 11/15/2017

**Boring Number:** B-4 (MW)



# Boring Number: B-4 (MW)

**Client:** Town of Salem**Project Name:** S. Broadway Watermain Replacement**Project Location:** Salem, NH**Project Number:** 25972-215427

Elev. Depth (ft)	Sample Type	Sample Number	Sample Length (in)	Blows per 6 inches	Sample Recovery (in)	N-Value	Organic Vapor Reading (ppm)	Graphic Log	Strata	Material Description	Remarks
111.2 20	SS	S-7	24	26 67	7	55	0.0				
										End of exploration at 21 ft bgs.	
106.2 25											
101.2 30											
96.2 35											
91.2 40											
86.2 45											

BL S. BROADWAY SALEM NH - BORING LOGS.GPJ - 12/15/17

**Boring Number: B-4 (MW)**



# Boring Number: B-5

**Client:** Town of Salem  
**Project Location:** Salem, NH

**Project Name:** S. Broadway Watermain Replacement  
**Project Number:** 25972-215427

**Drilling Contractor/Driller:** New England Boring Contractors / P. Schofield

**Surface Elevation (ft.):** 130.7

**Drilling Method/Casing/Core Barrel Size:** Drive and Wash / 4 in / NA

**Total Depth (ft.):** 20

**Hammer Weight/Drop Height/ Spoon Size:** 140 lb / 30 in / 2 in O.D.

**Depth to Initial Water Level (ft):**

**Bore Hole Location:** See boring location plan

**Depth Date Time**

**Drilling Date: Start:** 9/26/2017 **End:** 9/26/2017

6.0 9/26/2017 1:45 pm

**N:** 102431.17 **E:** 1101947.62

**Abandonment Method:** Backfilled with soil cuttings.

**Logged By:** A. Smith

Elev. Depth (ft)	Sample Type	Sample Number	Sample Length (in)	Blows per 6 inches	Sample Recovery (in)	N-Value	Organic Vapor Reading (ppm)	Graphic Log	Strata	Material Description	Remarks
130.7										6" Asphalt	
0										6" Subbase: Wet, brown, fine to coarse SAND, little fine gravel, trace silt (possible weathered concrete)	
	SS	S-1	18	25 31 23	10	54	0.0		Fill	Moist, very dense, brown, fine to coarse GRAVEL, some fine to coarse sand, little silt	
	SS	S-2	18	8 5 4	15	9	0.0			Moist, loose, brown, fine SAND, little silt	
										No Recovery	
125.7											
5											
	SS	S-3	24	6 10 13 12	0	23	0.0				
	SS	S-4	24	9 9 7 9	20	16	0.0		Clayey Silt	Wet, very stiff, brown, SILT, trace fine sand	
	SS	S-5	24	5 6 6 9	15	12	0.0			Wet, stiff, brown, SILT & CLAY	PP = 1.25 tsf.
120.7											
10											
	SS	S-6	24	7 14 20 19	16	34	0.0			Wet, hard, brown, SILT, trace fine sand	PP = 2.0 tsf. Sand in tip of split spoon.
115.7											
15											
	SS	S-7	24	15 21 12 11	12	33	0.0		Silty Sand	Wet, dense, gray-brown, fine SAND, some silt	
110.7										End of exploration at 20 ft bgs.	

**Sample Types**

AS - Auger/Grab Sample  
CS - California Sampler  
BQ - 1.5" Rock Core  
NX - 2" Rock Core  
HP - Hydro Punch  
SS - Split Spoon  
ST - Shelby Tube  
WS - Wash Sample  
GP - Geoprobe

**Consistency vs Blowcount/Foot**

**Granular (Sand):**  
V. Loose: 0-4  
Loose: 4-10  
M. Dense: 10-30  
Dense: 30-50  
V. Dense: >50

**Fine Grained (Clay):**  
V. Soft: <2  
Soft: 2-4  
M. Stiff: 4-8  
Stiff: 8-15  
V. Stiff: 15-30  
Hard: >30

**Burmister Classification**

and 35-50%  
some 20-35%  
little 10-20%  
trace <10%  
moisture, density, color

**Reviewed by:** T. Dunn

**Date:** 11/15/2017

**Boring Number:** B-5



# Boring Number: B-6

**Client:** Town of Salem  
**Project Location:** Salem, NH

**Project Name:** S. Broadway Watermain Replacement  
**Project Number:** 25972-215427

**Drilling Contractor/Driller:** New England Boring Contractors / P. Schofield

**Surface Elevation (ft.):** 130.1

**Drilling Method/Casing/Core Barrel Size:** Drive and Wash / 4 in / NA

**Total Depth (ft.):** 21

**Hammer Weight/Drop Height/ Spoon Size:** 140 lb / 30 in / 2 in O.D.

**Depth to Initial Water Level (ft):**

**Bore Hole Location:** See boring location plan

Depth	Date	Time
6.4	9/22/2017	11:00 am

**Drilling Date: Start:** 9/22/2017 **End:** 9/22/2017

**Abandonment Method:** Backfilled with soil cuttings.

**N:** 102139.63 **E:** 1102101.86

**Logged By:** A. Smith

Elev. Depth (ft)	Sample Type	Sample Number	Sample Length (in)	Blows per 6 inches	Sample Recovery (in)	N-Value	Organic Vapor Reading (ppm)	Graphic Log	Strata	Material Description	Remarks
130.1 0										5" Asphalt 7" Concrete	
	SS	S-1	18	16 9 9	4	18	0.0			Moist, medium dense, brown, fine SAND, little silt, trace fine gravel	
	SS	S-2	18	7 6 5	6	11	0.0			Moist, medium dense, brown, fine SAND, little silt, trace fine gravel	
125.1 5	SS	S-3	24	5 7 7 6	10	14	0.0			Wet, medium dense, brown, fine to medium SAND, trace silt, trace fine gravel	
	SS	S-4	24	9 9 12 10	12	21	0.0			Wet, medium dense, brown, fine to medium SAND, trace silt, trace fine gravel	
	SS	S-5	24	4 4 4 5	9	8	0.0			Wet, loose, brown, fine to medium SAND, trace silt, trace fine gravel	
120.1 10											
115.1 15	SS	S-6	24	2 3 2 2	18	5	0.0			Wet, loose, brown, fine SAND, little silt	
110.1	SS	S-7	24	4 5	18	11	0.0			Wet, medium dense, brown, fine SAND, little silt	

Sample Types				Consistency vs Blowcount/Foot				Burmister Classification	
AS - Auger/Grab Sample	HP - Hydro Punch	Granular (Sand):		Fine Grained (Clay):				and	35-50%
CS - California Sampler	SS - Split Spoon	V. Loose: 0-4	Dense: 30-50	V. Soft: <2	Stiff: 8-15			some	20-35%
BQ - 1.5" Rock Core	ST - Shelby Tube	Loose: 4-10	V. Dense: >50	Soft: 2-4	V. Stiff: 15-30			little	10-20%
NX - 2" Rock Core	WS - Wash Sample	M. Dense: 10-30		M. Stiff: 4-8	Hard: >30			trace	<10%
	GP - Geoprobe							moisture, density, color	

**Reviewed by:** T. Dunn


**Date:** 11/15/2017

**Boring Number:** B-6



# Boring Number: B-6

**Client:** Town of Salem**Project Name:** S. Broadway Watermain Replacement**Project Location:** Salem, NH**Project Number:** 25972-215427

Elev. Depth (ft)	Sample Type	Sample Number	Sample Length (in)	Blows per 6 inches	Sample Recovery (in)	N-Value	Organic Vapor Reading (ppm)	Graphic Log	Strata	Material Description	Remarks
110.1 20	SS	S-7	24	6 7	18	11	0.0		Sand		
										End of exploration at 21 ft bgs.	
105.1 25											
100.1 30											
95.1 35											
90.1 40											
85.1 45											

BL S. BROADWAY SALEM NH - BORING LOGS.GPJ - 12/15/17

**Boring Number: B-6**



# Boring Number: B-7

**Client:** Town of Salem  
**Project Location:** Salem, NH

**Project Name:** S. Broadway Watermain Replacement  
**Project Number:** 25972-215427

**Drilling Contractor/Driller:** New England Boring Contractors / P. Schofield

**Surface Elevation (ft.):** 129.2

**Drilling Method/Casing/Core Barrel Size:** Drive and Wash / 4 in / NA

**Total Depth (ft.):** 20

**Hammer Weight/Drop Height/ Spoon Size:** 140 lb / 30 in / 2 in O.D.

**Depth to Initial Water Level (ft):**

**Bore Hole Location:** See boring location plan

**Depth Date Time**

**Drilling Date: Start:** 9/21/2017 **End:** 9/21/2017

5.7 9/21/2017 2:29 pm

**N:** 101917.13 **E:** 1102237.21

**Abandonment Method:** Backfilled with soil cuttings.

**Logged By:** J. Todd

Elev. Depth (ft)	Sample Type	Sample Number	Sample Length (in)	Blows per 6 inches	Sample Recovery (in)	N-Value	Organic Vapor Reading (ppm)	Graphic Log	Strata	Material Description	Remarks
129.2										7" Asphalt	
0										5" Concrete	
	SS	S-1	18	15 28 17	10	45	0.0			Moist, dense, dark brown to black, fine to coarse SAND, little fine gravel, trace silt	
	SS	S-2	18	14 14 13	12	27	0.0			Moist, medium dense, brown, fine to medium SAND, little silt	
124.2	SS	S-3	24	7 4 4 6	8	8	0.0			Moist, loose, brown, fine to coarse SAND, trace silt	
5	SS	S-4	24	7 7 8 8	10	15	0.0			Moist, medium dense, brown, fine SAND, trace silt	
	SS	S-5	24	3 4 4 4	7	8	0.0			Moist, loose, brown, fine SAND, trace silt	
119.2											
10											
	SS	S-6	24	3 3 4 4	14	7	0.0			Moist, loose, brown, fine SAND, trace silt	
114.2											
15											
	SS	S-7	24	4 6 7 9	16	13	0.0			Moist, medium dense, brown, fine to medium SAND, some silt	
109.2										End of exploration at 20 ft bgs.	

Sample Types				Consistency vs Blowcount/Foot				Burmister Classification	
AS - Auger/Grab Sample	HP - Hydro Punch	Granular (Sand):		Fine Grained (Clay):				and	35-50%
CS - California Sampler	SS - Split Spoon	V. Loose: 0-4	Dense: 30-50	V. Soft: <2	Stiff: 8-15			some	20-35%
BQ - 1.5" Rock Core	ST - Shelby Tube	Loose: 4-10	V. Dense: >50	Soft: 2-4	V. Stiff: 15-30			little	10-20%
NX - 2" Rock Core	WS - Wash Sample	M. Dense: 10-30		M. Stiff: 4-8	Hard: >30			trace	<10%
	GP - Geoprobe							moisture, density, color	

**Reviewed by:** T. Dunn

**Date:** 11/15/2017

**Boring Number:** B-7

BL S. BROADWAY SALEM NH - BORING LOGS.GPJ - 12/15/17





# Boring Number: B-8

**Client:** Town of Salem  
**Project Location:** Salem, NH

**Project Name:** S. Broadway Watermain Replacement  
**Project Number:** 25972-215427

**Drilling Contractor/Driller:** New England Boring Contractors / P. Schofield

**Surface Elevation (ft.):** 128.1

**Drilling Method/Casing/Core Barrel Size:** Drive and Wash / 4 in / NA

**Total Depth (ft.):** 40

**Hammer Weight/Drop Height/ Spoon Size:** 140 lb / 30 in / 2 in O.D.

**Depth to Initial Water Level (ft):**

**Bore Hole Location:** See boring location plan

Depth	Date	Time
7	9/21/2017	11:52 am

**Drilling Date: Start:** 9/21/2017 **End:** 9/21/2017

**Abandonment Method:** Backfilled with soil cuttings.

**N:** 101781.74 **E:** 1102288.54

**Logged By:** J. Todd

Elev. Depth (ft)	Sample Type	Sample Number	Sample Length (in)	Blows per 6 inches	Sample Recovery (in)	N-Value	Organic Vapor Reading (ppm)	Graphic Log	Strata	Material Description	Remarks
128.1 0										7" Asphalt	
										5" Concrete	
	SS	S-1	18	19 18 12	7	30	0.0			Moist, dense, brown, fine to medium SAND, little fine to coarse gravel, trace silt	
	SS	S-2	18	10 11 13	13	24	0.0			Moist, medium dense, brown, fine to medium SAND, trace silt	
123.1 5	SS	S-3	24	9 7 8 9	6	15	0.0			Moist, medium dense, brown, fine to medium SAND, trace silt	
	SS	S-4	24	10 7 14 15	18	21	0.0			Moist, medium dense, brown, fine to medium SAND, little silt	
	SS	S-5	24	6 6 7 7	11	13	0.0			Moist, medium dense, light brown, fine to medium SAND, little silt	
118.1 10											
113.1 15	SS	S-6	24	2 3 3 3	1	6	0.0			Moist, loose, gray, fine to medium SAND, some silt	
108.1	SS	S-7	24	3 3	12	7	0.0			Moist, loose, gray, fine to medium SAND, some silt	

Sample Types				Consistency vs Blowcount/Foot				Burmister Classification	
AS - Auger/Grab Sample	HP - Hydro Punch	Granular (Sand):		Fine Grained (Clay):				and	35-50%
CS - California Sampler	SS - Split Spoon	V. Loose: 0-4	Dense: 30-50	V. Soft: <2	Stiff: 8-15			some	20-35%
BQ - 1.5" Rock Core	ST - Shelby Tube	Loose: 4-10	V. Dense: >50	Soft: 2-4	V. Stiff: 15-30			little	10-20%
NX - 2" Rock Core	WS - Wash Sample	M. Dense: 10-30		M. Stiff: 4-8	Hard: >30			trace	<10%
	GP - Geoprobe							moisture, density, color	

**Reviewed by:** T. Dunn

**Date:** 11/15/2017

**Boring Number:** B-8

BL S. BROADWAY SALEM NH - BORING LOGS.GPJ - 12/15/17



# Boring Number: B-8

**Client:** Town of Salem**Project Name:** S. Broadway Watermain Replacement**Project Location:** Salem, NH**Project Number:** 25972-215427

Elev. Depth (ft)	Sample Type	Sample Number	Sample Length (in)	Blows per 6 inches	Sample Recovery (in)	N-Value	Organic Vapor Reading (ppm)	Graphic Log	Strata	Material Description	Remarks
108.1 20	SS	S-7	24	4 4	12	7	0.0		Silty Sand	Moist, stiff, light brown, Clayey SILT, some fine sand	
103.1 25	SS	S-8	24	2 4 5 6	15	9	0.0				
									Silty Sand	Moist, medium dense, light brown, fine SAND, some silt	
98.1 30	SS	S-9	24	4 8 11 12	18	19	0.0				
									Silty Sand	Moist, medium dense, light brown, fine SAND and SILT	
93.1 35	SS	S-10	24	7 11 14 17	18	25	0.0				
									Silty Sand	Moist, dense, light brown, fine SAND, some silt	
88.1 40	SS	S-11	24	4 14 31 22	15	45	0.0		Silty Sand	Moist, dense, light brown, fine SAND, some silt	
									Silty Sand	End of exploration at 40 ft bgs.	
83.1 45									Silty Sand	End of exploration at 40 ft bgs.	

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**Boring Number: B-8**



# Boring Number: B-9

**Client:** Town of Salem  
**Project Location:** Salem, NH

**Project Name:** S. Broadway Watermain Replacement  
**Project Number:** 25972-215427

**Drilling Contractor/Driller:** New England Boring Contractors / P. Schofield

**Surface Elevation (ft.):** 129.1

**Drilling Method/Casing/Core Barrel Size:** Drive and Wash / 4 in / NA

**Total Depth (ft.):** 20

**Hammer Weight/Drop Height/ Spoon Size:** 140 lb / 30 in / 2 in O.D.

**Depth to Initial Water Level (ft):**

**Bore Hole Location:** See boring location plan

**Depth Date Time**

**Drilling Date: Start:** 9/27/2017 **End:** 9/27/2017

4.9 9/27/2017 2:45 pm

**N:** 101485.22 **E:** 1102460.87

**Abandonment Method:** Backfilled with soil cuttings.

**Logged By:** A. Smith

Elev. Depth (ft)	Sample Type	Sample Number	Sample Length (in)	Blows per 6 inches	Sample Recovery (in)	N-Value	Organic Vapor Reading (ppm)	Graphic Log	Strata	Material Description	Remarks
129.1 0										8" Asphalt	
	SS	S-1	18	15 9 8	12	17	0.0		Sand	Top 2": Moist, medium dense, gray, fine to medium SAND, some silt, trace fine gravel Bottom 10": Moist, medium dense, brown, fine SAND, little silt	
	SS	S-2	18	5 6 6	10	12	0.0			Moist, medium dense, brown, fine SAND, little silt	
124.1 5	SS	S-3	24	4 4 5 6	12	9	0.0		Silty Sand	Wet, stiff, brown, SILT and fine to medium SAND	
	SS	S-4	24	4 4 3 3	12	7	0.0			Wet, medium stiff, brown, SILT and fine to medium SAND	
	SS	S-5	24	3 1 1 2	9	2	0.0			Wet, soft, brown, SILT and fine to medium SAND	
119.1 10											
114.1 15	SS	S-6	24	3 5 6 6	18	11	0.0		Clayey Silt	Wet, stiff, gray-brown, SILT, little fine sand	
	SS	S-7	24	3 4 6 7	18	10	0.0			Wet, stiff, gray-brown, SILT, little fine sand	
109.1										End of exploration at 20 ft bgs.	

**Sample Types**

AS - Auger/Grab Sample  
CS - California Sampler  
BQ - 1.5" Rock Core  
NX - 2" Rock Core  
HP - Hydro Punch  
SS - Split Spoon  
ST - Shelby Tube  
WS - Wash Sample  
GP - Geoprobe

**Consistency vs Blowcount/Foot**

**Granular (Sand):**  
V. Loose: 0-4 Dense: 30-50  
Loose: 4-10 V. Dense: >50  
M. Dense: 10-30

**Fine Grained (Clay):**  
V. Soft: <2 Stiff: 8-15  
Soft: 2-4 V. Stiff: 15-30  
M. Stiff: 4-8 Hard: >30

**Burmister Classification**

and 35-50%  
some 20-35%  
little 10-20%  
trace <10%  
moisture, density, color

**Reviewed by:** T. Dunn

**Date:** 11/15/2017

**Boring Number:** B-9



# Boring Number: B-10

**Client:** Town of Salem  
**Project Location:** Salem, NH

**Project Name:** S. Broadway Watermain Replacement  
**Project Number:** 25972-215427

**Drilling Contractor/Driller:** New England Boring Contractors / P. Schofield

**Surface Elevation (ft.):** 126.1

**Drilling Method/Casing/Core Barrel Size:** Drive and Wash / 4 in / NA

**Total Depth (ft.):** 8

**Hammer Weight/Drop Height/ Spoon Size:** 140 lb / 30 in / 2 in O.D.

**Depth to Initial Water Level (ft):**

**Bore Hole Location:** See boring location plan

**Depth Date Time**

**Drilling Date: Start:** 10/4/2017 **End:** 10/4/2017

3.5 10/4/2017 2:45 pm

**N:** 101192.61 **E:** 1102615.18

**Abandonment Method:** Backfilled with soil cuttings.

**Logged By:** A. Smith

Elev. Depth (ft)	Sample Type	Sample Number	Sample Length (in)	Blows per 6 inches	Sample Recovery (in)	N-Value	Organic Vapor Reading (ppm)	Graphic Log	Strata	Material Description	Remarks
126.1 0										8" Asphalt	
	SS	S-1	18	10 12 13	14	25	0.0			Moist, medium dense, brown, fine to medium SAND, some fine gravel, little silt	Subbase: Moist, gray, coarse GRAVEL, trace fine to coarse sand, trace silt
	SS	S-2	18	8 10 7	10	17	0.0			Moist, medium dense, brown, fine to medium SAND, some fine gravel, little silt, trace organics Wet, stiff, dark brown, Slightly Organic SILT	
121.1 5	SS	S-3	24	9 6 3 2	10	9	0.0				
	SS	S-4	24	7 11 14 15	14	25	0.0			Top 6": Wet, medium dense, brown, fine SAND, trace silt Bottom 8": Wet, very stiff, gray-brown, SILT	An obstruction was encountered at 6 ft bgs while driving the casing to 8 ft bgs. Metal pieces were coming up in the drilling wash water. The casing was pulled out and a smooth 2 inch groove was observed inside of the soil. The driller terminated the boring at 8 ft bgs.
116.1 10										End of exploration at 8 ft bgs.	
111.1 15											
106.1											

**Sample Types****Consistency vs Blowcount/Foot****Burmister Classification**

AS - Auger/Grab Sample  
CS - California Sampler  
BQ - 1.5" Rock Core  
NX - 2" Rock Core

HP - Hydro Punch  
SS - Split Spoon  
ST - Shelby Tube  
WS - Wash Sample  
GP - Geoprobe

**Granular (Sand):**  
V. Loose: 0-4 Dense: 30-50  
Loose: 4-10 V. Dense: >50  
M. Dense: 10-30

**Fine Grained (Clay):**  
V. Soft: <2 Stiff: 8-15  
Soft: 2-4 V. Stiff: 15-30  
M. Stiff: 4-8 Hard: >30

and 35-50%  
some 20-35%  
little 10-20%  
trace <10%  
moisture, density, color

**Reviewed by:** T. Dunn

**Date:** 11/15/2017

**Boring Number:** B-10

**Boring Number:  
B-11 (MW)**

**Client:** Town of Salem

**Project Name:** S. Broadway Watermain Replacement

**Project Location:** Salem, NH

**Project Number:** 25972-215427

**Drilling Contractor/Driller:** New England Boring Contractors / B. Raiche

**Surface Elevation (ft.):** 126.4

**Drilling Method/Casing/Core Barrel Size:** Drive and Wash / 4 in / NA

**Total Depth (ft.): 19**

**Hammer Weight/Drop Height/ Spoon Size:** 140 lb / 30 in / 2 in O.D.

**Depth to Initial Water Level (ft):**

**Bore Hole Location:** See boring location plan

Depth	Date	Time
-------	------	------



**Drilling Date: Start: 4/12/2017 End: 4/13/2017**

3.1 4/13/2017 9:30 am

**N: 100804.4    E: 1102835.99**

**Abandonment Method:** Monitoring Well installed.

**Logged By:** J. Todd

Elev. Depth (ft)	Sample Type	Sample Number	Sample Length (in)	Blows per 6 inches	Sample Recovery (in)	N-Value	Organic Vapor Reading (ppm)	Graphic Log	Strata	Material Description	Remarks
126.4 0										6" Asphalt	Composite
	SS	S-1	18	11 7 5	12	12	0.0		Sand	Moist, medium dense, brown, fine to medium SAND, some silt, trace fine gravel	analytical sample taken from 0 ft to 10 ft bgs.
	SS	S-2	24	6 6 6 8	18	12	0.0			Moist, medium dense, light brown, fine to coarse SAND, little silt, trace fine gravel	
121.4 5	SS	S-3	24	2 7 10 9	15	17	0.0			Moist, medium dense, light brown, fine to coarse SAND, little silt, trace fine gravel	
	SS	S-4	24	4 4 4 15	24	8	0.0			Moist, loose, light brown, fine to coarse SAND, little fine gravel, trace silt	
	SS	S-5	23	14 12 20 80/5"	23	42	0.0			Moist, dense, light brown, fine to medium SAND, little fine to coarse gravel, trace silt	
116.4 10											
111.4 15	SS	S-6	24	7 3 4 3	12	7	0.0			Moist, loose, light brown, fine to coarse SAND, little silt, trace fine gravel	Roller bit to 19 ft bgs. No sample taken at 19 ft bgs. Repeated attempts were made at getting the sampler to the correct depth, but the casing would be filled with soil to 17 ft by the
											time the sampler was placed.
106.4										End of exploration at 19 ft bgs.	

<b><u>Sample Types</u></b>		<b><u>Consistency vs Blowcount/Foot</u></b>						<b><u>Burmister Classification</u></b>	
AS - Auger/Grab Sample	HP - Hydro Punch	<b><u>Granular (Sand):</u></b>				<b><u>Fine Grained (Clay):</u></b>		and	35-50%
CS - California Sampler	SS - Split Spoon	V. Loose:	0-4	Dense:	30-50	V. Soft:	<2	Stiff:	8-15
BQ - 1.5" Rock Core	ST - Shelby Tube	Loose:	4-10	V. Dense:	>50	Soft:	2-4	V. Stiff:	15-30
NX - 2" Rock Core	WS - Wash Sample	M. Dense:	10-30			M. Stiff:	4-8	Hard:	>30
	GP - Geoprobe								
									moisture density color

Reviewed by: T. Dunn

Date: 11/15/2017

**Boring Number: B-11 (MW)**

33 S. BROADWAY SALEM NH - BORING LOGS.GPJ - 12/15/17



# Boring Number: B-11 (MW)

**Client:** Town of Salem**Project Name:** S. Broadway Watermain Replacement**Project Location:** Salem, NH**Project Number:** 25972-215427

JL S. BROADWAY SALEM NH - BORING LOGS.GPJ - 12/15/17	Elev. Depth (ft)	Sample Type	Sample Number	Sample Length (in)	Blows per 6 inches	Sample Recovery (in)	N-Value	Organic Vapor Reading (ppm)	Graphic Log	Strata	Material Description	Remarks
	106.4 20											Monitoring well placed at 19 ft bgs.
	101.4 25											
	96.4 30											
	91.4 35											
	86.4 40											
	81.4 45											
Boring Number: B-11 (MW)												



# Boring Number: B-12

**Client:** Town of Salem  
**Project Location:** Salem, NH

**Project Name:** S. Broadway Watermain Replacement  
**Project Number:** 25972-215427

**Drilling Contractor/Driller:** New England Boring Contractors / P. Schofield

**Surface Elevation (ft.):** 126.1

**Drilling Method/Casing/Core Barrel Size:** Drive and Wash / 4 in / NA

**Total Depth (ft.):** 25

**Hammer Weight/Drop Height/ Spoon Size:** 140 lb / 30 in / 2 in O.D.

**Depth to Initial Water Level (ft):**

**Bore Hole Location:** See boring location plan

Depth	Date	Time
5.0	9/26/2017	10:15 am

**Drilling Date: Start:** 9/26/2017 **End:** 9/26/2017

**Abandonment Method:** Backfilled with soil cuttings.

**N:** 100545.41 **E:** 1102949.53

**Logged By:** A. Smith

Elev. Depth (ft)	Sample Type	Sample Number	Sample Length (in)	Blows per 6 inches	Sample Recovery (in)	N-Value	Organic Vapor Reading (ppm)	Graphic Log	Strata	Material Description	Remarks
126.1 0										10" Asphalt	
	SS	S-1	18	26 28 22	4	50	0.1			Wet, dense, brown, fine to coarse SAND, little fine gravel, trace silt	
	SS	S-2	18	7 12 7	6	19	0.0			Wet, medium dense, brown, fine to coarse SAND, little fine gravel, trace silt	
124.1 5	SS	S-3	24	5 1 1 5	3	2	0.0			Wet, very loose, brown, fine to coarse SAND, little fine gravel, trace silt	
	SS	S-4	24	5 4 4 3	12	8	0.0			Top 9": Wet, loose, brown, fine to coarse SAND, little fine gravel, trace silt Bottom 3": Wet, loose, brown, fine SAND, little silt, trace organics	
	SS	S-5	24	5 5 6 5	8	11	0.0			Wet, medium dense, gray-brown, fine SAND, trace silt	
116.1 10											
111.1 15	SS	S-6	24	4 4 3 3	12	7	0.0			Wet, loose, brown-orange to brown, fine SAND, little silt	
106.1	SS	S-7	24	3 4	14	8	0.0			Wet, loose, brown-orange to brown, fine SAND, little silt	

Sample Types				Consistency vs Blowcount/Foot				Burmister Classification	
AS - Auger/Grab Sample	HP - Hydro Punch	Granular (Sand):		Fine Grained (Clay):				and	35-50%
CS - California Sampler	SS - Split Spoon	V. Loose: 0-4	Dense: 30-50	V. Soft: <2	Stiff: 8-15	Soft: 2-4	V. Stiff: 15-30	some	20-35%
BQ - 1.5" Rock Core	ST - Shelby Tube	Loose: 4-10	V. Dense: >50	Soft: 2-4	V. Stiff: 15-30	M. Stiff: 4-8	Hard: >30	little	10-20%
NX - 2" Rock Core	WS - Wash Sample	M. Dense: 10-30						trace	<10%
	GP - Geoprobe							moisture, density, color	

**Reviewed by:** T. Dunn

**Date:** 11/15/2017

**Boring Number:** B-12



# Boring Number: B-12

**Client:** Town of Salem**Project Name:** S. Broadway Watermain Replacement**Project Location:** Salem, NH**Project Number:** 25972-215427

Elev. Depth (ft)	Sample Type	Sample Number	Sample Length (in)	Blows per 6 inches	Sample Recovery (in)	N-Value	Organic Vapor Reading (ppm)	Graphic Log	Strata	Material Description	Remarks
106.1 20	SS	S-7	24	4 4	14	8	0.0		Sand	Wet, medium dense, brown-orange to brown, fine SAND, little silt	
	SS	S-8	24	3 5 7 9	24	12	0.0				
101.1 25										End of exploration at 25 ft bgs.	
96.1 30											
91.1 35											
86.1 40											
81.1 45											

**Boring Number: B-12**





# Boring Number: B-13

**Client:** Town of Salem  
**Project Location:** Salem, NH

**Project Name:** S. Broadway Watermain Replacement  
**Project Number:** 25972-215427

**Drilling Contractor/Driller:** New England Boring Contractors / P. Schofield

**Surface Elevation (ft.):** 125.2

**Drilling Method/Casing/Core Barrel Size:** Drive and Wash / 4 in / NA

**Total Depth (ft.):** 28

**Hammer Weight/Drop Height/ Spoon Size:** 140 lb / 30 in / 2 in O.D.

**Depth to Initial Water Level (ft):**

**Bore Hole Location:** See boring location plan

Depth	Date	Time
4	9/20/2017	12:00 pm

**Drilling Date: Start:** 9/19/2017 **End:** 9/20/2017

**Abandonment Method:** Backfilled with soil cuttings.

**N:** 100251.5 **E:** 1103092.07

**Logged By:** J. Todd

Elev. Depth (ft)	Sample Type	Sample Number	Sample Length (in)	Blows per 6 inches	Sample Recovery (in)	N-Value	Organic Vapor Reading (ppm)	Graphic Log	Strata	Material Description	Remarks
125.2										7" Asphalt	
0										5" Concrete	
	SS	S-1	18	11 23 10	12	33	0.0		Gravelly Sand	Moist, dense, gray, fine GRAVEL, trace fine to coarse sand, trace silt	
	SS	S-2	18	12 13 15	14	28	0.0		Gravelly Sand	Moist, medium dense, gray-brown, fine to medium SAND, little silt, trace fine gravel	
	SS	S-3	24	8 5 3 3	6	8	0.0		Sand	Moist, loose, dark gray, fine to medium SAND, some silt	Organic odor
120.2 5	SS	S-4	24	7 10 10 9	12	20	0.0		Sand	Moist, medium dense, light brown, Slightly Organic fine to medium SAND, trace silt	Organic odor
	SS	S-5	24	4 5 7 6	13	12	0.0		Sand	Moist, medium dense, light brown, fine to medium SAND, trace silt, trace fine gravel	
115.2 10											
	SS	S-6	24	4 7 8 9	0	15	0.0		Gravelly Sand	No Recovery	
110.2 15											
	SS	S-7	24	4 5	3	9	0.0		Gravelly Sand	Moist, loose, gray, fine GRAVEL, trace fine sand, trace silt	Rig chatter was observed approximately 18 ft to 19 ft bgs.
105.2											

Sample Types				Consistency vs Blowcount/Foot				Burmister Classification	
AS - Auger/Grab Sample	HP - Hydro Punch	Granular (Sand):		Fine Grained (Clay):				and	35-50%
CS - California Sampler	SS - Split Spoon	V. Loose: 0-4	Dense: 30-50	V. Soft: <2	Stiff: 8-15			some	20-35%
BQ - 1.5" Rock Core	ST - Shelby Tube	Loose: 4-10	V. Dense: >50	Soft: 2-4	V. Stiff: 15-30			little	10-20%
NX - 2" Rock Core	WS - Wash Sample	M. Dense: 10-30		M. Stiff: 4-8	Hard: >30			trace	<10%
	GP - Geoprobe							moisture, density, color	

**Reviewed by:** T. Dunn

**Date:** 11/15/2017

**Boring Number:** B-13






# Boring Number: B-13

Client: Town of Salem

Project Name: S. Broadway Watermain Replacement

Project Location: Salem, NH

Project Number: 25972-215427

Elev. Depth (ft)	Sample Type	Sample Number	Sample Length (in)	Blows per 6 inches	Sample Recovery (in)	N-Value	Organic Vapor Reading (ppm)	Graphic Log	Strata	Material Description	Remarks
105.2 20	SS	S-7	24	4 6	3	9	0.0		Sand		
100.2 25	SS	S-8	11	80 100/5"	7	>100	0.0			Moist, very dense, gray, fine to medium SAND, some silt, trace fine gravel	Very dense material was encountered at approximately 22 ft bgs while advancing casing.
	SS	S-9	3	100/3"	3	>100	0.0			Moist, very dense, gray, fine to coarse SAND, little silt, trace fine gravel	
95.2 30										Casing broke in test boring and could not be recovered, boring was terminated at 28 ft bgs, and relocated to boring B-13A.	
90.2 35											
85.2 40											
80.2 45											

BL S. BROADWAY SALEM NH - BORING LOGS.GPJ - 12/15/17

Boring Number: B-13



# Boring Number: B-13A

**Client:** Town of Salem**Project Name:** S. Broadway Watermain Replacement**Project Location:** Salem, NH**Project Number:** 25972-215427**Drilling Contractor/Driller:** New England Boring Contractors / P. Schofield**Surface Elevation (ft.):** 125.2**Drilling Method/Casing/Core Barrel Size:** Drive and Wash / 4 in / NA**Total Depth (ft.):** 33**Hammer Weight/Drop Height/ Spoon Size:** 140 lb / 30 in / 2 in O.D.**Depth to Initial Water Level (ft):****Bore Hole Location:** See boring location plan**Depth Date Time****Drilling Date: Start:** 9/20/2017 **End:** 9/20/2017

8 9/20/2017 2:00 pm

**N:** 100253.32 **E:** 1103093.37**Abandonment Method:** Backfilled with soil cuttings.**Logged By:** J. Todd


Elev. Depth (ft)	Sample Type	Sample Number	Sample Length (in)	Blows per 6 inches	Sample Recovery (in)	N-Value	Organic Vapor Reading (ppm)	Graphic Log	Strata	Material Description	Remarks
125.2 0											
120.2 5											
115.2 10											
110.2 15											
105.2											
<b>Sample Types</b>						<b>Consistency vs Blowcount/Foot</b>				<b>Burmister Classification</b>	
AS - Auger/Grab Sample CS - California Sampler BQ - 1.5" Rock Core NX - 2" Rock Core						Granular (Sand): V. Loose: 0-4 Dense: 30-50 Loose: 4-10 V. Dense: >50 M. Dense: 10-30				Fine Grained (Clay): V. Soft: <2 Stiff: 8-15 Soft: 2-4 V. Stiff: 15-30 M. Stiff: 4-8 Hard: >30	
HP - Hydro Punch SS - Split Spoon ST - Shelby Tube WS - Wash Sample GP - Geoprobe										and 35-50% some 20-35% little 10-20% trace <10% moisture, density, color	
<b>Reviewed by:</b> T. Dunn									<b>Date:</b> 11/15/2017		<b>Boring Number:</b> B-13A

BL S. BROADWAY SALEM NH - BORING LOGS.GPJ - 12/15/17



# Boring Number: B-13A

**Client:** Town of Salem**Project Name:** S. Broadway Watermain Replacement**Project Location:** Salem, NH**Project Number:** 25972-215427

Elev. Depth (ft)	Sample Type	Sample Number	Sample Length (in)	Blows per 6 inches	Sample Recovery (in)	N-Value	Organic Vapor Reading (ppm)	Graphic Log	Strata	Material Description	Remarks
105.2 20											
100.2 25											
95.2 30	NX	C-1	60		34	--			Bedrock	See core log for description	Casing refusal at 28 ft bgs, attempt rock core.
90.2 35										End of exploration at 33 ft bgs.	
85.2 40											
80.2 45											

BL S. BROADWAY SALEM NH - BORING LOGS.GPJ - 12/15/17

**Boring Number: B-13A**




# Boring Number: B-13A

Client: Town of Salem

Project Name: S. Broadway Watermain Replacement

Project Location: Salem, NH

Project Number: 25972-215427

Elevation Depth (ft)	Sample Type	Sample Number	Sample Length (in)	Recovery (%)	RQD (%)	Drill Rate (min/ft)	Down Press. (psi)	Graphic Log	Strata	Material Description	Remarks								
95.2 30.0	NX	C-1	60	57	38	4:36 4:19 6:30 5:07 5:10			Granofels	Hard, fresh, dark gray, fine to medium grained GRANOFELS; Primary Joint Set horizontal, smooth, planar, fresh, tight.	Drilled to 28 ft bgs, from ground surface. Top of bedrock encountered at approximately 28 ft bgs.								
90.2 35.0										End of exploration at 33 ft bgs.									
85.2 40.0																			
80.2 45.0																			
Bedding (mm)				Joint Spacing (mm)				Continuity (mm)		Attitude Angle		Aperture (mm)							
Extremely Thin		<20		Extremely Close		<20		Extremely		<25		Horizontal		0° - 5°		Very Tight		< 0.1	
Very Thin		20-60		Very Close		20-60		Moderately		25-100		Shallow		5° - 35°		Tight		0.1 - 0.25	
Thin		60-200		Close		60-200		Slightly		100-200		Moderate		35° - 55°		Partly Open		0.25 - 0.5	
Medium		200-600		Mod Close		200-600		Sound		>200		Steep		55° - 85°		Open		0.5 - 2.5	
Thick		600-2000		Wide		600-2000						Vertical		85° - 90°		Mod. Wide		2.5 - 10	
Very Thick		2000-6000		Very Wide		2000-6000										Wide		>10	
Extremely Thick		>6000		Extremely Wide		>6000													
Field Hardness				Weathering															
Very Hard		Knife Can't Scratch		Fresh		No Visible sign of rock material weathering; slight to no discoloration.													
Hard		Scratches with Difficulty		Slight		Discoloration indicated weathering. All the rock material may be discolored and may be weaker externally than its fresh condition.													
Med. Hard		Scratches Readily		Moderate		Less than half of the rock material is decomposed and/or disintegrated to a soil. Fresh or discolored rock is present either as a continuous framework or as corestones.													
Medium		Grooves with Difficulty		Severe		More than half of the rock material is decomposed and/or disintegrated to a soil. Fresh or discolored rock is present either as a continuous framework or as corestone.													
Soft		Grooves Readily		Complete		All rock material is decomposed and/or disintegrated to soil. The original mass structure is largely intact.													
Very Soft		Carves with Knife		Residual Soil		All rock material is converted to soil. The mass structure and material fabric are destroyed. There is a large change in volume, but the soil has not been significantly transported.													
Reviewed by: T. Dunn										Date: 11/15/2017		Boring Number: B-13A							

ROCK CORING LOG S. BROADWAY SALEM NH - BORING LOGS.GPJ - 12/14/17





# Boring Number: B-14

**Client:** Town of Salem**Project Name:** S. Broadway Watermain Replacement**Project Location:** Salem, NH**Project Number:** 25972-215427

Elev. Depth (ft)	Sample Type	Sample Number	Sample Length (in)	Blows per 6 inches	Sample Recovery (in)	N-Value	Organic Vapor Reading (ppm)	Graphic Log	Strata	Material Description	Remarks
105.9 20	SS	S-7	24	10 10	18	19	0.0		Clayey Silt		PP = 1.5 tsf
	SS	S-8	24	6 10 8 8	18	18	0.0			Wet, very stiff, gray, Silty CLAY	PP = 0.25 tsf
100.9 25										End of exploration at 25 ft bgs.	
95.9 30											
90.9 35											
85.9 40											
80.9 45											

**Boring Number: B-14**



# Boring Number: B-15 (MW)

**Client:** Town of Salem**Project Name:** S. Broadway Watermain Replacement**Project Location:** Salem, NH**Project Number:** 25972-215427**Drilling Contractor/Driller:** New England Boring Contractors / B. Raiche**Surface Elevation (ft.):** 125.9**Drilling Method/Casing/Core Barrel Size:** Drive and Wash / 4 in / NA**Total Depth (ft.):** 19.25**Hammer Weight/Drop Height/ Spoon Size:** 140 lb / 30 in / 2 in O.D.**Depth to Initial Water Level (ft):****Bore Hole Location:** See boring location plan

Depth	Date	Time
5	4/13/2017	10:00 am

**Drilling Date: Start:** 4/13/2017 **End:** 4/13/2017**Abandonment Method:** Monitoring Well installed.**N:** 99616.94 **E:** 1103471.51**Logged By:** J. Todd

Elev. Depth (ft)	Sample Type	Sample Number	Sample Length (in)	Blows per 6 inches	Sample Recovery (in)	N-Value	Organic Vapor Reading (ppm)	Graphic Log	Strata	Material Description	Remarks
125.9 0										3" Asphalt Sidewalk	Test boring hand cleared from ground surface to 5 ft bgs. Composite analytical sample taken from 4 ft to 5 ft bgs. From 4 ft to 16 ft bgs: Chemical odor in soil samples.
	AS	A-1	60				0.0			Moist, brown, fine to coarse SAND, some fine to coarse gravel, little silt, occasional cobbles.	
120.9 5	SS	S-1	24	2 4 5 11	13	9	0.0			Moist, stiff, gray-brown, SILT and fine SAND	
	SS	S-2	24	13 8 8 11	16	16	0.0			Moist, medium dense, gray-brown, fine SAND, some silt	
115.9 10	SS	S-3	24	11 10 9 13	8	19	0.0			Moist, medium dense, gray, fine SAND and SILT, little fine gravel	
110.9 15	SS	S-4	24	5 78 39 41	15	117	0.0			Moist, very dense, gray, fine to coarse Sand and fine Gravel, some silt	
	SS	S-5	3	100/3"	3	>100	0.0			Moist, very dense, brown-gray, fine to coarse SAND, some silt, little fine gravel	
105.9											

**Sample Types****Consistency vs Blowcount/Foot****Burmister Classification**AS - Auger/Grab Sample  
CS - California Sampler  
BQ - 1.5" Rock Core  
NX - 2" Rock CoreHP - Hydro Punch  
SS - Split Spoon  
ST - Shelby Tube  
WS - Wash Sample  
GP - Geoprobe

Granular (Sand):	
V. Loose: 0-4	Dense: 30-50
Loose: 4-10	V. Dense: >50
M. Dense: 10-30	

Fine Grained (Clay):	
V. Soft: <2	Stiff: 8-15
Soft: 2-4	V. Stiff: 15-30
M. Stiff: 4-8	Hard: >30

and	35-50%
some	20-35%
little	10-20%
trace	<10%
moisture, density, color	

**Reviewed by:** T. Dunn**Date:** 11/15/2017**Boring Number:** B-15 (MW)





# Boring Number: B-15 (MW)

**Client:** Town of Salem**Project Name:** S. Broadway Watermain Replacement**Project Location:** Salem, NH**Project Number:** 25972-215427

Elev. Depth (ft)	Sample Type	Sample Number	Sample Length (in)	Blows per 6 inches	Sample Recovery (in)	N-Value	Organic Vapor Reading (ppm)	Graphic Log	Strata	Material Description	Remarks
105.9 20											
100.9 25											
95.9 30											
90.9 35											
85.9 40											
80.9 45											

End of exploration at 19.25 ft bgs.

**Boring Number: B-15 (MW)**



# Boring Number: B-16

**Client:** Town of Salem**Project Name:** S. Broadway Watermain Replacement**Project Location:** Salem, NH**Project Number:** 25972-215427**Drilling Contractor/Driller:** New England Boring Contractors / P. Schofield**Surface Elevation (ft.):** 125.5**Drilling Method/Casing/Core Barrel Size:** Drive and Wash / 4 in / NA**Total Depth (ft.):** 8**Hammer Weight/Drop Height/ Spoon Size:** 140 lb / 30 in / 2 in O.D.**Depth to Initial Water Level (ft):****Bore Hole Location:** See boring location plan**Depth Date Time****Drilling Date: Start:** 9/18/2017 **End:** 9/18/2017

NE 9/18/2017 9:50 am

**N:** 99409.56 **E:** 1103559.09**Abandonment Method:** Backfilled with soil cuttings.**Logged By:** J. Todd

Elev. Depth (ft)	Sample Type	Sample Number	Sample Length (in)	Blows per 6 inches	Sample Recovery (in)	N-Value	Organic Vapor Reading (ppm)	Graphic Log	Strata	Material Description	Remarks
125.5 0										6" Asphalt	
										4" Concrete	
	SS	S-1	18	15 17 39	10	56	0.0		Gravelly Sand	Moist, very dense, brown, fine to coarse SAND, some fine gravel, trace silt	
	SS	S-2	4	100/4"	1	>100	0.0			Moist, very dense, brown, fine GRAVEL, trace fine to coarse sand, trace silt	
120.5 5	SS	S-3	24	9 9 8 11	14	17	0.0		Silty Sand	Moist, medium dense, light brown, fine to medium SAND, some silt	
	SS	S-4	24	12 11 13 14	16	24	0.0			Moist, medium dense, gray brown, fine SAND and SILT	
115.5 10										Obstruction encountered, boring offset 9.5 feet north to B-16A.	
110.5 15											
105.5											

**Sample Types****Consistency vs Blowcount/Foot****Burmister Classification**AS - Auger/Grab Sample  
CS - California Sampler  
BQ - 1.5" Rock Core  
NX - 2" Rock CoreHP - Hydro Punch  
SS - Split Spoon  
ST - Shelby Tube  
WS - Wash Sample  
GP - Geoprobe**Granular (Sand):**V. Loose: 0-4 Dense: 30-50  
Loose: 4-10 V. Dense: >50  
M. Dense: 10-30**Fine Grained (Clay):**V. Soft: <2 Stiff: 8-15  
Soft: 2-4 V. Stiff: 15-30  
M. Stiff: 4-8 Hard: >30and 35-50%  
some 20-35%  
little 10-20%  
trace <10%  
moisture, density, color**Reviewed by:** T. Dunn**Date:** 11/15/2017**Boring Number:** B-16



# Boring Number: B-16A

**Client:** Town of Salem  
**Project Location:** Salem, NH

**Project Name:** S. Broadway Watermain Replacement  
**Project Number:** 25972-215427

**Drilling Contractor/Driller:** New England Boring Contractors / P. Schofield

**Surface Elevation (ft.):** 125.5

**Drilling Method/Casing/Core Barrel Size:** Drive and Wash / 4 in / NA

**Total Depth (ft.):** 25

**Hammer Weight/Drop Height/ Spoon Size:** 140 lb / 30 in / 2 in O.D.

**Depth to Initial Water Level (ft):**

**Bore Hole Location:** See boring location plan

**Depth Date Time**

**Drilling Date: Start:** 9/18/2017 **End:** 9/18/2017

4.1 9/18/2017 11:49 am

**N:** 99401.17 **E:** 1103563.55

**Abandonment Method:** Backfilled with soil cuttings.

**Logged By:** J. Todd

Elev. Depth (ft)	Sample Type	Sample Number	Sample Length (in)	Blows per 6 inches	Sample Recovery (in)	N-Value	Organic Vapor Reading (ppm)	Graphic Log	Strata	Material Description	Remarks		
125.5 0										Continuation of B-16, rollerbit directly to 8 ft bgs and continue sampling.			
120.5 5													
115.5 10	SS	S-1	24	7 10 8 7	17	18	0.0						
110.5 15	SS	S-2	24	4 6 6 6	16	12	0.0		Clayey Silt	Moist, stiff, gray, SILT & CLAY, some fine to medium sand			
105.5	SS	S-3	24	4 6	15	13	0.0			Moist, stiff, gray, SILT & CLAY, little fine sand			
Sample Types						Consistency vs Blowcount/Foot					Burmister Classification		
AS - Auger/Grab Sample		HP - Hydro Punch		Granular (Sand):			Fine Grained (Clay):			and 35-50%			
CS - California Sampler		SS - Split Spoon		V. Loose: 0-4 Dense: 30-50			V. Soft: <2 Stiff: 8-15			some 20-35%			
BQ - 1.5" Rock Core		ST - Shelby Tube		Loose: 4-10 V. Dense: >50			Soft: 2-4 V. Stiff: 15-30			little 10-20%			
NX - 2" Rock Core		WS - Wash Sample		M. Dense: 10-30			M. Stiff: 4-8 Hard: >30			trace <10%			
		GP - Geoprobe								moisture, density, color			
Reviewed by: T. Dunn										Date: 11/15/2017		Boring Number: B-16A	

BL S. BROADWAY SALEM NH - BORING LOGS.GPJ - 12/15/17



# Boring Number: B-16A

**Client:** Town of Salem**Project Name:** S. Broadway Watermain Replacement**Project Location:** Salem, NH**Project Number:** 25972-215427

Elev. Depth (ft)	Sample Type	Sample Number	Sample Length (in)	Blows per 6 inches	Sample Recovery (in)	N-Value	Organic Vapor Reading (ppm)	Graphic Log	Strata	Material Description	Remarks
105.5 20	SS	S-3	24	7 8	15	13	0.0		Clayey Silt	Moist, stiff, gray, SILT & CLAY, little fine sand	
	SS	S-4	24	5 7 8 7	18	15	0.0				
100.5 25										End of exploration at 25 ft bgs.	
95.5 30											
90.5 35											
85.5 40											
80.5 45											

**Boring Number: B-16A**



# Boring Number: B-17

**Client:** Town of Salem  
**Project Location:** Salem, NH

**Project Name:** S. Broadway Watermain Replacement  
**Project Number:** 25972-215427

**Drilling Contractor/Driller:** New England Boring Contractors / P. Schofield

**Surface Elevation (ft.):** 124.2

**Drilling Method/Casing/Core Barrel Size:** Drive and Wash / 4 in / NA

**Total Depth (ft.):** 25

**Hammer Weight/Drop Height/ Spoon Size:** 140 lb / 30 in / 2 in O.D.

**Depth to Initial Water Level (ft):**

**Bore Hole Location:** See boring location plan

**Depth Date Time**

**Drilling Date: Start:** 9/15/2017 **End:** 9/15/2017

7.8 9/15/2017 1:00 pm

**N:** 99147.47 **E:** 1103703.64

**Abandonment Method:** Backfilled with soil cuttings.

**Logged By:** J. Todd

Elev. Depth (ft)	Sample Type	Sample Number	Sample Length (in)	Blows per 6 inches	Sample Recovery (in)	N-Value	Organic Vapor Reading (ppm)	Graphic Log	Strata	Material Description	Remarks
124.2 0										10" Asphalt	
	SS	S-1	18	18 16 11	9	27				Moist, medium dense, brown, fine to medium SAND, little silt	
	SS	S-2	18	5 3 6	6	9				Moist, loose, brown, Slightly Organic fine to medium SAND, some silt	Odor of rotting organics.
119.2 5	SS	S-3	24	6 7 8 8	12	15				Moist, medium dense, brown, fine SAND and SILT	
	SS	S-4	24	13 11 11 10	19	22				Moist, medium dense, brown, fine SAND and SILT	
	SS	S-5	24	7 10 12 13	11	22				Moist, medium dense, brown-gray, fine SAND, some silt	
114.2 10											
109.2 15	SS	S-6	24	7 10 8 8	12	18				Moist, medium dense, gray, fine SAND, some silt	
104.2	SS	S-7	24	6 7	22	12				Moist, stiff, gray-brown, SILT, trace fine sand	

Sample Types				Consistency vs Blowcount/Foot				Burmister Classification	
AS - Auger/Grab Sample	HP - Hydro Punch	Granular (Sand):		Fine Grained (Clay):				and	35-50%
CS - California Sampler	SS - Split Spoon	V. Loose: 0-4	Dense: 30-50	V. Soft: <2	Stiff: 8-15			some	20-35%
BQ - 1.5" Rock Core	ST - Shelby Tube	Loose: 4-10	V. Dense: >50	Soft: 2-4	V. Stiff: 15-30			little	10-20%
NX - 2" Rock Core	WS - Wash Sample	M. Dense: 10-30		M. Stiff: 4-8	Hard: >30			trace	<10%
	GP - Geoprobe							moisture, density, color	

**Reviewed by:** T. Dunn

**Date:** 11/15/2017

**Boring Number:** B-17





# Boring Number: B-18

**Client:** Town of Salem  
**Project Location:** Salem, NH

**Project Name:** S. Broadway Watermain Replacement  
**Project Number:** 25972-215427

**Drilling Contractor/Driller:** New England Boring Contractors / P. Schofield

**Surface Elevation (ft.):** 125.1

**Drilling Method/Casing/Core Barrel Size:** Drive and Wash / 4 in / NA

**Total Depth (ft.):** 21

**Hammer Weight/Drop Height/ Spoon Size:** 140 lb / 30 in / 2 in O.D.

**Depth to Initial Water Level (ft):**

**Bore Hole Location:** See boring location plan

Depth	Date	Time
8.2	9/15/2017	10:45 am

**Drilling Date: Start:** 9/15/2017 **End:** 9/15/2017

**Abandonment Method:** Backfilled with soil cuttings.

**N:** 98933.24 **E:** 1103819.38

**Logged By:** J. Todd

Elev. Depth (ft)	Sample Type	Sample Number	Sample Length (in)	Blows per 6 inches	Sample Recovery (in)	N-Value	Organic Vapor Reading (ppm)	Graphic Log	Strata	Material Description	Remarks
125.1											
0										8" Asphalt	
	SS	S-1	18	20 25 23	4	48	1.1		Sand	Moist, dense, brown, fine to coarse SAND, trace fine gravel, trace silt	
	SS	S-2	18	8 9 11	14	20	0.0			Moist, medium dense, brown, fine SAND, some silt	
120.1	SS	S-3	24	9 14 13 11	13	27	0.0			Moist, medium dense, tan to brown, fine SAND and SILT	
5	SS	S-4	24	10 10 11 11	20	21	0.0			Moist, medium dense, brown, SILT, little fine sand	
	SS	S-5	24	8 9 6 7	14	15	0.0			Moist, medium dense, brown, fine SAND and SILT	
115.1											
10											
110.1	SS	S-6	24	7 6 6 6	14	12	0.0			Moist, medium dense, light brown, fine SAND, some silt	
15											
105.1	SS	S-7	24	4 4	15	9	0.0			Moist, stiff, light brown, SILT, some fine sand	

Sample Types				Consistency vs Blowcount/Foot				Burmister Classification	
AS - Auger/Grab Sample	HP - Hydro Punch			Granular (Sand):		Fine Grained (Clay):		and	35-50%
CS - California Sampler	SS - Split Spoon			V. Loose: 0-4	Dense: 30-50	V. Soft: <2	Stiff: 8-15	some	20-35%
BQ - 1.5" Rock Core	ST - Shelby Tube			Loose: 4-10	V. Dense: >50	Soft: 2-4	V. Stiff: 15-30	little	10-20%
NX - 2" Rock Core	WS - Wash Sample			M. Dense: 10-30		M. Stiff: 4-8	Hard: >30	trace	<10%
	GP - Geoprobe							moisture, density, color	

**Reviewed by:** T. Dunn

**Date:** 11/15/2017

**Boring Number:** B-18



# Boring Number: B-18

**Client:** Town of Salem**Project Name:** S. Broadway Watermain Replacement**Project Location:** Salem, NH**Project Number:** 25972-215427

Elev. Depth (ft)	Sample Type	Sample Number	Sample Length (in)	Blows per 6 inches	Sample Recovery (in)	N-Value	Organic Vapor Reading (ppm)	Graphic Log	Strata	Material Description	Remarks
105.1 20	SS	S-7	24	5 4	15	9	0.0		Silty Sand	Moist, stiff, brown, SILT and fine SAND	
	SS	S-8	24	4 5 6 5	15	11	0.0				
100.1 25										End of exploration at 25 ft bgs.	
95.1 30											
90.1 35											
85.1 40											
80.1 45											

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**Boring Number: B-18**



**Boring Number:  
B-19**

**Client:** Town of Salem

**Project Name:** S. Broadway Watermain Replacement

**Project Location:** Salem, NH

**Project Number:** 25972-215427

**Drilling Contractor/Driller:** New England Boring Contractors / P. Schofield

**Surface Elevation (ft.):** 125.3

**Drilling Method/Casing/Core Barrel Size:** Drive and Wash / 4 in / NA

**Total Depth (ft.): 25**

**Hammer Weight/Drop Height/ Spoon Size:** 140 lb / 30 in / 2 in O.D.

**Depth to Initial Water Level (ft):**

**Bore Hole Location:** See boring location plan

Depth	Date	Time
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**Drilling Date: Start: 9/14/2017 End: 9/14/2017**

5.8 9/14/2017 2:33 pm

**N: 98636.17    E: 1103981.84**

**Abandonment Method:** Backfilled with soil cuttings.

**Logged By:** J. Todd

Elev. Depth (ft)	Sample Type	Sample Number	Sample Length (in)	Blows per 6 inches	Sample Recovery (in)	N-Value	Organic Vapor Reading (ppm)	Graphic Log	Strata	Material Description	Remarks
125.3 0										7" Asphalt	
										5" Concrete	
	SS	S-1	17	12 20 80/5"	5	>100	0.0		Gravelly Sand	Moist, very dense, brown, fine to coarse SAND and fine to coarse GRAVEL, trace silt	
120.3 5 ▼	SS	S-2	24	11 8 9 8	8	17	0.0			Moist, medium dense, brown, fine SAND and SILT	
	SS	S-3	24	11 12 12 10	18	24	0.0			Moist, medium dense, light brown, fine to medium SAND and SILT	
	SS	S-4	24	6 8 11 10	13	19	0.0			Moist, medium dense, brown gray, fine SAND and SILT	
115.3 10											
110.3 15	SS	S-5	24	3 4 5 5	15	9	0.0			Moist, stiff, gray, SILT, trace fine to medium sand	
105.3	SS	S-6	24	6 7	17	13	0.0			Moist, stiff, gray, SILT, some fine sand	

Sample Types		Consistency vs Blowcount/Foot				Burmister Classification	
AS - Auger/Grab Sample	HP - Hydro Punch	<u>Granular (Sand):</u>		<u>Fine Grained (Clay):</u>		and	35-50%
CS - California Sampler	SS - Split Spoon	V. Loose:	0-4	Dense:	30-50	some	20-35%
BQ - 1.5" Rock Core	ST - Shelby Tube	Loose:	4-10	V. Dense:	>50	little	10-20%
NX - 2" Rock Core	WS - Wash Sample	M. Dense:	10-30			trace	<10%
	GP - Geoprobe					moisture	density, color

Reviewed by: T. Dunn

Date: 11/15/2017

**Boring Number: B-19**



# Boring Number: B-19

Client: Town of Salem

Project Name: S. Broadway Watermain Replacement

Project Location: Salem, NH

Project Number: 25972-215427

Elev. Depth (ft)	Sample Type	Sample Number	Sample Length (in)	Blows per 6 inches	Sample Recovery (in)	N-Value	Organic Vapor Reading (ppm)	Graphic Log	Strata	Material Description	Remarks
105.3 20	SS	S-6	24	6 6	17	13	0.0		Silty Sand	Moist, medium dense, gray, fine SAND and SILT	
	SS	S-7	24	6 9 11 11	18	20	0.0				
100.3 25										End of exploration at 25 ft bgs.	
95.3 30											
90.3 35											
85.3 40											
80.3 45											

BL S. BROADWAY SALEM NH - BORING LOGS.GPJ - 12/15/17

Boring Number: B-19

**Boring Number:  
B-20**

**Client:** Town of Salem

**Project Name:** S. Broadway Watermain Replacement

**Project Location:** Salem, NH

**Project Number:** 25972-215427

**Drilling Contractor/Driller:** New England Boring Contractors / P. Schofield

**Surface Elevation (ft.):** 123.2

**Drilling Method/Casing/Core Barrel Size:** Drive and Wash / 4 in / NA

**Total Depth (ft.): 13**

**Hammer Weight/Drop Height/ Spoon Size:** 140 lb / 30 in / 2 in O.D.

**Depth to Initial Water Level (ft):**

**Bore Hole Location:** See boring location plan

Depth	Date	Time
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**Drilling Date: Start: 9/13/2017 End: 9/14/2017**

4.2      9/14/17      8:00 am

**N: 98266.35    E: 1104185.53**

**Abandonment Method:** Backfilled with soil cuttings.

**Logged By:** J. Todd

[illegible]

Sample Types		Consistency vs Blowcount/Foot				Burmister Classification	
AS - Auger/Grab Sample	HP - Hydro Punch	Granular (Sand):		Fine Grained (Clay):		and	35-50%
CS - California Sampler	SS - Split Spoon	V. Loose:	0-4	Dense:	30-50	some	20-35%
BQ - 1.5" Rock Core	ST - Shelby Tube	Loose:	4-10	V. Dense:	>50	little	10-20%
NX - 2" Rock Core	WS - Wash Sample	M. Dense:	10-30			trace	<10%
	GP - Geoprobe					moisture	densitv. color

Reviewed by: T. Dunn

Date: 11/15/2017

**Boring Number: B-20**

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# Boring Number: B-20

**Client:** Town of Salem**Project Name:** S. Broadway Watermain Replacement**Project Location:** Salem, NH**Project Number:** 25972-215427

Elevation Depth (ft)	Sample Type	Sample Number	Sample Length (in)	Recovery (%)	RQD (%)	Drill Rate (min/ft)	Down Press. (psi)	Graphic Log	Strata	Material Description	Remarks								
	Core	C-1	12	0	0		NR		Boulders/Cobbles	Broke through hard material at approximately 11.8 ft bgs. More hard material was encountered at 12.5 ft bgs. No recovery on first rock core attempt, the driller needs to reseal the casing before attempting a second core at 12.5 ft bgs. The casing crimped while attempting to advance it to 13 ft bgs. Offset boring 5 ft north to B-20A.									
108.2 15.0																			
103.2 20.0																			
98.2 25.0																			
93.2 30.0																			
Bedding (mm)				Joint Spacing (mm)				Continuity (mm)		Attitude Angle		Aperture (mm)							
Extremely Thin		<20		Extremely Close		<20		Extremely		<25		Horizontal		0° - 5°		Very Tight		< 0.1	
Very Thin		20-60		Very Close		20-60		Moderately		25-100		Shallow		5° - 35°		Tight		0.1 - 0.25	
Thin		60-200		Close		60-200		Slightly		100-200		Moderate		35° - 55°		Partly Open		0.25 - 0.5	
Medium		200-600		Mod Close		200-600		Sound		>200		Steep		55° - 85°		Open		0.5 - 2.5	
Thick		600-2000		Wide		600-2000						Vertical		85° - 90°		Mod. Wide		2.5 - 10	
Very Thick		2000-6000		Very Wide		2000-6000										Wide		>10	
Extremely Thick		>6000		Extremely Wide		>6000													
Field Hardness				Weathering															
Very Hard		Knife Can't Scratch		Fresh		No Visible sign of rock material weathering; slight to no discoloration.													
Hard		Scratches with Difficulty		Slight		Discoloration indicated weathering. All the rock material may be discolored and may be weaker externally than its fresh condition.													
Med. Hard		Scratches Readily		Moderate		Less than half of the rock material is decomposed and/or disintegrated to a soil. Fresh or discolored rock is present either as a continuous framework or as corestones.													
Medium		Grooves with Difficulty		Severe		More than half of the rock material is decomposed and/or disintegrated to a soil. Fresh or discolored rock is present either as a continuous framework or as corestone.													
Soft		Grooves Readily		Complete		All rock material is decomposed and/or disintegrated to soil. The original mass structure is largely intact.													
Very Soft		Carves with Knife		Residual Soil		All rock material is converted to soil. The mass structure and material fabric are destroyed. There is a large change in volume, but the soil has not been significantly transported.													
Reviewed by: T. Dunn										Date: 11/15/2017		Boring Number: B-20							

ROCK CORING LOG S. BROADWAY SALEM NH - BORING LOGS.GPJ - 12/14/17



# Boring Number: B-20A

**Client:** Town of Salem**Project Name:** S. Broadway Watermain Replacement**Project Location:** Salem, NH**Project Number:** 25972-215427**Drilling Contractor/Driller:** New England Boring Contractors / P. Schofield**Surface Elevation (ft.):** 123.2**Drilling Method/Casing/Core Barrel Size:** Drive and Wash / 4 in / NA**Total Depth (ft.):** 16.5**Hammer Weight/Drop Height/ Spoon Size:** 140 lb / 30 in / 2 in O.D.**Depth to Initial Water Level (ft):****Bore Hole Location:** See boring location plan**Depth Date Time****Drilling Date: Start:** 9/14/2017 **End:** 9/14/2017

NR

**N:** 98265.45 **E:** 1104185.96**Abandonment Method:** Backfilled with soil cuttings.**Logged By:** J. Todd

Elev. Depth (ft)	Sample Type	Sample Number	Sample Length (in)	Blows per 6 inches	Sample Recovery (in)	N-Value	Organic Vapor Reading (ppm)	Graphic Log	Strata	Material Description	Remarks
123.2 0										Continuation of B-20, rollerbit directly to 14 ft bgs and continue sampling.	
118.2 5											
113.2 10											
108.2 15	SS	S-1	20	42 27 28 72/2"	7	55	0.0		Gravelly Sand	Moist, very dense, light brown, fine to coarse SAND, some fine to coarse gravel, trace silt	Casing stuck at 14 ft bgs. Rollerbit refusal at 16.5 ft bgs. Rollerbit was advanced into hard material for approximately 1 ft (15.5 to 16.5 ft).
103.2										Casing stuck at 14 ft bgs, driller is unable to core rock. End of exploration at 16.5 ft bgs.	

**Sample Types****Consistency vs Blowcount/Foot****Burmister Classification**AS - Auger/Grab Sample  
CS - California Sampler  
BQ - 1.5" Rock Core  
NX - 2" Rock CoreHP - Hydro Punch  
SS - Split Spoon  
ST - Shelby Tube  
WS - Wash Sample  
GP - Geoprobe**Granular (Sand):**V. Loose: 0-4 Dense: 30-50  
Loose: 4-10 V. Dense: >50  
M. Dense: 10-30**Fine Grained (Clay):**V. Soft: <2 Stiff: 8-15  
Soft: 2-4 V. Stiff: 15-30  
M. Stiff: 4-8 Hard: >30and 35-50%  
some 20-35%  
little 10-20%  
trace <10%  
moisture, density, color**Reviewed by:** T. Dunn**Date:** 11/15/2017**Boring Number:** B-20A



# Boring Number: B-21

**Client:** Town of Salem  
**Project Location:** Salem, NH

**Project Name:** S. Broadway Watermain Replacement  
**Project Number:** 25972-215427

**Drilling Contractor/Driller:** New England Boring Contractors / P. Schofield

**Surface Elevation (ft.):** 124.4

**Drilling Method/Casing/Core Barrel Size:** Drive and Wash / 4 in / NA

**Total Depth (ft.):** 25

**Hammer Weight/Drop Height/ Spoon Size:** 140 lb / 30 in / 2 in O.D.

**Depth to Initial Water Level (ft):**

**Bore Hole Location:** See boring location plan

Depth	Date	Time
7.8	9/13/2017	10:34 am

**Drilling Date: Start:** 9/13/2017 **End:** 9/13/2017

**Abandonment Method:** Backfilled with soil cuttings.

**N:** 98036.44 **E:** 1104311.34

**Logged By:** J. Todd

Elev. Depth (ft)	Sample Type	Sample Number	Sample Length (in)	Blows per 6 inches	Sample Recovery (in)	N-Value	Organic Vapor Reading (ppm)	Graphic Log	Strata	Material Description	Remarks
124.4										5" Asphalt	
0										5" Concrete	
	SS	S-1	18	10 13 11	7	24	0.0			Moist, medium dense, light brown, fine to medium SAND, some silt, trace fine gravel	
	SS	S-2	18	13 10 10	8	20	0.0			Moist, medium dense, brown, Slightly Organic fine to medium SAND, little clayey silt, trace fine gravel	
119.4	SS	S-3	24	4 3 6 11	16	9	0.0			Moist, loose, dark brown to black, Slightly Organic fine to medium SAND, little clayey silt	
5	SS	S-4	24	14 10 10 9	18	20	0.0			Moist, medium dense, brown, fine to coarse SAND, some silt	
	SS	S-5	24	7 6 7 6	14	13	0.0			Moist, medium dense, gray, SILT, trace fine sand	
114.4											
10											
	SS	S-6	24	7 5 6 10	13	11	0.0			Moist, medium dense, brown, fine to coarse SAND, trace silt, trace fine gravel	
109.4											
15											
	SS	S-7	24	18 18	6	64	0.0			Moist, very dense, brown-gray, fine to coarse Sand and fine to coarse Gravel, little	
104.4											

**Sample Types**

AS - Auger/Grab Sample  
CS - California Sampler  
BQ - 1.5" Rock Core  
NX - 2" Rock Core  
HP - Hydro Punch  
SS - Split Spoon  
ST - Shelby Tube  
WS - Wash Sample  
GP - Geoprobe

**Consistency vs Blowcount/Foot**

**Granular (Sand):**  
V. Loose: 0-4  
Loose: 4-10  
M. Dense: 10-30  
Dense: 30-50  
V. Dense: >50

**Fine Grained (Clay):**  
V. Soft: <2  
Soft: 2-4  
M. Stiff: 4-8  
Stiff: 8-15  
V. Stiff: 15-30  
Hard: >30

**Burmister Classification**

and 35-50%  
some 20-35%  
little 10-20%  
trace <10%  
moisture, density, color

**Reviewed by:** T. Dunn

**Date:** 11/15/2017

**Boring Number:** B-21





# Boring Number: B-22

**Client:** Town of Salem  
**Project Location:** Salem, NH

**Project Name:** S. Broadway Watermain Replacement  
**Project Number:** 25972-215427

**Drilling Contractor/Driller:** New England Boring Contractors / P. Schofield

**Surface Elevation (ft.):** 124.3

**Drilling Method/Casing/Core Barrel Size:** Drive and Wash / 4 in / NA

**Total Depth (ft.):** 23

**Hammer Weight/Drop Height/ Spoon Size:** 140 lb / 30 in / 2 in O.D.

**Depth to Initial Water Level (ft):**

**Bore Hole Location:** See boring location plan

Depth	Date	Time
4.0	10/4/2017	12:30 pm

**Drilling Date: Start:** 10/4/2017 **End:** 10/4/2017

**Abandonment Method:** Backfilled with soil cuttings.

**N:** 97773.37 **E:** 1104414.74

**Logged By:** A. Smith

Elev. Depth (ft)	Sample Type	Sample Number	Sample Length (in)	Blows per 6 inches	Sample Recovery (in)	N-Value	Organic Vapor Reading (ppm)	Graphic Log	Strata	Material Description	Remarks
124.3										8" Asphalt	
0	SS	S-1	18	27 32 38	12	70	0.0			4" Subbase: Fine to coarse SAND, some fine gravel, little silt Moist, very dense, brown, fine to coarse SAND, some fine gravel, little silt	Obtained sample of subbase soil beneath asphalt.
	SS	S-2	18	18 52 45	10	97	0.0			Moist, very dense, brown, fine to coarse SAND, some fine gravel, little silt	
119.3	SS	S-3	24	20 22 28 52	12	50	0.0			Moist, very dense, brown, fine to coarse SAND, some fine to coarse gravel, trace silt	
5	SS	S-4	24	28 34 26 22	16	60	0.0			Wet, very dense, light brown, fine SAND, some silt	
	SS	S-5	24	19 16 11 10	12	27	0.0			Wet, medium dense, light brown, fine to medium SAND, some silt, trace fine to coarse gravel	
114.3											
10											
	SS	S-6	24	11 16 30 32	8	46	0.0			Wet, dense, brown, fine to coarse Sand and fine to coarse Gravel, little silt	
109.3											
15											
	SS	S-7	0	100/0"	0	>100	0.0			No Recovery	Casing refusal at 17.5 ft bgs, advance boring with rollerbit to 19 ft bgs. Advance rollerbit to 21.5 ft bgs.
104.3											

Sample Types				Consistency vs Blowcount/Foot				Burmister Classification	
AS - Auger/Grab Sample	HP - Hydro Punch	Granular (Sand):		Fine Grained (Clay):				and	35-50%
CS - California Sampler	SS - Split Spoon	V. Loose: 0-4 Dense: 30-50		V. Soft: <2 Stiff: 8-15				some	20-35%
BQ - 1.5" Rock Core	ST - Shelby Tube	Loose: 4-10 V. Dense: >50		Soft: 2-4 V. Stiff: 15-30				little	10-20%
NX - 2" Rock Core	WS - Wash Sample	M. Dense: 10-30		M. Stiff: 4-8 Hard: >30				trace	<10%
	GP - Geoprobe							moisture, density, color	

**Reviewed by:** T. Dunn

**Date:** 11/15/2017

**Boring Number:** B-22





# Boring Number: B-22

Client: Town of Salem

Project Name: S. Broadway Watermain Replacement

Project Location: Salem, NH

Project Number: 25972-215427

Elev. Depth (ft)	Sample Type	Sample Number	Sample Length (in)	Blows per 6 inches	Sample Recovery (in)	N-Value	Organic Vapor Reading (ppm)	Graphic Log	Strata	Material Description	Remarks
104.3 20											
	SS	S-8	0	100/0"	0	>100	0.0		Weathered Rock	No Recovery	Attempt to core rock at 21.5 ft bgs. Core barrel plugged with fine sand, advance rollerbit to 23 ft bgs.
99.3 25										End of exploration at 23 ft bgs.	
94.3 30											
89.3 35											
84.3 40											
79.3 45											

BL S. BROADWAY SALEM NH - BORING LOGS.GPJ - 12/15/17

Boring Number: B-22



# Boring Number: B-23

**Client:** Town of Salem  
**Project Location:** Salem, NH

**Project Name:** S. Broadway Watermain Replacement  
**Project Number:** 25972-215427

**Drilling Contractor/Driller:** New England Boring Contractors / P. Schofield

**Surface Elevation (ft.):** 124.3

**Drilling Method/Casing/Core Barrel Size:** Drive and Wash / 4 in / NA

**Total Depth (ft.):** 24

**Hammer Weight/Drop Height/ Spoon Size:** 140 lb / 30 in / 2 in O.D.

**Depth to Initial Water Level (ft):**

**Bore Hole Location:** See boring location plan

**Depth Date Time**

**Drilling Date: Start:** 9/12/2017 **End:** 9/12/2017

7 9/12/2017 3:08 pm

**N:** 97443.66 **E:** 1104639.4

**Abandonment Method:** Backfilled with soil cuttings.

**Logged By:** J. Todd

Elev. Depth (ft)	Sample Type	Sample Number	Sample Length (in)	Blows per 6 inches	Sample Recovery (in)	N-Value	Organic Vapor Reading (ppm)	Graphic Log	Strata	Material Description	Remarks
124.3										7" Asphalt	
0										5" Concrete	
	SS	S-1	18	8 27 17	4	44	0.0		Sand	Moist, dense, brown, fine to coarse SAND, little fine gravel, little silt	
	SS	S-2	18	9 9 9	3	18	0.0			Moist, medium dense, brown, fine to coarse SAND, little silt, trace fine gravel	
119.3	SS	S-3	24	9 11 9 9	1	20	0.0			Moist, medium dense, brown, fine to coarse SAND, trace fine gravel, trace silt	
5	SS	S-4	24	13 13 14 11	13	27	0.0		Silty Sand	Moist, medium dense, light brown, fine SAND and SILT	
	SS	S-5	24	4 5 5 4	16	10	0.0			Moist, stiff, light brown, SILT & CLAY, little fine sand	
114.3											
10											
	SS	S-6	24	9 15 12 14	15	27	0.0		Sand	Moist, medium dense, light brown, fine to coarse SAND, some silt, little fine gravel	Hard drilling (rig chatter and slow advance) from 20 ft to 22 ft bgs. Rollerbit refusal at 24 ft bgs after 3 attempts to drive a split spoon to 24 ft bgs. The borehole repeatedly collapsed at 21 ft or less. Soil cuttings appear similar to sample
109.3											
15	SS	S-7	12	76 100	2	100	0.0			Moist, very dense, gray, fine to coarse SAND, little silt, trace fine gravel	
104.3											

**Sample Types**

AS - Auger/Grab Sample  
CS - California Sampler  
BQ - 1.5" Rock Core  
NX - 2" Rock Core  
HP - Hydro Punch  
SS - Split Spoon  
ST - Shelby Tube  
WS - Wash Sample  
GP - Geoprobe

**Consistency vs Blowcount/Foot**

**Granular (Sand):**  
V. Loose: 0-4 Dense: 30-50  
Loose: 4-10 V. Dense: >50  
M. Dense: 10-30

**Fine Grained (Clay):**  
V. Soft: <2 Stiff: 8-15  
Soft: 2-4 V. Stiff: 15-30  
M. Stiff: 4-8 Hard: >30

**Burmister Classification**

and 35-50%  
some 20-35%  
little 10-20%  
trace <10%  
moisture, density, color

**Reviewed by:** T. Dunn

**Date:** 11/15/2017

**Boring Number:** B-23



# Boring Number: B-23

**Client:** Town of Salem**Project Name:** S. Broadway Watermain Replacement**Project Location:** Salem, NH**Project Number:** 25972-215427

Elev. Depth (ft)	Sample Type	Sample Number	Sample Length (in)	Blows per 6 inches	Sample Recovery (in)	N-Value	Organic Vapor Reading (ppm)	Graphic Log	Strata	Material Description	Remarks
104.3 20											S-7.
									Sand		
99.3 25										End of exploration at 24 ft bgs.	
94.3 30											
89.3 35											
84.3 40											
79.3 45											

BL S. BROADWAY SALEM NH - BORING LOGS.GPJ - 12/15/17

**Boring Number: B-23**



# Boring Number: B-24

**Client:** Town of Salem  
**Project Location:** Salem, NH

**Project Name:** S. Broadway Watermain Replacement  
**Project Number:** 25972-215427

**Drilling Contractor/Driller:** New England Boring Contractors / P. Schofield

**Surface Elevation (ft.):** 124.4

**Drilling Method/Casing/Core Barrel Size:** Drive and Wash / 4 in / NA

**Total Depth (ft.):** 25

**Hammer Weight/Drop Height/ Spoon Size:** 140 lb / 30 in / 2 in O.D.

**Depth to Initial Water Level (ft):**

**Bore Hole Location:** See boring location plan

Depth	Date	Time
7.2	9/12/2017	10:52 am

**Drilling Date: Start:** 9/12/2017 **End:** 9/12/2017

**Abandonment Method:** Backfilled with soil cuttings.

**N:** 97215.59 **E:** 1104764.11

**Logged By:** J. Todd

Elev. Depth (ft)	Sample Type	Sample Number	Sample Length (in)	Blows per 6 inches	Sample Recovery (in)	N-Value	Organic Vapor Reading (ppm)	Graphic Log	Strata	Material Description	Remarks
124.4										7" Asphalt	
0										5" Concrete	
	SS	S-1	18	15 20 13	12	33	0.0		Sand	Moist, dense, brown, fine to coarse SAND, some fine gravel, trace silt	
	SS	S-2	18	12 9 7	4	16	0.0			Moist, medium dense, brown, fine to coarse SAND, little fine gravel, little silt	
119.4 5	SS	S-3	24	8 10 8 8	14	18	0.0			Moist, medium dense, brown, fine to medium SAND, trace fine gravel, trace silt	
	SS	S-4	24	15 15 14 13	20	29	0.0		Silty Sand	Moist, medium dense, light brown to brown, fine to medium SAND, some silt	
	SS	S-5	24	4 10 9 9	16	19	0.0			Moist, medium dense, light brown, fine to medium SAND and SILT	
114.4 10											
	SS	S-6	24	9 29 26 22	10	55	0.0		Gravelly Sand	Moist, very dense, light gray, fine to coarse Sand and fine to coarse Gravel, trace silt	
109.4 15											
	SS	S-7	24	23 61	4	140	0.0			Moist, very dense, gray, fine to coarse GRAVEL, some fine to coarse sand, little silt	
104.4											

Sample Types				Consistency vs Blowcount/Foot				Burmister Classification	
AS - Auger/Grab Sample	HP - Hydro Punch			Granular (Sand):		Fine Grained (Clay):		and	35-50%
CS - California Sampler	SS - Split Spoon			V. Loose: 0-4	Dense: 30-50	V. Soft: <2	Stiff: 8-15	some	20-35%
BQ - 1.5" Rock Core	ST - Shelby Tube			Loose: 4-10	V. Dense: >50	Soft: 2-4	V. Stiff: 15-30	little	10-20%
NX - 2" Rock Core	WS - Wash Sample			M. Dense: 10-30		M. Stiff: 4-8	Hard: >30	trace	<10%
	GP - Geoprobe							moisture, density, color	

**Reviewed by:** T. Dunn

**Date:** 11/15/2017

**Boring Number:** B-24



# Boring Number: B-24

Client: Town of Salem

Project Name: S. Broadway Watermain Replacement

Project Location: Salem, NH

Project Number: 25972-215427

Elev. Depth (ft)	Sample Type	Sample Number	Sample Length (in)	Blows per 6 inches	Sample Recovery (in)	N-Value	Organic Vapor Reading (ppm)	Graphic Log	Strata	Material Description	Remarks
104.4 20	SS	S-7	24	79 24	4	140	0.0		Gravelly Sand		
99.4 25	SS	S-8	10	60 100/4"	3	>100	0.0			Moist, very dense, gray, fine GRAVEL, some fine to coarse sand, trace silt	
										End of exploration at 25 ft bgs.	
94.4 30											
89.4 35											
84.4 40											
79.4 45											

BL S. BROADWAY SALEM NH - BORING LOGS.GPJ - 12/15/17

Boring Number: B-24



# Boring Number: B-25

**Client:** Town of Salem  
**Project Location:** Salem, NH

**Project Name:** S. Broadway Watermain Replacement  
**Project Number:** 25972-215427

**Drilling Contractor/Driller:** New England Boring Contractors / P. Schofield

**Surface Elevation (ft.):** 123.5

**Drilling Method/Casing/Core Barrel Size:** Drive and Wash / 4 in / NA

**Total Depth (ft.):** 26

**Hammer Weight/Drop Height/ Spoon Size:** 140 lb / 30 in / 2 in O.D.

**Depth to Initial Water Level (ft):**

**Bore Hole Location:** See boring location plan

**Depth Date Time**

**Drilling Date: Start:** 9/11/2017 **End:** 9/11/2017

7.4 9/11/2017 2:22 pm

**N:** 97015.81 **E:** 1104864.68

**Abandonment Method:** Backfilled with soil cuttings.

**Logged By:** J. Todd

Elev. Depth (ft)	Sample Type	Sample Number	Sample Length (in)	Blows per 6 inches	Sample Recovery (in)	N-Value	Organic Vapor Reading (ppm)	Graphic Log	Strata	Material Description	Remarks
123.5										6" Asphalt	
0										6" Concrete	
	SS	S-1	18	14 26 16	15	42	0.0			Moist, dense, gray, fine to coarse SAND, some fine to coarse gravel, trace silt	
	SS	S-2	18	15 11 7	14	18	0.0			Moist, medium dense, dark gray, fine to medium SAND, some clayey silt	
118.5	SS	S-3	24	37 14 10 19	13	24	0.0			Moist, medium dense, dark gray to black, Slightly Organic fine to medium SAND, little fine gravel, trace clayey silt	
5	SS	S-4	24	28 41 25 25	24	66	0.0			Top 12": Moist, very dense, gray, fine to coarse SAND, some fine gravel, trace silt Bottom 12": Moist, very dense, brown, fine SAND, little silt	
	SS	S-5	24	7 7 8 7	15	15	0.0			Moist, medium dense, gray, fine SAND, some silt	
113.5											
10											
	SS	S-6	24	3 4 5 5	14	9	0.0			Moist, stiff, gray, SILT, trace fine to medium sand	
108.5											
15											
	SS	S-7	24	5 6	18	13	0.0			Moist, stiff, gray, SILT, trace fine sand	
103.5											

**Sample Types****Consistency vs Blowcount/Foot****Burmister Classification**

AS - Auger/Grab Sample  
CS - California Sampler  
BQ - 1.5" Rock Core  
NX - 2" Rock Core

HP - Hydro Punch  
SS - Split Spoon  
ST - Shelby Tube  
WS - Wash Sample  
GP - Geoprobe

**Granular (Sand):**  
V. Loose: 0-4 Dense: 30-50  
Loose: 4-10 V. Dense: >50  
M. Dense: 10-30

**Fine Grained (Clay):**  
V. Soft: <2 Stiff: 8-15  
Soft: 2-4 V. Stiff: 15-30  
M. Stiff: 4-8 Hard: >30

and 35-50%  
some 20-35%  
little 10-20%  
trace <10%  
moisture, density, color

**Reviewed by:** T. Dunn

**Date:** 11/15/2017

**Boring Number:** B-25



# Boring Number: B-25

**Client:** Town of Salem**Project Name:** S. Broadway Watermain Replacement**Project Location:** Salem, NH**Project Number:** 25972-215427

Elev. Depth (ft)	Sample Type	Sample Number	Sample Length (in)	Blows per 6 inches	Sample Recovery (in)	N-Value	Organic Vapor Reading (ppm)	Graphic Log	Strata	Material Description	Remarks
103.5 20	SS	S-7	24	7 7	18	13	0.0		Silty Sand		
98.5 25	SS	S-8	24	4 6 6 7	20	12	0.0			Moist, medium dense, gray, fine SAND, some silt  End of exploration at 26 ft bgs.	
93.5 30											
88.5 35											
83.5 40											
78.5 45											

BL S. BROADWAY SALEM NH - BORING LOGS.GPJ - 12/15/17

**Boring Number: B-25**

**Boring Number:  
B-26**

**Client:** Town of Salem

**Project Name:** S. Broadway Watermain Replacement

**Project Location:** Salem, NH

**Project Number:** 25972-215427

**Drilling Contractor/Driller:** New England Boring Contractors / P. Schofield

**Surface Elevation (ft.):** 123.5

**Drilling Method/Casing/Core Barrel Size:** Drive and Wash / 4 in / NA

**Total Depth (ft.): 26**

**Hammer Weight/Drop Height/ Spoon Size:** 140 lb / 30 in / 2 in O.D.

**Depth to Initial Water Level (ft):**

**Bore Hole Location:** See boring location plan

Depth	Date	Time
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**Drilling Date: Start: 9/11/2017 End: 9/11/2017**

8.8 9/11/2017 11:50 am

**N: 96757.42    E: 1105007.64**

**Abandonment Method:** Backfilled with soil cuttings.

**Logged By:** J. Todd

Elev. Depth (ft)	Sample Type	Sample Number	Sample Length (in)	Blows per 6 inches	Sample Recovery (in)	N-Value	Organic Vapor Reading (ppm)	Graphic Log	Strata	Material Description	Remarks
123.5 0										5" Asphalt	
										2.5" Subbase	
	SS	S-1	18	9 13 13	12	26	0.0			5" Concrete	Void for
	SS	S-2	18	10 11 9	9	20	0.0			Moist, medium dense, brown, fine to coarse SAND, some fine to coarse gravel, trace silt Moist, medium dense, tan, fine SAND, some silt	approximately 2 inches around circumference of borehole. No subbase sample could be gathered.
118.5 5	SS	S-3	24	9 10 10 12	13	20	0.0			Moist, medium dense, light brown, fine SAND, little silt	
	SS	S-4	24	13 15 15 16	15	30	0.0			Moist, dense, light brown, fine to medium SAND, little silt	
▼	SS	S-5	24	7 8 8 9	13	16	0.0			Moist, medium dense, light brown, fine SAND, trace silt	
113.5 10											
108.5 15	SS	S-6	24	2 3 3 4	12	6	0.0			Moist, loose, light brown, fine SAND, some silt	
103.5	SS	S-7	24	4 4	18	8	0.0			Moist, medium stiff, light brown to gray, SILT, trace fine sand	

Sample Types		Consistency vs Blowcount/Foot						Burmister Classification			
AS - Auger/Grab Sample	HP - Hydro Punch	Granular (Sand):				Fine Grained (Clay):				and	35-50%
CS - California Sampler	SS - Split Spoon	V. Loose:	0-4	Dense:	30-50	V. Soft:	<2	Stiff:	8-15	some	20-35%
BQ - 1.5" Rock Core	ST - Shelby Tube	Loose:	4-10	V. Dense:	>50	Soft:	2-4	V. Stiff:	15-30	little	10-20%
NX - 2" Rock Core	WS - Wash Sample	M. Dense:	10-30			M. Stiff:	4-8	Hard:	>30	trace	<10%
	GP - Geoprobe									moisture density color	

Reviewed by: T. Dunn

Date: 11/15/2017

**Boring Number: B-26**





# Boring Number: B-26

**Client:** Town of Salem**Project Name:** S. Broadway Watermain Replacement**Project Location:** Salem, NH**Project Number:** 25972-215427

Elev. Depth (ft)	Sample Type	Sample Number	Sample Length (in)	Blows per 6 inches	Sample Recovery (in)	N-Value	Organic Vapor Reading (ppm)	Graphic Log	Strata	Material Description	Remarks
103.5 20	SS	S-7	24	4 6	18	8	0.0		Silty Sand		
98.5 25	SS	S-8	24	4 4 5 6	18	9	0.0			Moist, loose, gray, fine to medium SAND, little silt  End of exploration at 26 ft bgs.	
93.5 30											
88.5 35											
83.5 40											
78.5 45											

BL S. BROADWAY SALEM NH - BORING LOGS.GPJ - 12/15/17

**Boring Number: B-26**



**Boring Number:  
B-27**

**Project Name:** S. Broadway Watermain Replacement

**Project Number:** 25972-215427

**Surface Elevation (ft.):** 143.1

**Total Depth (ft.): 10**

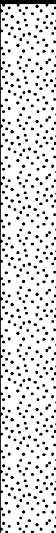
**Depth to Initial Water Level (ft):**

Depth	Date	Time
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NE 9/25/2017 2:15 pm

**Abandonment Method:** Backfilled with soil cuttings.

**Logged By:** A. Smith

Elev. Depth (ft)	Sample Type	Sample Number	Sample Length (in)	Blows per 6 inches	Sample Recovery (in)	N-Value	Organic Vapor Reading (ppm)	Graphic Log	Strata	Material Description	Remarks		
143.1													
0										5" Asphalt			
	SS	S-1	18	14 14 11	14	25	0.0		Sand	Moist, medium dense, brown, fine to coarse SAND, little fine gravel, little silt			
	SS	S-2	24	5 5 8 8	12	13	0.0			Moist, medium dense, brown, fine SAND, little silt, little fine gravel, trace organics			
				3 5 2 2						Moist, loose, brown, fine SAND, little silt			
138.1 5	SS	S-3	24	4 3 2 3	14	7	0.0			Moist, loose, dark brown, fine SAND, some silt, trace fine gravel, trace organics			
	SS	S-4	24	3 8 14 15	12	5	0.0			Moist, medium dense, brown, fine to coarse SAND, little fine gravel, trace silt			
	SS	S-5	24		16	22	0.0						
133.1 10										End of exploration at 10 ft bgs.			
128.1 15													
123.1													
Sample Types						Consistency vs Blowcount/Foot					Burmister Classification		
AS - Auger/Grab Sample CS - California Sampler BQ - 1.5" Rock Core NX - 2" Rock Core		HP - Hydro Punch SS - Split Spoon ST - Shelby Tube WS - Wash Sample GP - Geoprobe				Granular (Sand): V. Loose: 0-4    Dense: 30-50 Loose: 4-10    V. Dense: >50 M. Dense: 10-30			Fine Grained (Clay): V. Soft: <2    Stiff: 8-15 Soft: 2-4    V. Stiff: 15-30 M. Stiff: 4-8    Hard: >30			and 35-50% some 20-35% little 10-20% trace <10% moisture, density, color	
Reviewed by: T. Dunn										Date: 11/15/2017		Boring Number: B-27	



# Boring Number: B-28

**Client:** Town of Salem

**Project Name:** S. Broadway Watermain Replacement

**Project Location:** Salem, NH

**Project Number:** 25972-215427

**Drilling Contractor/Driller:** New England Boring Contractors / P. Schofield

**Surface Elevation (ft.):** 136.2

**Drilling Method/Casing/Core Barrel Size:** Hollow Stem Auger / 4 in / NA

**Total Depth (ft.):** 10

**Hammer Weight/Drop Height/ Spoon Size:** 140 lb / 30 in / 2 in O.D.

**Depth to Initial Water Level (ft):**
**Bore Hole Location:** See boring location plan

**Depth Date Time**
**Drilling Date: Start:** 9/18/2017 **End:** 9/18/2017

NE 9/18/2017 1:55 pm

**N:** 103142.89 **E:** 1102177.13

**Abandonment Method:** Backfilled with soil cuttings.

**Logged By:** J. Todd

Elev. Depth (ft)	Sample Type	Sample Number	Sample Length (in)	Blows per 6 inches	Sample Recovery (in)	N-Value	Organic Vapor Reading (ppm)	Graphic Log	Strata	Material Description	Remarks
136.2 0	SS	S-1	24	3 6 8 8	16	14	0.0		Sand	Moist, medium dense, brown, Slightly Organic fine to medium SAND, little clayey silt, trace fine gravel	Grass was observed at ground surface.
	SS	S-2	24	6 8 7 7	14	15	0.0			Moist, medium dense, light brown, fine SAND, little fine gravel, little silt	
131.2 5	SS	S-3	24	6 8 8 6	11	16	0.0			Moist, medium dense, light brown, fine SAND, little fine gravel, little silt	
	SS	S-4	24	9 10 12 16	18	22	0.0			Moist, medium dense, light brown, fine SAND, little fine gravel, little silt	
	SS	S-5	24	10 11 24 30	15	35	0.0			Moist, dense, light brown, fine SAND, some fine to coarse gravel, little silt	
126.2 10										End of exploration at 10 ft bgs.	
121.2 15											
116.2											

## Sample Types

AS - Auger/Grab Sample  
CS - California Sampler  
BQ - 1.5" Rock Core  
NX - 2" Rock Core  
HP - Hydro Punch  
SS - Split Spoon  
ST - Shelby Tube  
WS - Wash Sample  
GP - Geoprobe

## Consistency vs Blowcount/Foot

**Granular (Sand):**  
V. Loose: 0-4 Dense: 30-50  
Loose: 4-10 V. Dense: >50  
M. Dense: 10-30

**Fine Grained (Clay):**  
V. Soft: <2 Stiff: 8-15  
Soft: 2-4 V. Stiff: 15-30  
M. Stiff: 4-8 Hard: >30

## Burmister Classification

and 35-50%  
some 20-35%  
little 10-20%  
trace <10%  
moisture, density, color

**Reviewed by:** T. Dunn

**Date:** 11/15/2017

**Boring Number:** B-28



# Boring Number: B-29

**Client:** Town of Salem  
**Project Location:** Salem, NH

**Project Name:** S. Broadway Watermain Replacement  
**Project Number:** 25972-215427

**Drilling Contractor/Driller:** New England Boring Contractors / P. Schofield

**Surface Elevation (ft.):**

**Drilling Method/Casing/Core Barrel Size:** Drive and Wash / 4 in / NA

**Total Depth (ft.):** 7

**Hammer Weight/Drop Height/ Spoon Size:** 140 lb / 30 in / 2 in O.D.

**Depth to Initial Water Level (ft):**

**Bore Hole Location:** See boring location plan

**Depth Date Time**

**Drilling Date: Start:** 9/22/2017 **End:** 9/22/2017

NE 9/22/2017 2:50 pm

**N: E:**

**Abandonment Method:** Backfilled with soil cuttings.

**Logged By:** A. Smith

Elev. Depth (ft)	Sample Type	Sample Number	Sample Length (in)	Blows per 6 inches	Sample Recovery (in)	N-Value	Organic Vapor Reading (ppm)	Graphic Log	Strata	Material Description	Remarks
0										4" Asphalt	
	SS	S-1	24	19 46 40 38	14	86	0.0		Gravelly Sand	Moist, very dense, gray-brown, fine to coarse SAND and fine to coarse GRAVEL, little silt	Obtained sample of subbase soil beneath asphalt, appears similar to sample S-1.
	SS	S-2	24	60 63 38 32	12	101	0.0			Moist, very dense, gray, fine to coarse SAND, some fine to coarse gravel, little silt	
5	SS	S-3	24	20 15 21 34	16	36	0.0			Moist, dense, gray, fine to coarse SAND, some fine to coarse gravel, little silt	
										End of exploration at 7 ft bgs.	
10											
15											

**Sample Types****Consistency vs Blowcount/Foot****Burmister Classification**

AS - Auger/Grab Sample  
CS - California Sampler  
BQ - 1.5" Rock Core  
NX - 2" Rock Core

HP - Hydro Punch  
SS - Split Spoon  
ST - Shelby Tube  
WS - Wash Sample  
GP - Geoprobe

**Granular (Sand):**

V. Loose: 0-4 Dense: 30-50  
Loose: 4-10 V. Dense: >50  
M. Dense: 10-30

**Fine Grained (Clay):**

V. Soft: <2 Stiff: 8-15  
Soft: 2-4 V. Stiff: 15-30  
M. Stiff: 4-8 Hard: >30

and 35-50%  
some 20-35%  
little 10-20%  
trace <10%  
moisture, density, color

**Reviewed by:** T. Dunn

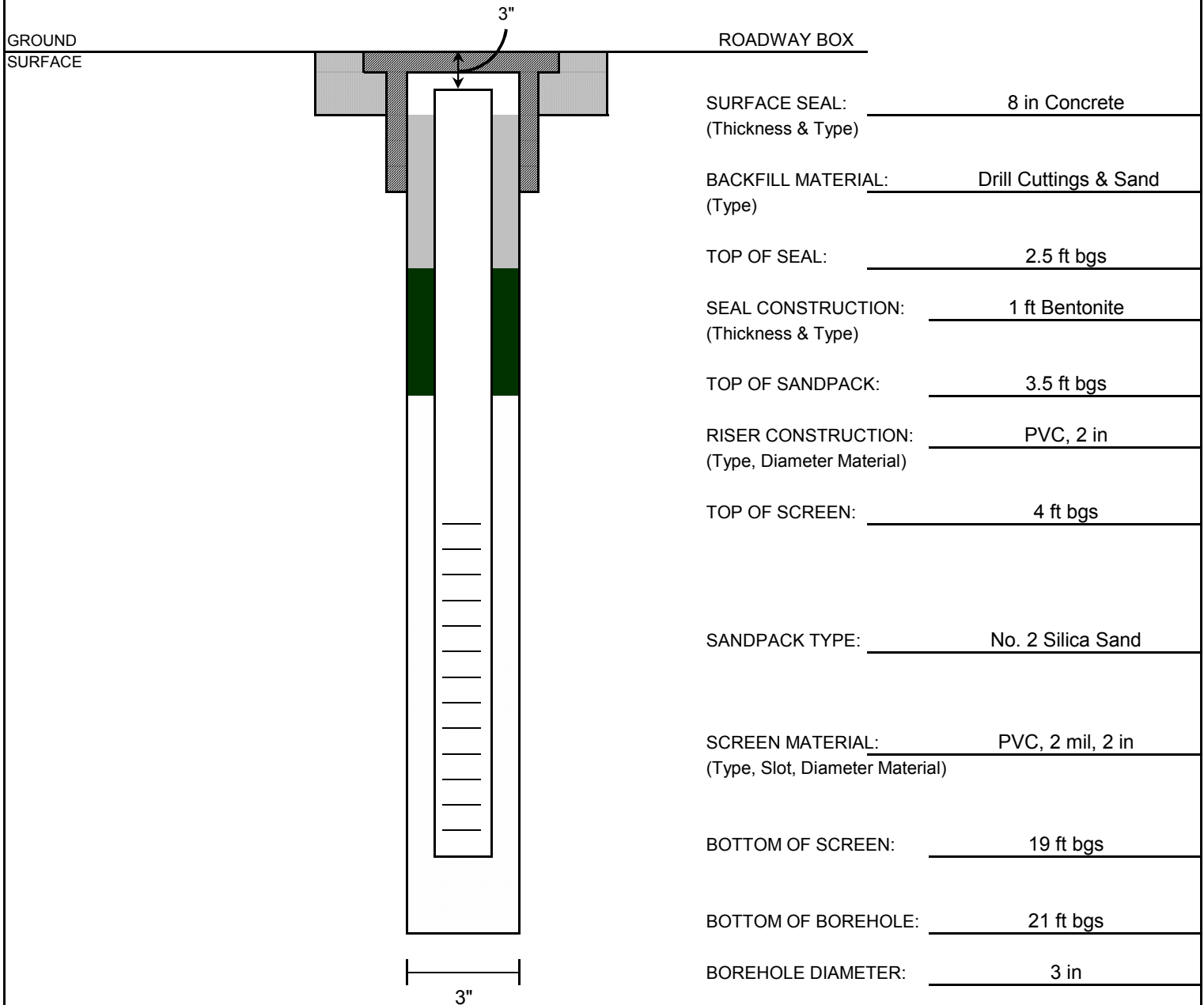
**Date:** 11/15/2017

**Boring Number:** B-29

**Attachment B – Monitoring Well Installation Logs and Monitoring Well Reports**

## Monitoring Well Installation Log

Client:	Town of Salem, NH	Contractor:	New England Boring Contractors	Boring/Well No.:	B-2 (MW)
Project Name:	S. Broadway Water Main Replacement	Driller:	B. Raiche	Date Installed:	4/12/2017
Project Location:	Salem, NH	Ground EL:	135.0	Logged By:	J. Todd
Project Number:	25972-215427	Riser EL:	134.8	Page:	1 of 2



**NOTE:** All depths are in feet below ground surface, unless noted otherwise.

Remarks: Monitoring well developed on 4/12/17 at 9:30 am.

# Monitoring Well Report

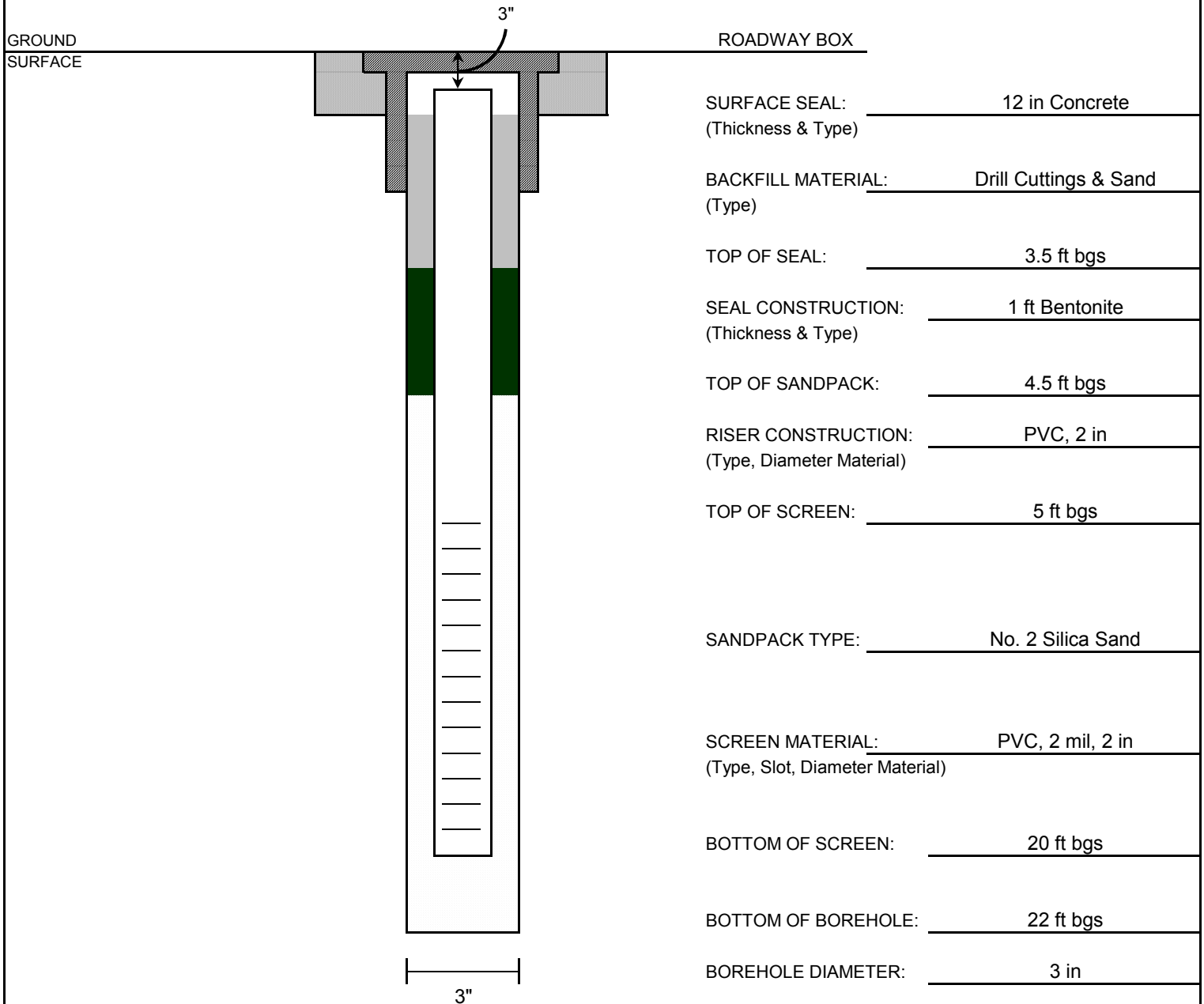
75 State Street  
Boston, MA 02109  
(617) 452-6000

[illegible]

Remarks:

## Monitoring Well Installation Log

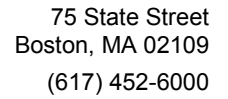
Client:	Town of Salem, NH	Contractor:	New England Boring Contractors	Boring/Well No.:	B-3 (MW)
Project Name:	S. Broadway Water Main Replacement	Driller:	B. Raiche	Date Installed:	4/11/2017
Project Location:	Salem, NH	Ground EL:	133.3	Logged By:	J. Todd
Project Number:	25972-215427	Riser EL:	133.1	Page:	1 of 2



**NOTE:** All depths are in feet below ground surface, unless noted otherwise.

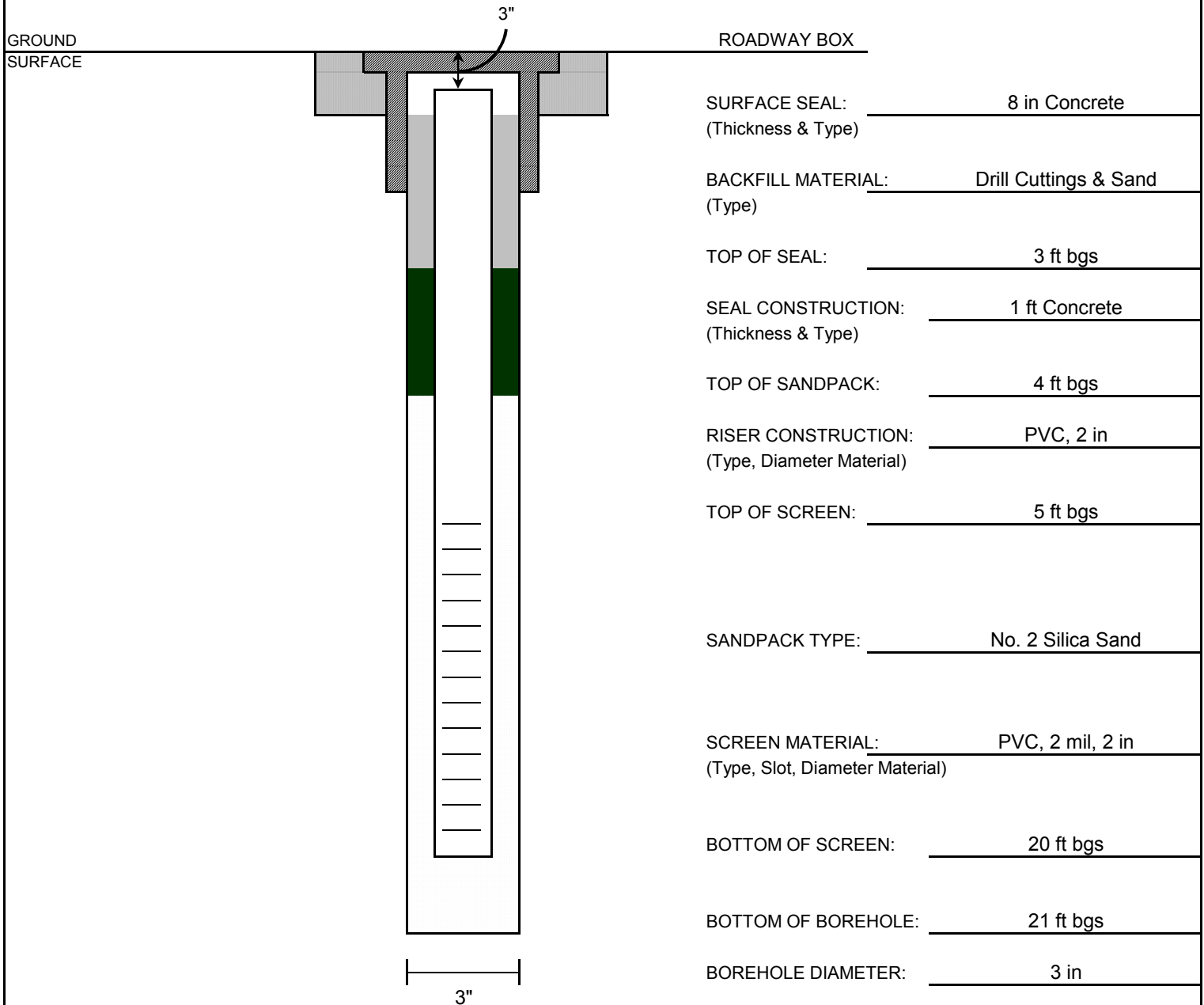
Remarks: Monitoring well developed on 4/11/17 at 12:00 pm.



[illegible]

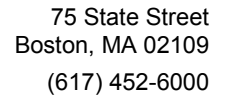
## Monitoring Well Installation Log

Client:	Town of Salem, NH	Contractor:	New England Boring Contractors	Boring/Well No.:	B-4 (MW)
Project Name:	S. Broadway Water Main Replacement	Driller:	B. Raiche	Date Installed:	4/10/2017
Project Location:	Salem, NH	Ground EL:	131.2	Logged By:	J. Todd
Project Number:	25972-215427	Riser EL:	131.0	Page:	1 of 2



**NOTE:** All depths are in feet below ground surface, unless noted otherwise.

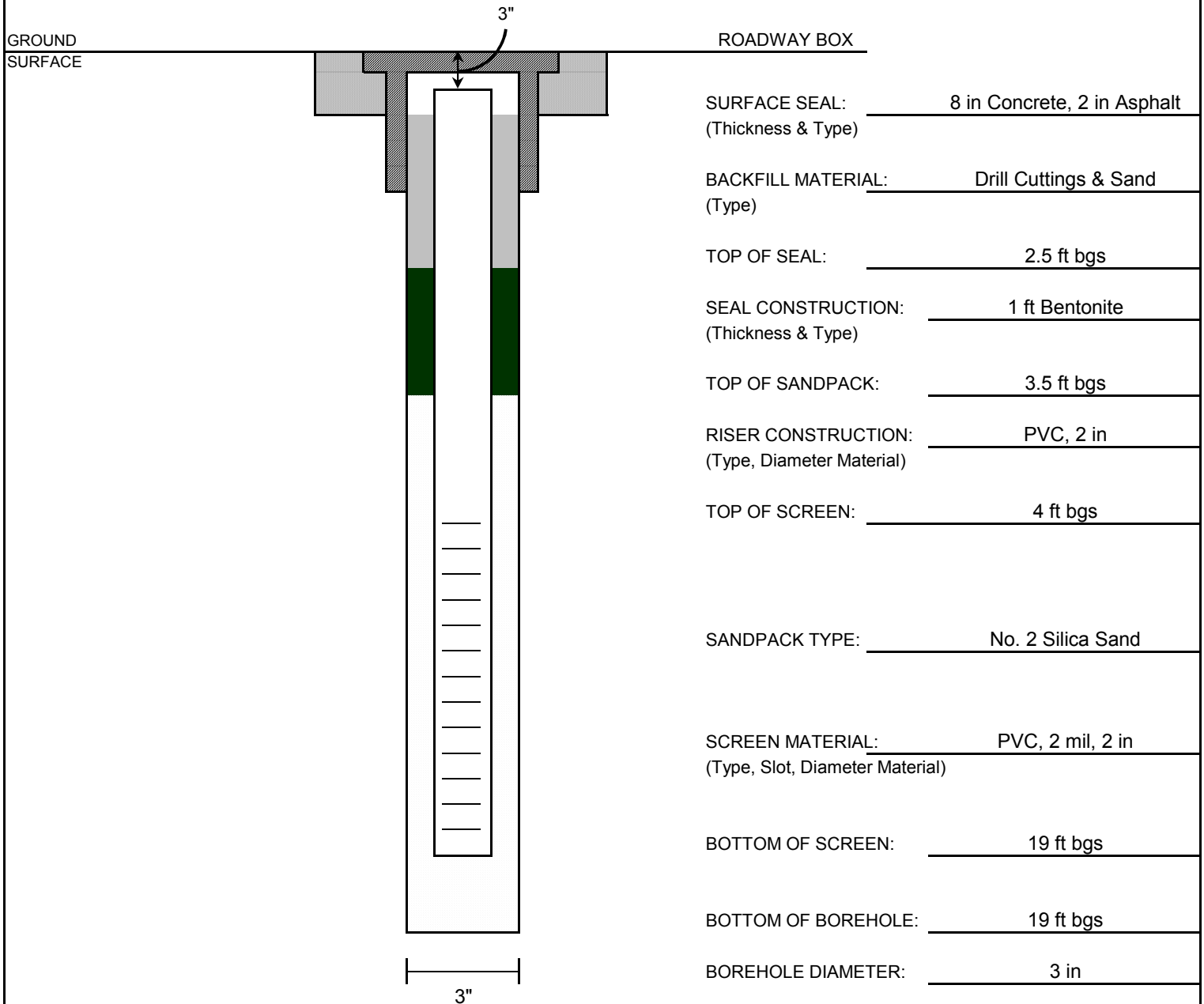
Remarks: Monitoring well developed on 4/10/17 at 2:15 pm.

[illegible]

Updated On: 04/09/01

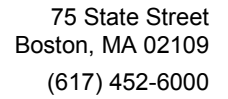
## Monitoring Well Installation Log

Client:	Town of Salem, NH	Contractor:	New England Boring Contractors	Boring/Well No.:	B-11 (MW)
Project Name:	S. Broadway Water Main Replacement	Driller:	B. Raiche	Date Installed:	4/13/2017
Project Location:	Salem, NH	Ground EL:	126.4	Logged By:	J. Todd
Project Number:	25972-215427	Riser EL:	126.2	Page:	1 of 2



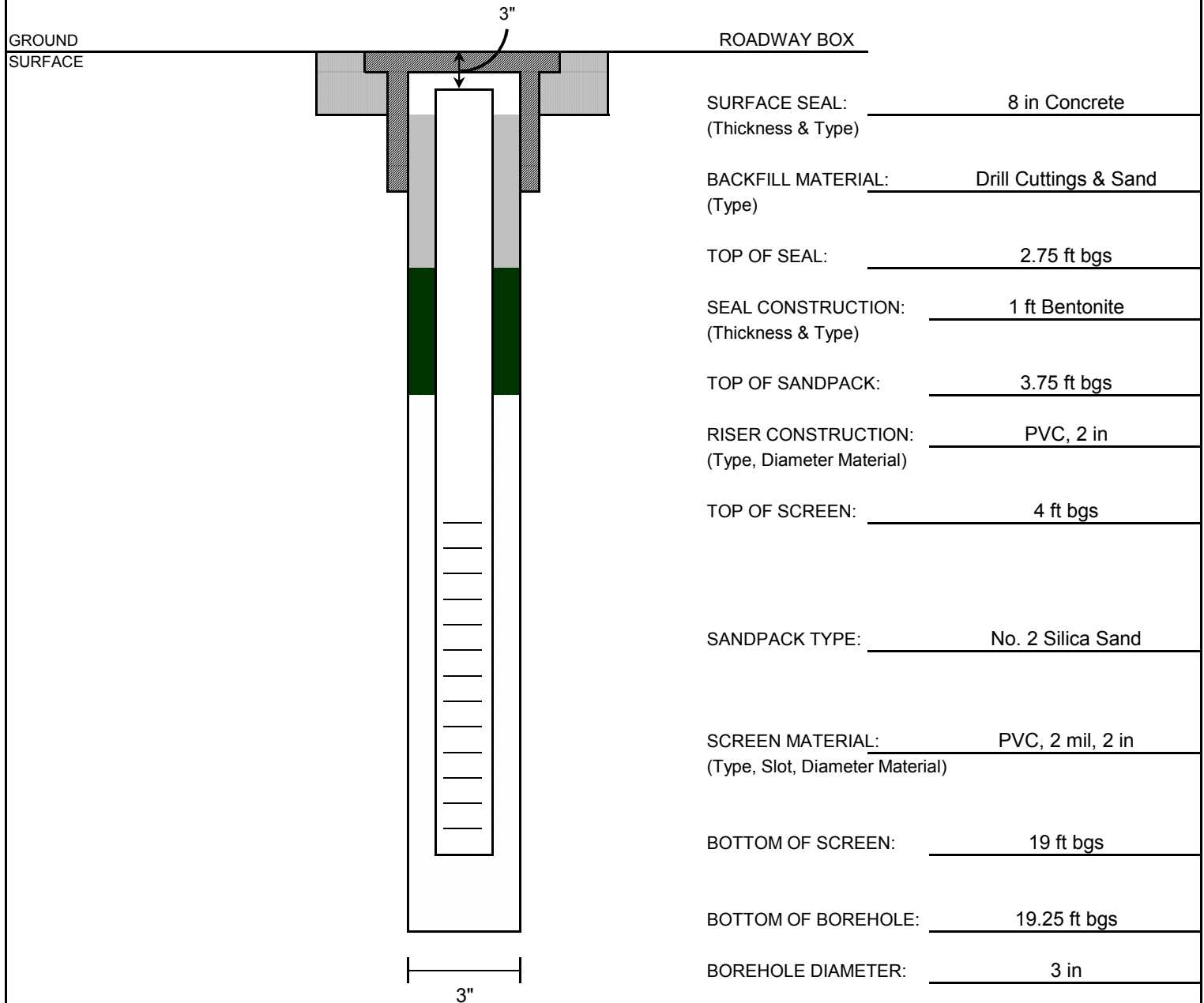
**NOTE:** All depths are in feet below ground surface, unless noted otherwise.

Remarks: Monitoring well developed on 4/13/17 at 9:00 am.

[illegible]

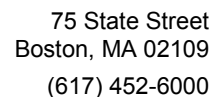
## Monitoring Well Installation Log

Client:	Town of Salem, NH	Contractor:	New England Boring Contractors	Boring/Well No.:	B-15 (MW)
Project Name:	S. Broadway Water Main Replacement	Driller:	B. Raiche	Date Installed:	4/13/2017
Project Location:	Salem, NH	Ground EL:	125.9	Logged By:	J. Todd
Project Number:	25972-215427	Riser EL:	125.7	Page:	1 of 2



**NOTE:** All depths are in feet below ground surface, unless noted otherwise.

Remarks: Monitoring well developed on 4/13/17 at 2:00 pm.

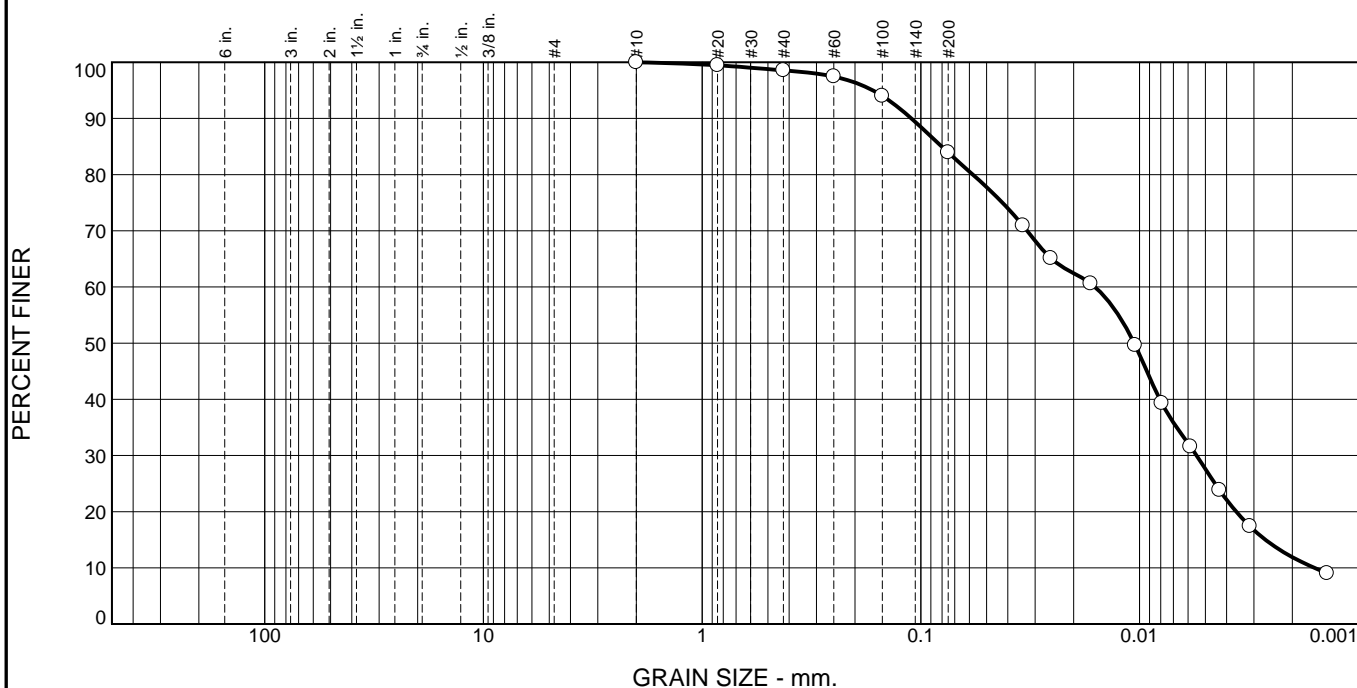
[illegible]

Updated On: 04/09/01

## **Attachment C – Geotechnical Laboratory Testing Results**



# Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	0.0	0.0	1.4	14.7	56.3	27.6

Test Results (ASTM D 422 & ASTM D 1140)			
Opening Size	Percent Finer	Spec.* (Percent)	Pass? (X=Fail)
#10	100.0		
#20	99.5		
#40	98.6		
#60	97.5		
#100	94.0		
#200	83.9		
0.0341 mm.	70.9		
0.0255 mm.	65.1		
0.0167 mm.	60.6		
0.0105 mm.	49.7		
0.0079 mm.	39.3		
0.0059 mm.	31.6		
0.0043 mm.	23.9		
0.0031 mm.	17.4		
0.0014 mm.	9.0		

\* (no specification provided)

**Material Description**  
Light brown and gray silt with sand

**Atterberg Limits (ASTM D 4318)**  
PL=                      LL=                      PI=

**Classification**  
USCS (D 2487)= ML                      AASHTO (M 145)= A-4(0)

**Coefficients**  
D<sub>90</sub>= 0.1105                      D<sub>85</sub>= 0.0802                      D<sub>60</sub>= 0.0160  
D<sub>50</sub>= 0.0106                      D<sub>30</sub>= 0.0055                      D<sub>15</sub>= 0.0027  
D<sub>10</sub>= 0.0016                      C<sub>u</sub>= 10.09                      C<sub>c</sub>= 1.19

**Remarks**  
As received MC = 22.0%

**Date Received:** 10/4/2017                      **Date Tested:** 10/17/2017  
**Tested By:** GW  
**Checked By:** MP  
**Title:** Laboratory Manager

**Source of Sample:** B-1                      **Depth:** 8-10'  
**Sample Number:** S-5

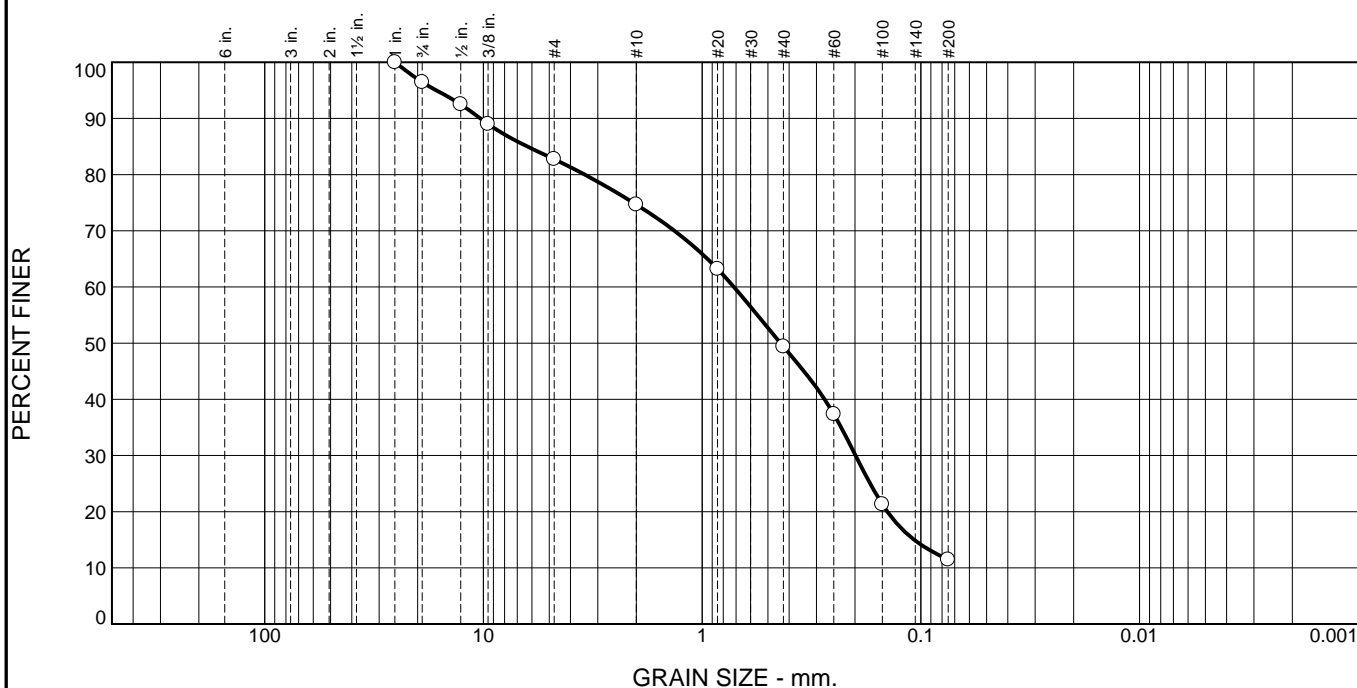
**Date Sampled:** 9/25/2017

**CDM Smith**  
**Boston, Massachusetts**

**Client:** Town of Salem  
**Project:** South Broadway Watermain Replacement  
Salem, NH  
**Project No:** 25972-215427

**Figure**

# Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	3.5	13.8	8.0	25.3	37.9	11.5	

Test Results (ASTM D422 & ASTM D1140)			
Opening Size	Percent Finer	Spec.* (Percent)	Pass? (X=Fail)
1"	100.0		
.75"	96.5		
.5"	92.5		
.375"	89.0		
#4	82.7		
#10	74.7		
#20	63.2		
#40	49.4		
#60	37.4		
#100	21.3		
#200	11.5		

\* (no specification provided)

**Material Description**  
Brown poorly graded sand with silt and gravel

**Atterberg Limits (ASTM D 4318)**  
 PL= 0      LL=      PI=

**Classification**  
 USCS (D 2487)= SP-SM      AASHTO (M 145)= A-1-b

**Coefficients**  
 D<sub>90</sub>= 10.3351      D<sub>85</sub>= 6.2960      D<sub>60</sub>= 0.7157  
 D<sub>50</sub>= 0.4382      D<sub>30</sub>= 0.1987      D<sub>15</sub>= 0.1073  
 D<sub>10</sub>=      C<sub>u</sub>=      C<sub>c</sub>=

**Remarks**  
As received MC = 12.9%

**Date Received:** 10/4/2017      **Date Tested:** 10/16/2017  
**Tested By:** GW  
**Checked By:** MP  
**Title:** Laboratory Manager

**Source of Sample:** B-1      **Depth:** 14-16'  
**Sample Number:** S-6

**Date Sampled:** 9/25/2017

**CDM Smith**  
  
**Boston, Massachusetts**

**Client:** Town of Salem  
**Project:** South Broadway Watermain Replacement  
 Salem, NH  
**Project No:** 25972-215427

**Figure**

# CDM Smith

## Geotechnical Engineering Laboratory

### Standard Test Method for Moisture, Ash, and Organic Matter of Peat and Other Organic Soils (ASTM D2974)

Client: Town of Salem  
Project Name: South Broadway WM Replacement  
Project Location: Salem, NH  
Project Number: 25972-215427  
Boring Number: B-2  
Sample Number: S-4  
Sample Depth (ft): 6-8  
Sample Date: 4/11-12/2017

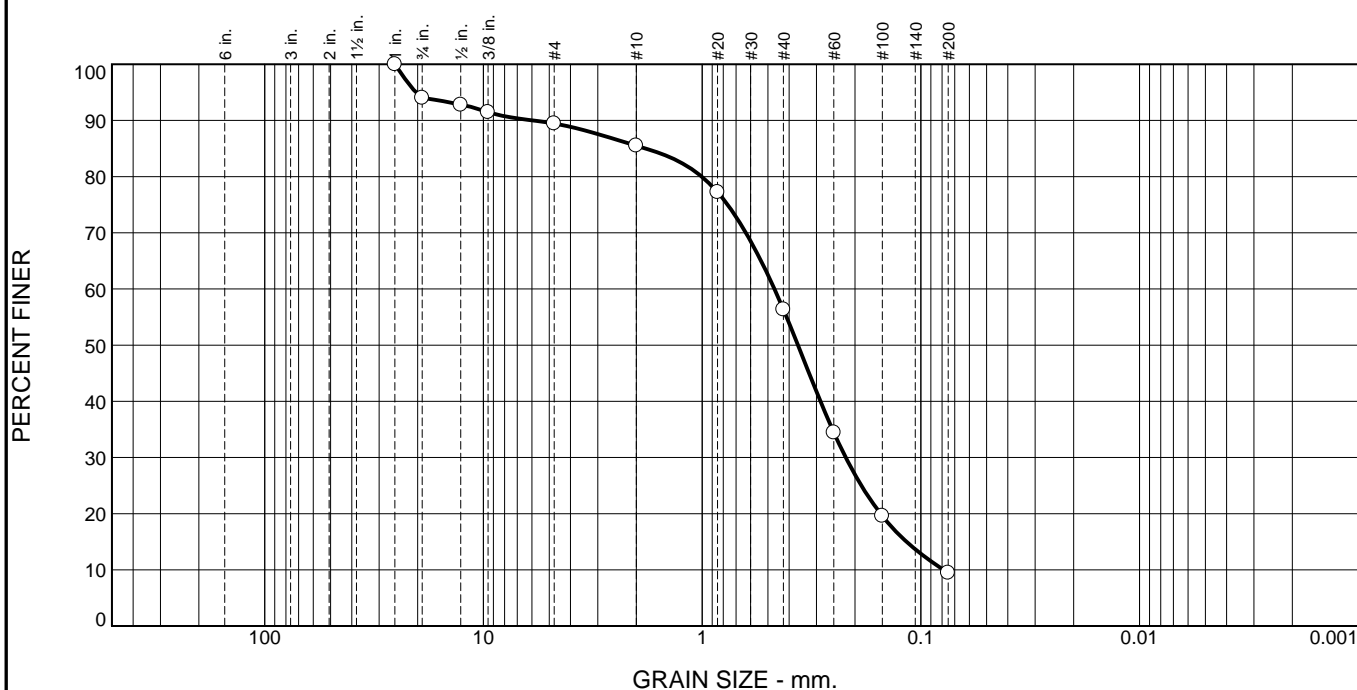
Tested By: MP  
Test Date: 10/16/2017

Procedure: C  
Temperature: 440° C

AS RECEIVED MOISTURE CONTENT	
Tin Mass (g)	126.56
Wet Mass of Sample & Tin (g)	164.78
Dry Mass of Sample & Tin (g)	145.97
Mass of Water (g)	18.81
Mass of Dry Soil (g)	19.41
<b>Moisture Content (%)</b>	<b>96.9</b>

ASH CONTENT	
Porcelain Dish Mass (g)	126.56
Porcelain Dish + Oven Dried Soil (g)	145.97
Mass of Oven Dried Soil (g)	19.41
Mass of Dish & Burned Soil (g)	143.15
Mass of Burned Soil (g)	16.59
Mass of Organic Material (g)	2.82
Ash Content (%)	85.5
<b>Organic Content (%)</b>	<b>14.5</b>

# Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	6.0	4.6	3.9	29.2	46.8	9.5	

Test Results (ASTM D422 & ASTM D1140)			
Opening Size	Percent Finer	Spec.* (Percent)	Pass? (X=Fail)
1"	100.0		
.75"	94.0		
.5"	92.8		
.375"	91.5		
#4	89.4		
#10	85.5		
#20	77.2		
#40	56.3		
#60	34.4		
#100	19.6		
#200	9.5		

\* (no specification provided)

**Material Description**  
Dark brown poorly graded sand with silt

**Atterberg Limits (ASTM D 4318)**  
 PL=                      LL=                      PI=  
**Classification**  
 USCS (D 2487)= SP-SM      AASHTO (M 145)= A-3  
**Coefficients**  
 D<sub>90</sub>= 5.9533      D<sub>85</sub>= 1.8146      D<sub>60</sub>= 0.4671  
 D<sub>50</sub>= 0.3646      D<sub>30</sub>= 0.2204      D<sub>15</sub>= 0.1162  
 D<sub>10</sub>= 0.0787      C<sub>u</sub>= 5.94      C<sub>c</sub>= 1.32

**Remarks**  
As received MC = 20.0%

**Date Received:** 10/4/2017      **Date Tested:** 10/16/2017  
**Tested By:** GW  
**Checked By:** MP  
**Title:** Laboratory Manager

**Source of Sample:** B-2      **Depth:** 8-10'  
**Sample Number:** S-5

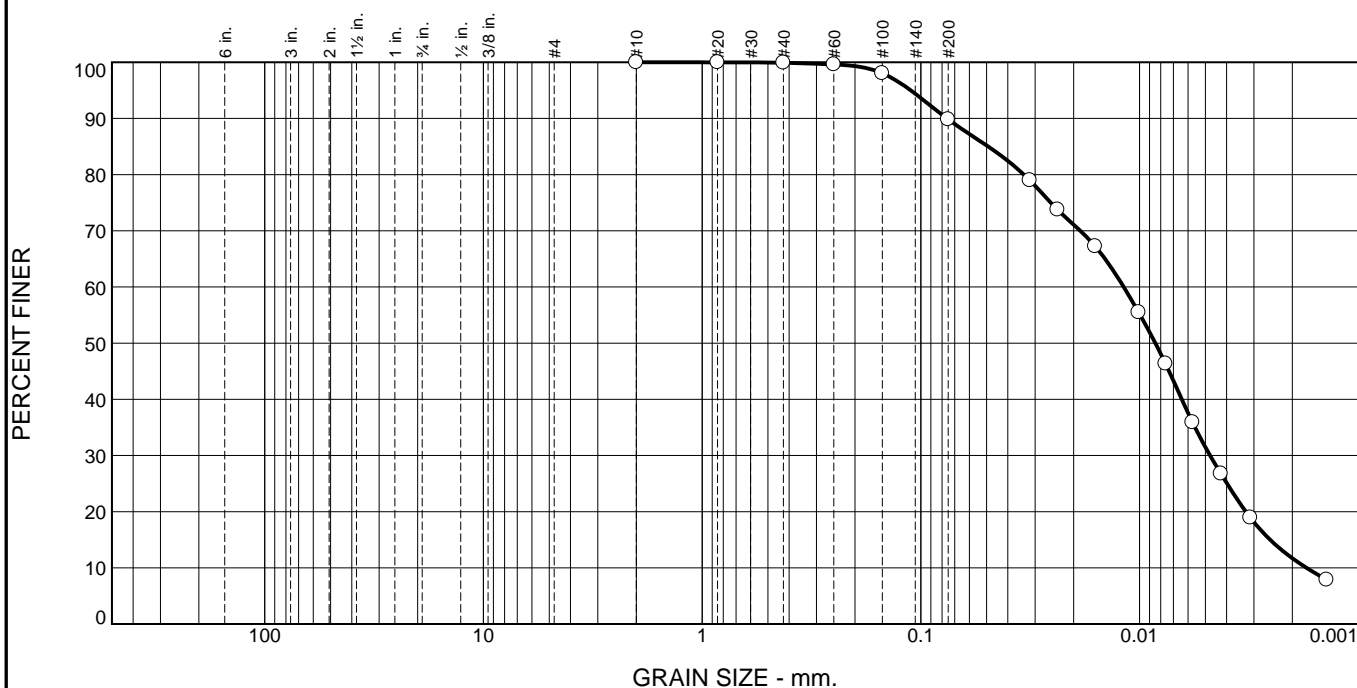
**Date Sampled:** 4/11-12/2017

**CDM Smith**  
  
**Boston, Massachusetts**

**Client:** Town of Salem  
**Project:** South Broadway Watermain Replacement  
 Salem, NH  
**Project No:** 25972-215427

**Figure**

# Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	0.0	0.0	0.1	10.1	58.3	31.5

Test Results (ASTM D 422 & ASTM D 1140)			
Opening Size	Percent Finer	Spec.* (Percent)	Pass? (X=Fail)
#10	100.0		
#20	100.0		
#40	99.9		
#60	99.6		
#100	98.1		
#200	89.8		
0.0317 mm.	79.0		
0.0237 mm.	73.8		
0.0159 mm.	67.2		
0.0101 mm.	55.5		
0.0076 mm.	46.3		
0.0057 mm.	35.9		
0.0042 mm.	26.8		
0.0031 mm.	18.9		
0.0014 mm.	7.9		

\* (no specification provided)

**Material Description**  
Light brown silt

**Atterberg Limits (ASTM D 4318)**  
 PL=                      LL=                      PI=

**Classification**  
 USCS (D 2487)= ML                      AASHTO (M 145)= A-4(0)

**Coefficients**  
 D<sub>90</sub>= 0.0759                      D<sub>85</sub>= 0.0492                      D<sub>60</sub>= 0.0118  
 D<sub>50</sub>= 0.0085                      D<sub>30</sub>= 0.0048                      D<sub>15</sub>= 0.0025  
 D<sub>10</sub>= 0.0017                      C<sub>u</sub>= 6.86                      C<sub>c</sub>= 1.11

**Remarks**  
As received MC = 26.5%

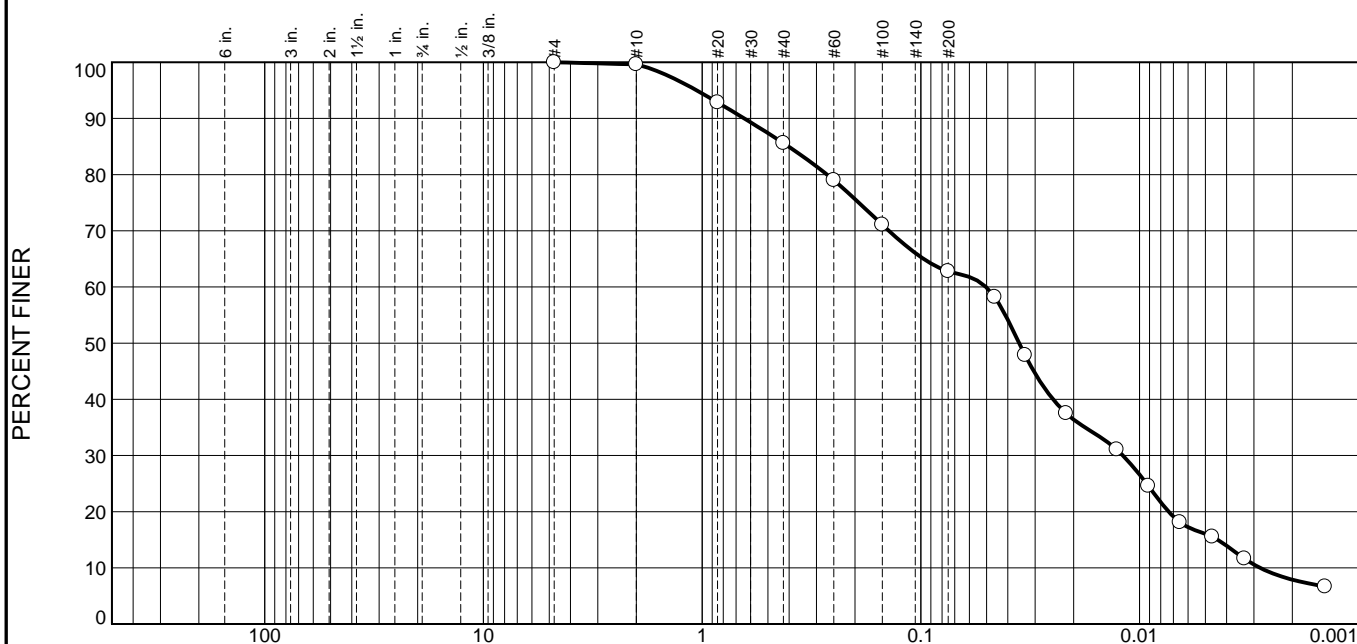
**Date Received:** 10/4/2017                      **Date Tested:** 10/17/2017  
**Tested By:** GW  
**Checked By:** MP  
**Title:** Laboratory Manager

**Source of Sample:** B-2                      **Depth:** 14-16'  
**Sample Number:** S-6

**Date Sampled:** 4/11-12/2017

<b>CDM Smith</b>  <b>Boston, Massachusetts</b>	<b>Client:</b> Town of Salem
	<b>Project:</b> South Broadway Watermain Replacement Salem, NH
<b>Project No:</b> 25972-215427	<b>Figure</b>

# Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	0.0	0.4	14.0	22.8	46.8	16.0

Test Results (ASTM D 422 & ASTM D 1140)			
Opening Size	Percent Finer	Spec.* (Percent)	Pass? (X=Fail)
#4	100.0		
#10	99.6		
#20	92.8		
#40	85.6		
#60	79.0		
#100	71.1		
#200	62.8		
0.0460 mm.	58.2		
0.0334 mm.	47.9		
0.0217 mm.	37.5		
0.0127 mm.	31.1		
0.0091 mm.	24.6		
0.0065 mm.	18.1		
0.0047 mm.	15.5		
0.0033 mm.	11.6		
0.0014 mm.	6.7		

\* (no specification provided)

<b>Material Description</b>	
Black and brown sandy organic silt	
<b>Atterberg Limits (ASTM D 4318)</b>	
PL=	LL= PI=
<b>Classification</b>	
USCS (D 2487)= OL/ML	AASHTO (M 145)= A-4(0)
<b>Coefficients</b>	
D <sub>90</sub> = 0.6416	D <sub>85</sub> = 0.4030 D <sub>60</sub> = 0.0503
D <sub>50</sub> = 0.0355	D <sub>30</sub> = 0.0119 D <sub>15</sub> = 0.0044
D <sub>10</sub> = 0.0028	C <sub>u</sub> = 17.92 C <sub>c</sub> = 1.00
<b>Remarks</b>	
As received MC = 92.3%	
<b>Date Received:</b> 10/4/2017 <b>Date Tested:</b> 10/17/2017	
<b>Tested By:</b> GW	
<b>Checked By:</b> MP	
<b>Title:</b> Laboratory Manager	

Source of Sample: B-3 Depth: 6-8'  
Sample Number: S-4

Date Sampled: 4/11/2017

<b>CDM Smith</b>  <b>Boston, Massachusetts</b>	<b>Client:</b> Town of Salem <b>Project:</b> South Broadway Watermain Replacement Salem, NH
	<b>Project No:</b> 25972-215427 <b>Figure</b>

# CDM Smith

## Geotechnical Engineering Laboratory

### Standard Test Method for Moisture, Ash, and Organic Matter of Peat and Other Organic Soils (ASTM D2974)

Client: Town of Salem  
Project Name: South Broadway WM Replacement  
Project Location: Salem, NH  
Project Number: 25972-215427  
Boring Number: B-3  
Sample Number: S-4  
Sample Depth (ft): 6-8  
Sample Date: 4/11/2017

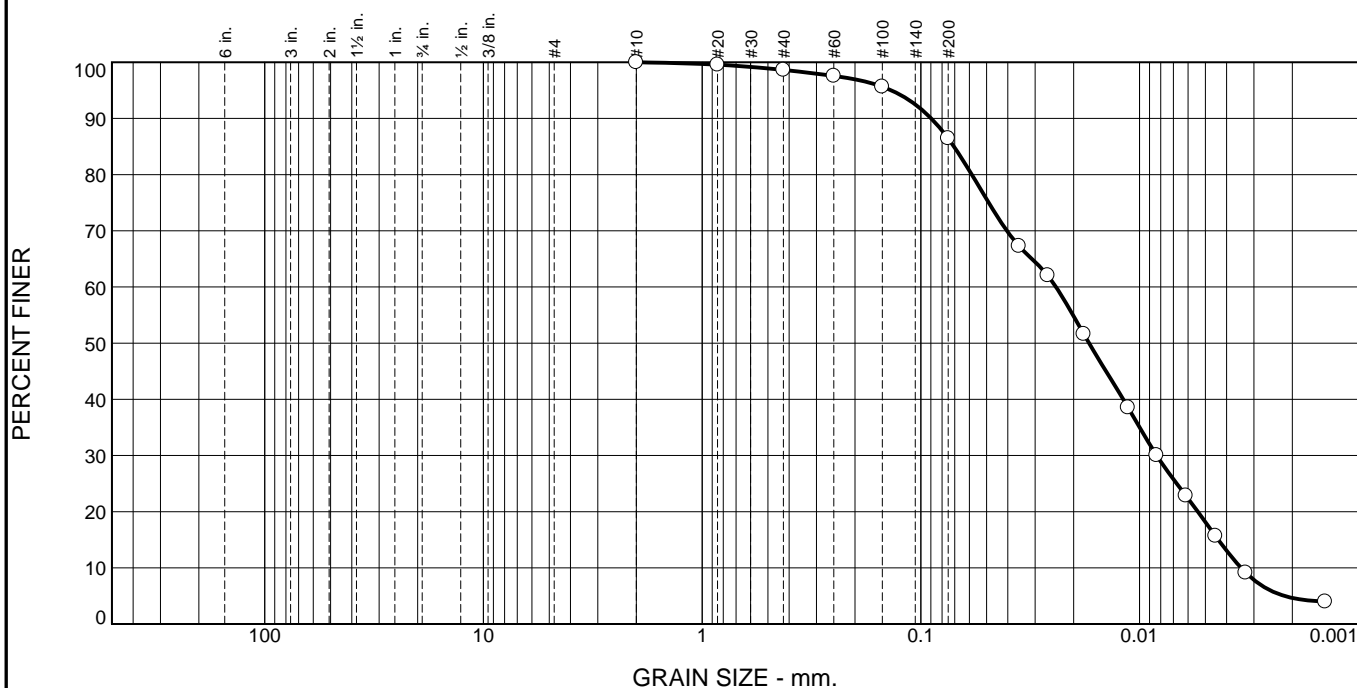
Tested By: MP  
Test Date: 10/21/2017

Procedure: C  
Temperature: 440° C

AS RECEIVED MOISTURE CONTENT	
Tin Mass (g)	90.37
Wet Mass of Sample & Tin (g)	135.78
Dry Mass of Sample & Tin (g)	114.00
Mass of Water (g)	21.78
Mass of Dry Soil (g)	23.63
<b>Moisture Content (%)</b>	<b>92.2</b>

ASH CONTENT	
Porcelain Dish Mass (g)	90.37
Porcelain Dish + Oven Dried Soil (g)	114.00
Mass of Oven Dried Soil (g)	23.63
Mass of Dish & Burned Soil (g)	110.92
Mass of Burned Soil (g)	20.55
Mass of Organic Material (g)	3.08
Ash Content (%)	87.0
<b>Organic Content (%)</b>	<b>13.0</b>

# Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	0.0	0.0	1.4	12.1	68.3	18.2

Test Results (ASTM D 422 & ASTM D 1140)			
Opening Size	Percent Finer	Spec.* (Percent)	Pass? (X=Fail)
#10	100.0		
#20	99.6		
#40	98.6		
#60	97.6		
#100	95.6		
#200	86.5		
0.0356 mm.	67.3		
0.0263 mm.	62.1		
0.0180 mm.	51.6		
0.0113 mm.	38.5		
0.0084 mm.	30.1		
0.0061 mm.	22.9		
0.0045 mm.	15.7		
0.0033 mm.	9.1		
0.0014 mm.	4.0		

\* (no specification provided)

**Material Description**  
Light brown silt

**Atterberg Limits (ASTM D 4318)**  
 PL= \_\_\_\_\_ LL= \_\_\_\_\_ PI= \_\_\_\_\_

**Classification**  
 USCS (D 2487)= ML AASHTO (M 145)= A-4(0)

**Coefficients**  
 D<sub>90</sub>= 0.0896 D<sub>85</sub>= 0.0705 D<sub>60</sub>= 0.0241  
 D<sub>50</sub>= 0.0170 D<sub>30</sub>= 0.0083 D<sub>15</sub>= 0.0044  
 D<sub>10</sub>= 0.0034 C<sub>u</sub>= 7.01 C<sub>c</sub>= 0.84

**Remarks**  
As received MC = 24.0%

**Date Received:** 10/4/2017 **Date Tested:** 10/17/2017  
**Tested By:** GW  
**Checked By:** MP  
**Title:** Laboratory Manager

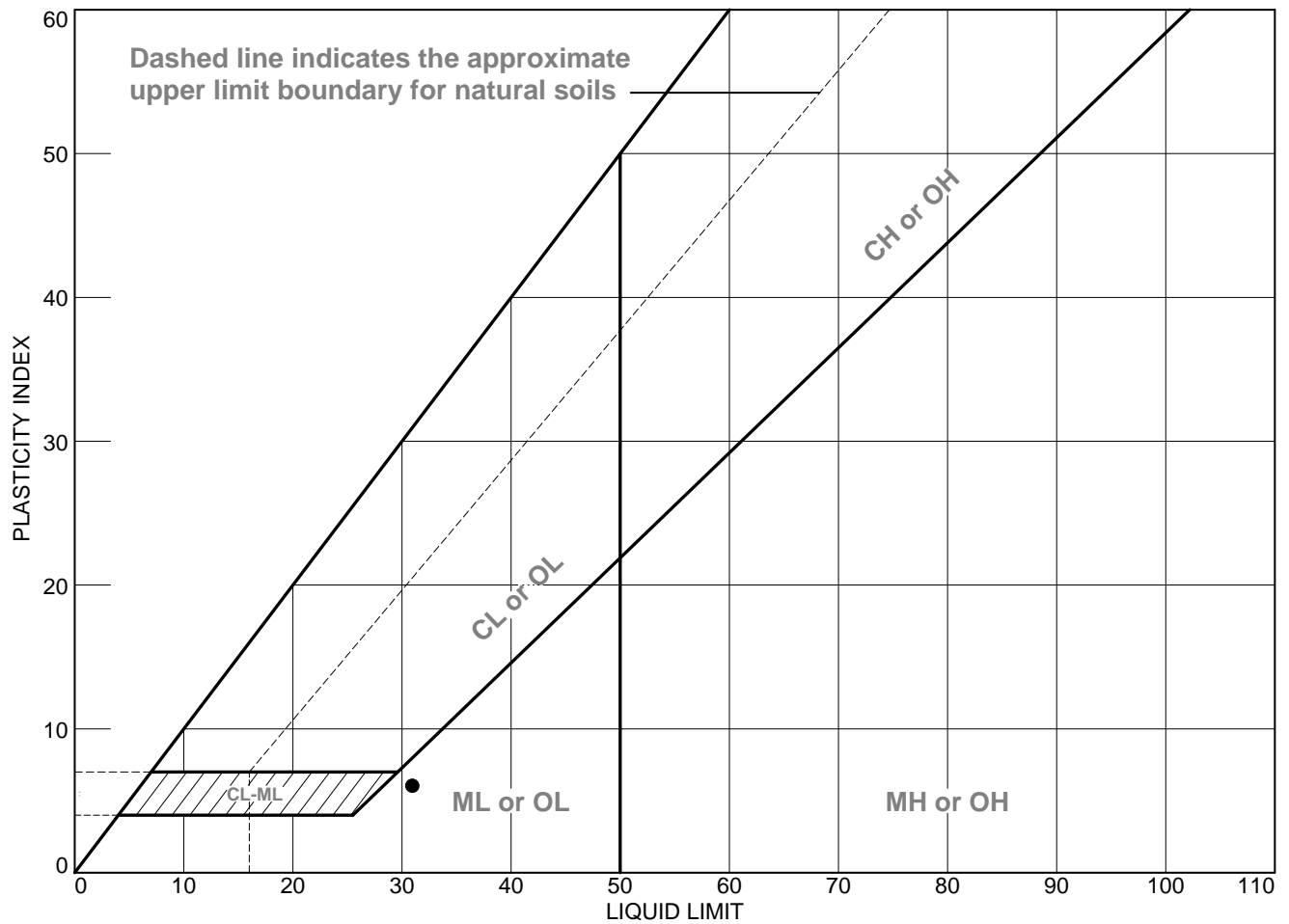
**Source of Sample:** B-4 **Depth:** 8-10'  
**Sample Number:** S-5

**Date Sampled:** 4/10/2017

<b>CDM Smith</b>  <b>Boston, Massachusetts</b>	<b>Client:</b> Town of Salem <b>Project:</b> South Broadway Watermain Replacement Salem, NH
	<b>Project No:</b> 25972-215427 <b>Figure</b>



# LIQUID AND PLASTIC LIMITS TEST REPORT



SOIL DATA								
SYMBOL	SOURCE	SAMPLE NO.	DEPTH	NATURAL WATER CONTENT (%)	PLASTIC LIMIT (%)	LIQUID LIMIT (%)	PLASTICITY INDEX (%)	USCS
●	B-5	S-5	8-10'	25.7	25	31	6	ML

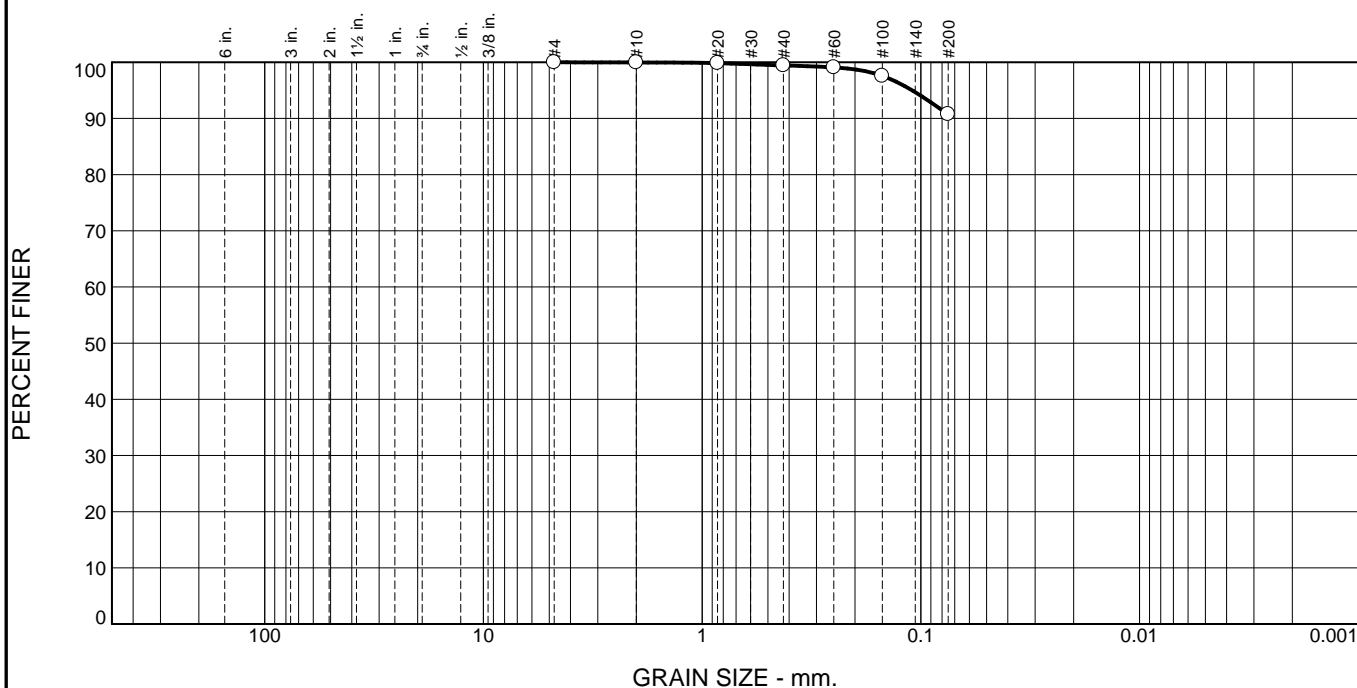
**CDM Smith**  
**Boston, Massachusetts**

**Client:** Town of Salem  
**Project:** South Broadway Watermain Replacement  
Salem, NH  
**Project No.:** 25972-215427

**Figure**

**Tested By:** RZ **Checked By:** MP

# Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	0.0	0.0	0.6	8.7	90.7	

Test Results (ASTM D422 & ASTM D1140)			
Opening Size	Percent Finer	Spec.* (Percent)	Pass? (X=Fail)
#4	100.0		
#10	100.0		
#20	99.8		
#40	99.4		
#60	99.1		
#100	97.6		
#200	90.7		

\* (no specification provided)

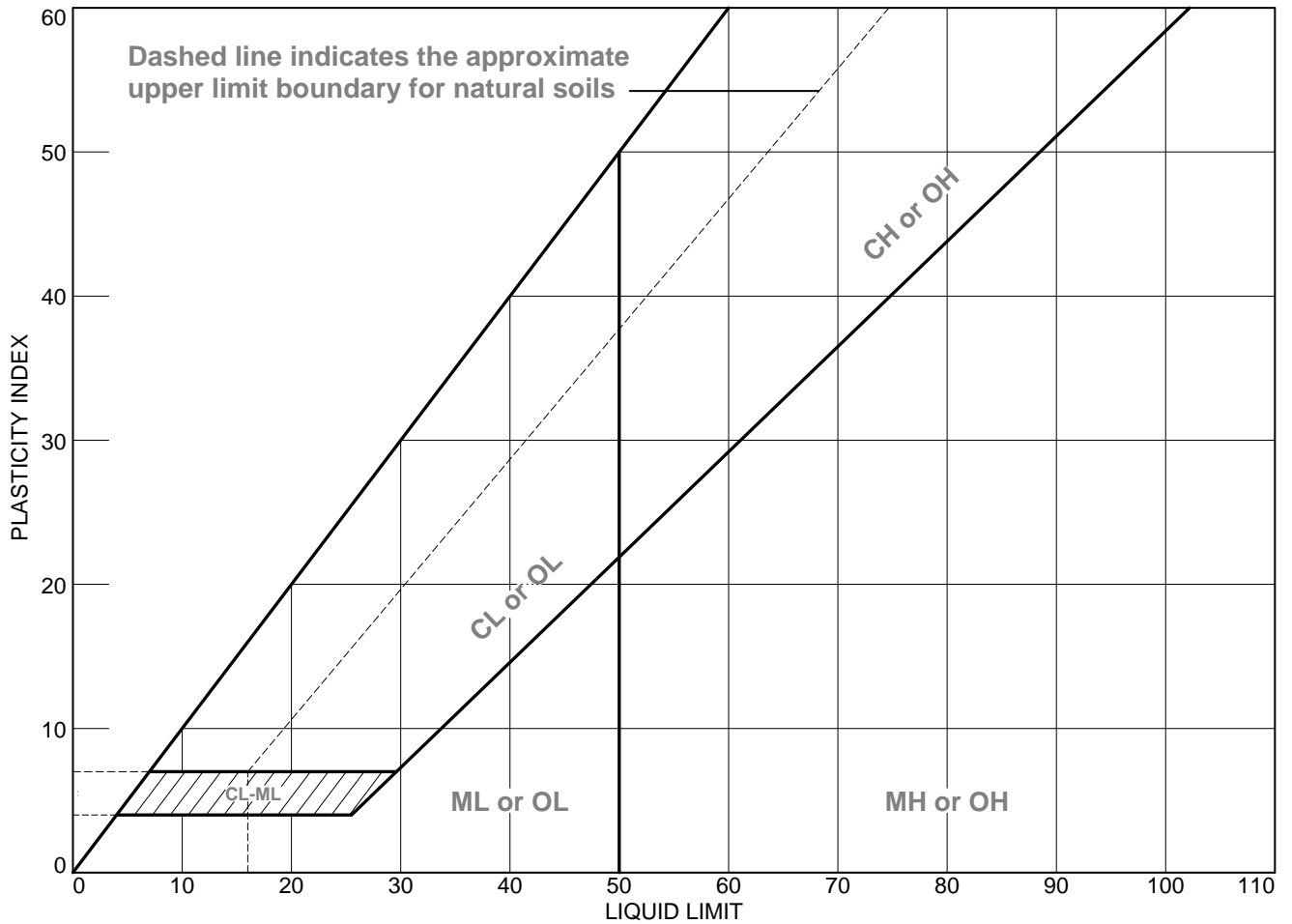
<b>Material Description</b>		
Light brown silt		
<b>Atterberg Limits (ASTM D 4318)</b>		
PL= NP	LL= NV	PI= NP
<b>Classification</b>		
USCS (D 2487)= ML	AASHTO (M 145)= A-4(0)	
<b>Coefficients</b>		
D <sub>90</sub> =	D <sub>85</sub> =	D <sub>60</sub> =
D <sub>50</sub> =	D <sub>30</sub> =	D <sub>15</sub> =
D <sub>10</sub> =	C <sub>u</sub> =	C <sub>c</sub> =
<b>Remarks</b>		
As received MC = 21.1%		
<b>Date Received:</b> 10/4/2017		<b>Date Tested:</b> 10/17/2017
<b>Tested By:</b> RZ		
<b>Checked By:</b> MP		
<b>Title:</b> Laboratory Manager		

Source of Sample: B-5      Depth: 14-16'  
Sample Number: S-6

Date Sampled: 9/26/2017

<b>CDM Smith</b>  <b>Boston, Massachusetts</b>		<b>Client:</b> Town of Salem <b>Project:</b> South Broadway Watermain Replacement Salem, NH <b>Project No:</b> 25972-215427
		<b>Figure</b>

# LIQUID AND PLASTIC LIMITS TEST REPORT



SOIL DATA								
SYMBOL	SOURCE	SAMPLE NO.	DEPTH	NATURAL WATER CONTENT (%)	PLASTIC LIMIT (%)	LIQUID LIMIT (%)	PLASTICITY INDEX (%)	USCS
●	B-5	S-6	14-16'	21.1	NP	NV	NP	ML

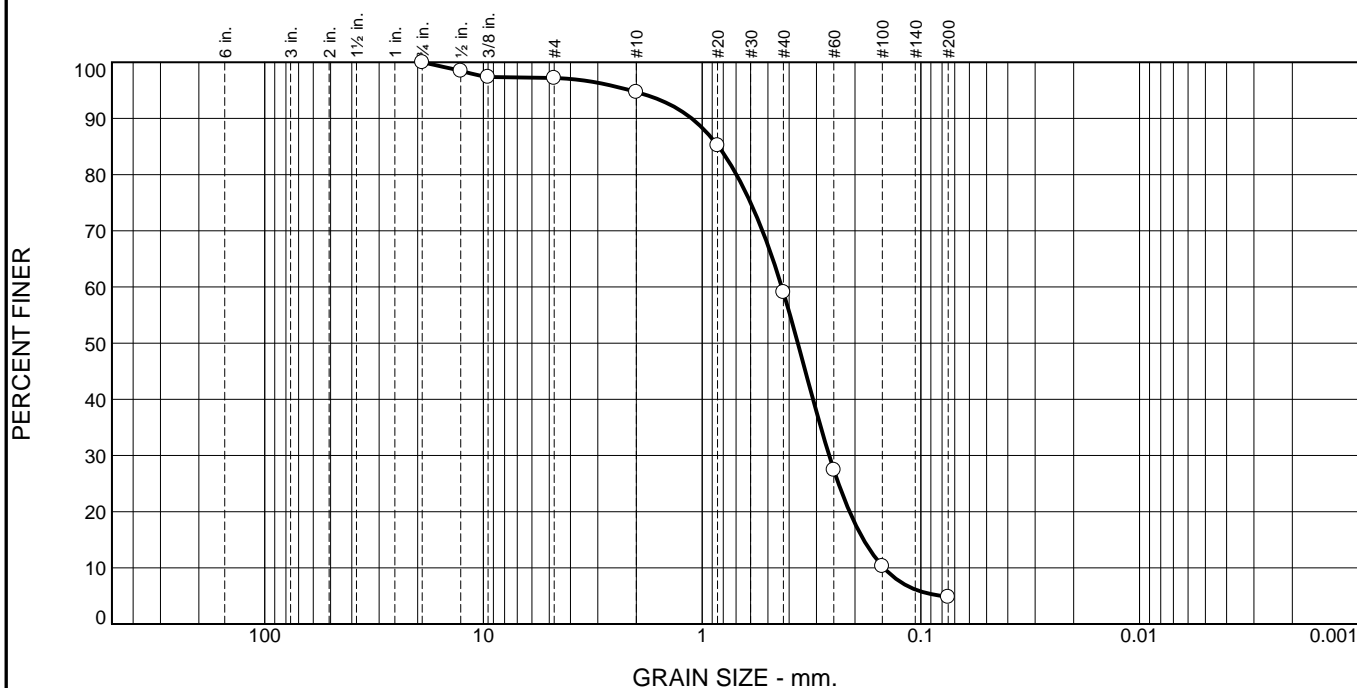
**CDM Smith**  
**Boston, Massachusetts**

**Client:** Town of Salem  
**Project:** South Broadway Watermain Replacement  
Salem, NH  
**Project No.:** 25972-215427

**Figure**

**Tested By:** GW **Checked By:** MP

# Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	2.8	2.5	35.6	54.3	4.8	

Test Results (ASTM D422 & ASTM D1140)			
Opening Size	Percent Finer	Spec.* (Percent)	Pass? (X=Fail)
.75"	100.0		
.5"	98.5		
.375"	97.4		
#4	97.2		
#10	94.7		
#20	85.2		
#40	59.1		
#60	27.4		
#100	10.3		
#200	4.8		

\* (no specification provided)

**Material Description**  
Light brown poorly graded sand

**Atterberg Limits (ASTM D 4318)**  
 PL= \_\_\_\_\_ LL= \_\_\_\_\_ PI= \_\_\_\_\_

**Classification**  
 USCS (D 2487)= SP AASHTO (M 145)= A-3

**Coefficients**  
 D<sub>90</sub>= 1.1165 D<sub>85</sub>= 0.8422 D<sub>60</sub>= 0.4322  
 D<sub>50</sub>= 0.3650 D<sub>30</sub>= 0.2625 D<sub>15</sub>= 0.1827  
 D<sub>10</sub>= 0.1475 C<sub>u</sub>= 2.93 C<sub>c</sub>= 1.08

**Remarks**  
As received MC = 18.9%

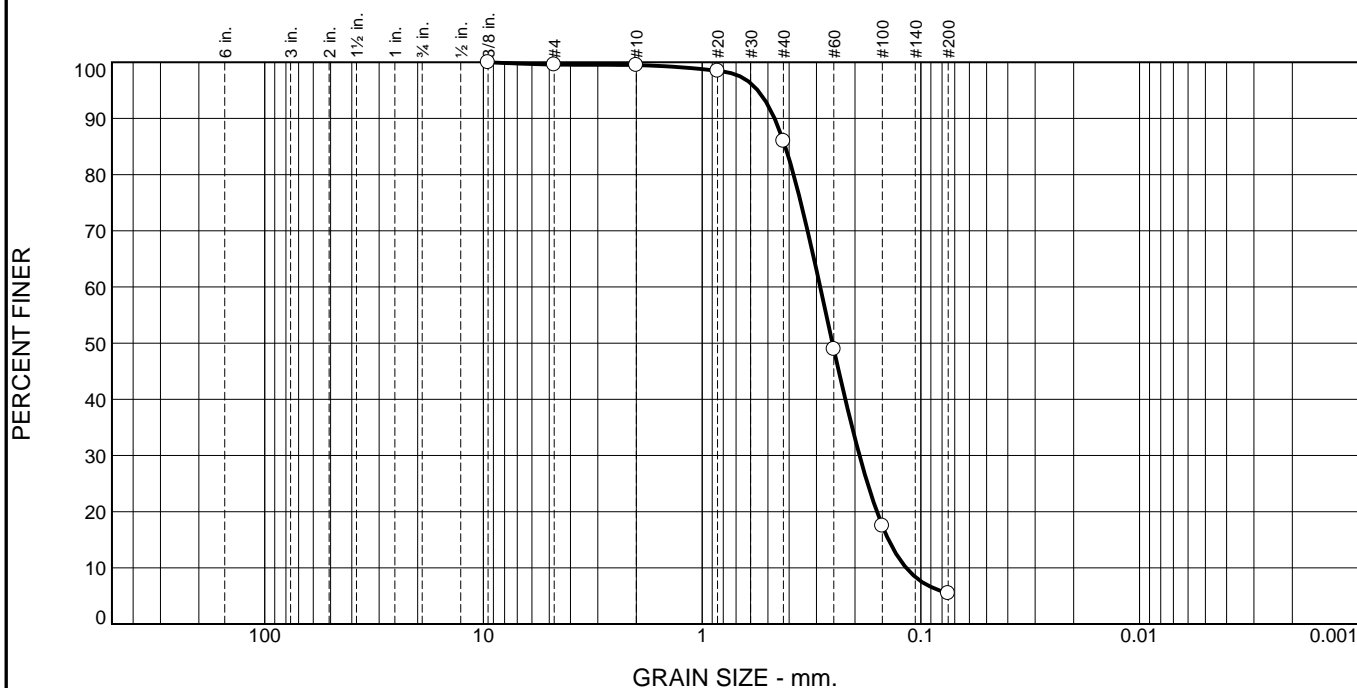
**Date Received:** 10/4/2017 **Date Tested:** 10/16/2017  
**Tested By:** GW  
**Checked By:** MP  
**Title:** Laboratory Manager

**Source of Sample:** B-6 **Depth:** 4-6'  
**Sample Number:** S-3

**Date Sampled:** 9/22/2017

<b>CDM Smith</b>  <b>Boston, Massachusetts</b>	<b>Client:</b> Town of Salem
	<b>Project:</b> South Broadway Watermain Replacement Salem, NH
<b>Project No:</b> 25972-215427	<b>Figure</b>

# Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	0.4	0.1	13.5	80.5	5.5	

Test Results (ASTM D422 & ASTM D1140)			
Opening Size	Percent Finer	Spec.* (Percent)	Pass? (X=Fail)
.375"	100.0		
#4	99.6		
#10	99.5		
#20	98.5		
#40	86.0		
#60	48.9		
#100	17.5		
#200	5.5		

\* (no specification provided)

## Material Description

Light brown poorly graded sand with silt

## Atterberg Limits (ASTM D 4318)

PL= LL= PI=

## Classification

USCS (D 2487)= SP-SM AASHTO (M 145)= A-3

## Coefficients

D<sub>90</sub>= 0.4663 D<sub>85</sub>= 0.4167 D<sub>60</sub>= 0.2886  
D<sub>50</sub>= 0.2535 D<sub>30</sub>= 0.1906 D<sub>15</sub>= 0.1405  
D<sub>10</sub>= 0.1167 C<sub>u</sub>= 2.47 C<sub>c</sub>= 1.08

## Remarks

As received MC = 23.0%

Date Received: 10/4/2017 Date Tested: 10/16/2017

Tested By: GW

Checked By: MP

Title: Laboratory Manager

Source of Sample: B-6 Depth: 8-10'  
Sample Number: S-5

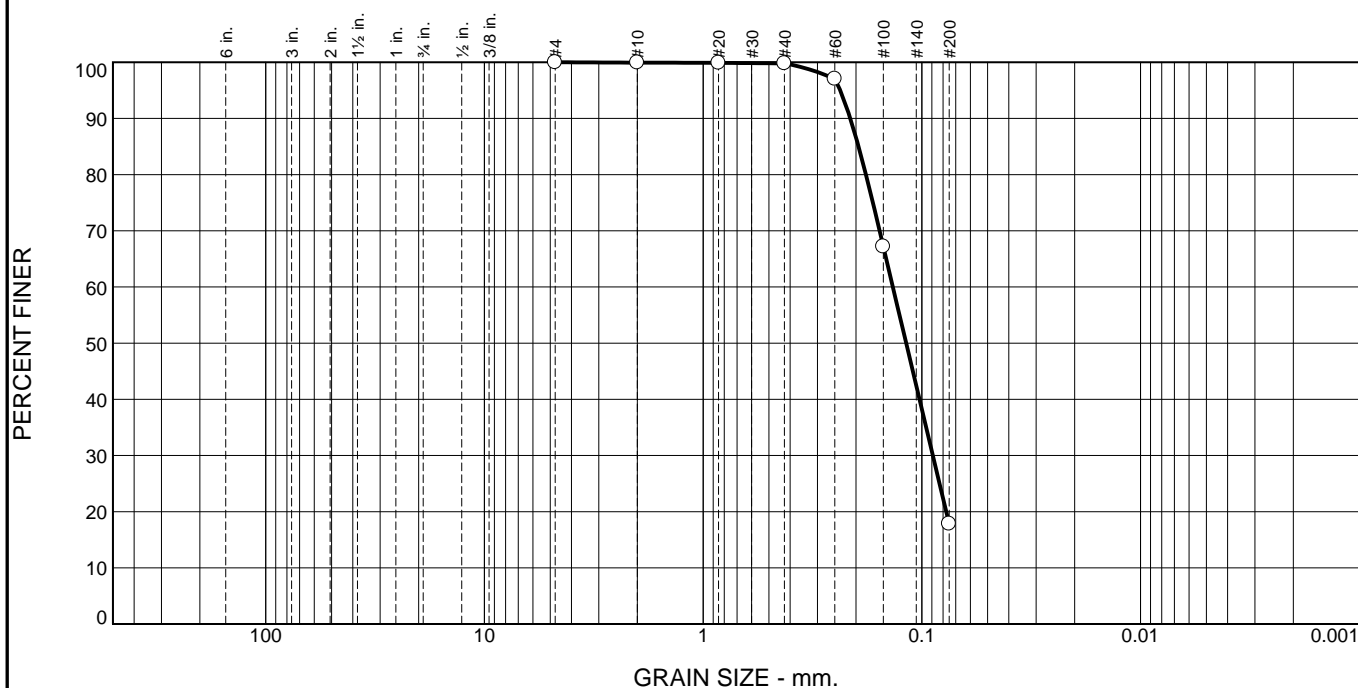
Date Sampled: 9/22/2017

**CDM Smith**  
**Boston, Massachusetts**

Client: Town of Salem  
Project: South Broadway Watermain Replacement  
Salem, NH  
Project No: 25972-215427

Figure

# Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	0.0	0.1	0.1	82.0	17.8	

Test Results (ASTM D422 & ASTM D1140)			
Opening Size	Percent Finer	Spec.* (Percent)	Pass? (X=Fail)
#4	100.0		
#10	99.9		
#20	99.9		
#40	99.8		
#60	97.0		
#100	67.2		
#200	17.8		

\* (no specification provided)

**Material Description**  
Light brown silty sand

**Atterberg Limits (ASTM D 4318)**  
PL=                      LL=                      PI=

**Classification**  
USCS (D 2487)= SM                      AASHTO (M 145)= A-2-4(0)

**Coefficients**  
D<sub>90</sub>= 0.2123                      D<sub>85</sub>= 0.1946                      D<sub>60</sub>= 0.1356  
D<sub>50</sub>= 0.1179                      D<sub>30</sub>= 0.0890                      D<sub>15</sub>=  
D<sub>10</sub>=                      C<sub>u</sub>=                      C<sub>c</sub>=

**Remarks**  
As received MC = 28.4%

**Date Received:** 10/4/2017                      **Date Tested:** 10/16/2017  
**Tested By:** GW  
**Checked By:** MP  
**Title:** Laboratory Manager

**Source of Sample:** B-6                      **Depth:** 14-16'  
**Sample Number:** S-6

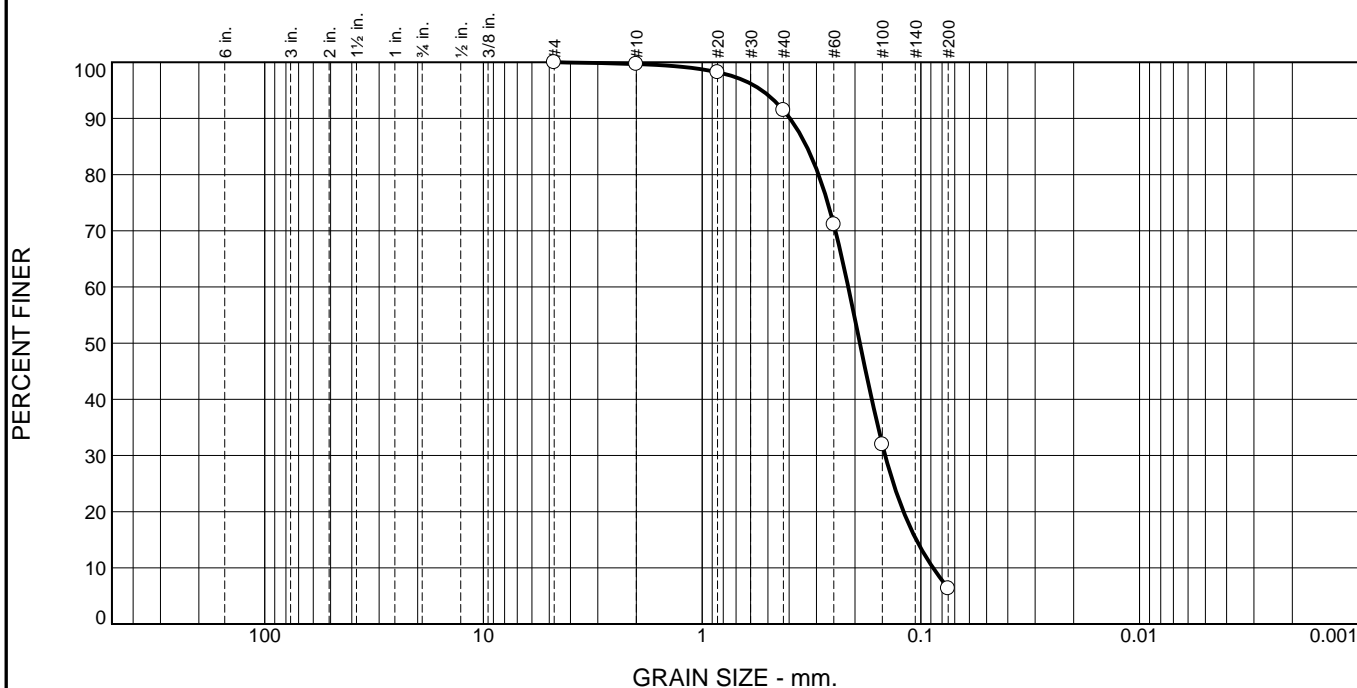
**Date Sampled:** 9/22/2017

**CDM Smith**  
**Boston, Massachusetts**

**Client:** Town of Salem  
**Project:** South Broadway Watermain Replacement  
Salem, NH  
**Project No:** 25972-215427

**Figure**

# Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	0.0	0.3	8.2	85.2	6.3	

Test Results (ASTM D422 & ASTM D1140)			
Opening Size	Percent Finer	Spec.* (Percent)	Pass? (X=Fail)
#4	100.0		
#10	99.7		
#20	98.2		
#40	91.5		
#60	71.1		
#100	32.0		
#200	6.3		

\* (no specification provided)

## Material Description

Light brown poorly graded sand with silt

## Atterberg Limits (ASTM D 4318)

PL= LL= PI=

## Classification

USCS (D 2487)= SP-SM AASHTO (M 145)= A-3

## Coefficients

D<sub>90</sub>= 0.3966 D<sub>85</sub>= 0.3324 D<sub>60</sub>= 0.2147  
D<sub>50</sub>= 0.1899 D<sub>30</sub>= 0.1455 D<sub>15</sub>= 0.1050  
D<sub>10</sub>= 0.0879 C<sub>u</sub>= 2.44 C<sub>c</sub>= 1.12

## Remarks

As received MC = 24.3%

Date Received: 10/4/2017 Date Tested: 10/16/2017

Tested By: GW

Checked By: MP

Title: Laboratory Manager

Source of Sample: B-7 Depth: 8-10'  
Sample Number: S-5

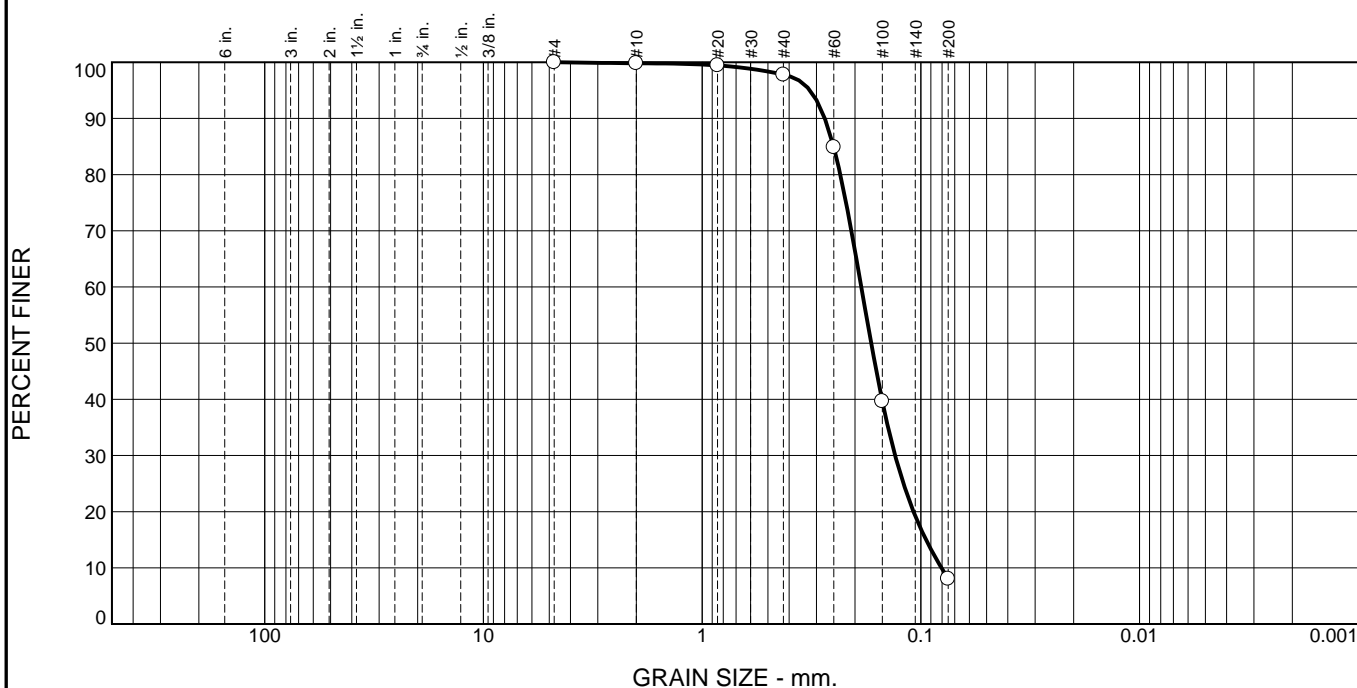
Date Sampled: 9/21/2017

**CDM Smith**  
**Boston, Massachusetts**

Client: Town of Salem  
Project: South Broadway Watermain Replacement  
Salem, NH  
Project No: 25972-215427

Figure

# Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	0.0	0.2	2.0	89.8	8.0	

Test Results (ASTM D422 & ASTM D1140)			
Opening Size	Percent Finer	Spec.* (Percent)	Pass? (X=Fail)
#4	100.0		
#10	99.8		
#20	99.4		
#40	97.8		
#60	84.9		
#100	39.6		
#200	8.0		

\* (no specification provided)

## Material Description

Light brown poorly graded sand with silt

## Atterberg Limits (ASTM D 4318)

PL= LL= PI=

## Classification

USCS (D 2487)= SP-SM AASHTO (M 145)= A-3

## Coefficients

D<sub>90</sub>= 0.2749 D<sub>85</sub>= 0.2505 D<sub>60</sub>= 0.1871  
D<sub>50</sub>= 0.1686 D<sub>30</sub>= 0.1310 D<sub>15</sub>= 0.0947  
D<sub>10</sub>= 0.0804 C<sub>u</sub>= 2.33 C<sub>c</sub>= 1.14

## Remarks

As received MC = 25.8%

Date Received: 10/4/2017 Date Tested: 10/16/2017

Tested By: GW

Checked By: MP

Title: Laboratory Manager

Source of Sample: B-7 Depth: 14-16'  
Sample Number: S-6

Date Sampled: 9/21/2017

**CDM Smith**  
**Boston, Massachusetts**

Client: Town of Salem  
Project: South Broadway Watermain Replacement  
Salem, NH  
Project No: 25972-215427

Figure



PERCENT FINER

GRAIN SIZE - mm.

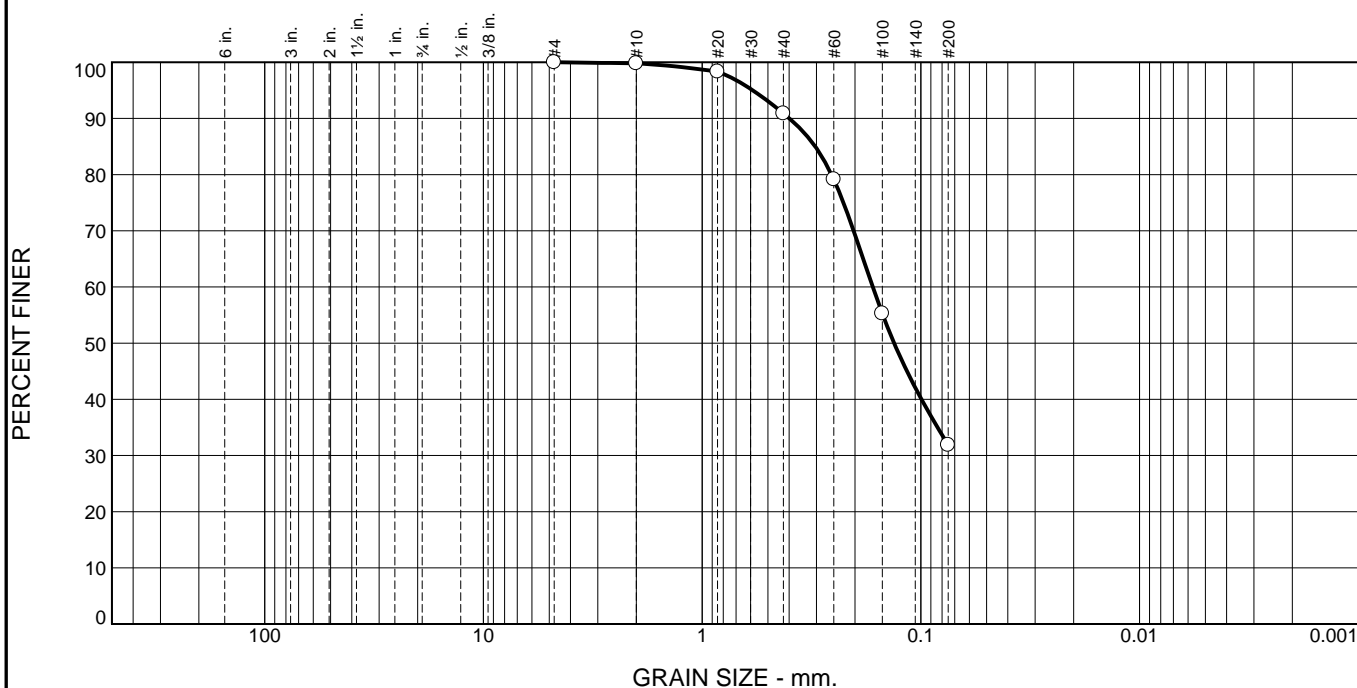
Grain Size (mm)	Percent Finer (%)
6 in.	100
3 in.	100
2 in.	100
1 1/2 in.	100
1 in.	100
3/4 in.	100
1/2 in.	100
3/8 in.	100
#4 (4.75 mm)	100
#10 (2.0 mm)	100
#20 (0.85 mm)	98
#40 (0.425 mm)	75
#60 (0.25 mm)	38
#100 (0.15 mm)	15
#200 (0.075 mm)	5

Test Results (ASTM D422 & ASTM D1140)			
Opening Size	Percent Finer	Spec.* (Percent)	Pass? (X=Fail)
#4	100.0		
#10	99.7		
#20	97.2		
#40	74.4		
#60	37.9		
#100	15.2		
#200	6.5		

**Title:** Laboratory Manager

### Figure

# Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	0.0	0.2	8.9	59.0	31.9	

Test Results (ASTM D422 & ASTM D1140)			
Opening Size	Percent Finer	Spec.* (Percent)	Pass? (X=Fail)
#4	100.0		
#10	99.8		
#20	98.3		
#40	90.9		
#60	79.1		
#100	55.3		
#200	31.9		

\* (no specification provided)

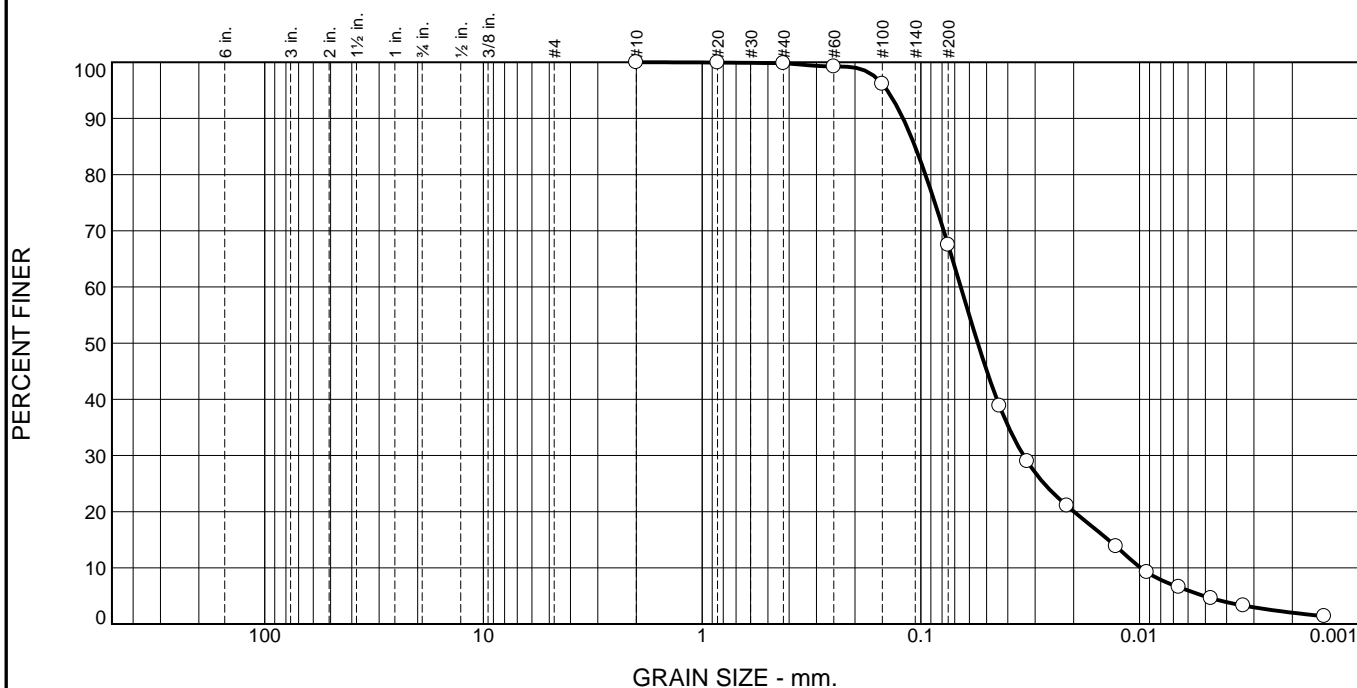
<b>Material Description</b>	
Light brown silty sand	
<b>Atterberg Limits (ASTM D 4318)</b>	
PL=	LL= PI=
<b>Classification</b>	
USCS (D 2487)= SM	AASHTO (M 145)= A-2-4(0)
<b>Coefficients</b>	
D <sub>90</sub> = 0.3993	D <sub>85</sub> = 0.3039 D <sub>60</sub> = 0.1658
D <sub>50</sub> = 0.1325	D <sub>30</sub> =
D <sub>10</sub> =	C <sub>u</sub> = C <sub>c</sub> =
<b>Remarks</b>	
As received MC = 23.8%	
Date Received: 10/4/2017	Date Tested: 10/16/2017
Tested By: GW	
Checked By: MP	
Title: Laboratory Manager	

Source of Sample: B-8 Depth: 8-10'  
Sample Number: S-5

Date Sampled: 9/21/2017

<b>CDM Smith</b>  <b>Boston, Massachusetts</b>	<b>Client:</b> Town of Salem <b>Project:</b> South Broadway Watermain Replacement Salem, NH
	<b>Project No:</b> 25972-215427 <b>Figure</b>

# Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	0.0	0.0	0.2	32.3	62.6	4.9

Test Results (ASTM D 422 & ASTM D 1140)			
Opening Size	Percent Finer	Spec.* (Percent)	Pass? (X=Fail)
#10	100.0		
#20	99.9		
#40	99.8		
#60	99.2		
#100	96.1		
#200	67.5		
0.0437 mm.	38.8		
0.0326 mm.	29.0		
0.0215 mm.	21.1		
0.0128 mm.	13.8		
0.0092 mm.	9.2		
0.0066 mm.	6.6		
0.0047 mm.	4.6		
0.0034 mm.	3.3		
0.0014 mm.	1.4		

\* (no specification provided)

**Material Description**  
Light brown sandy silt

**Atterberg Limits (ASTM D 4318)**  
PL=                      LL=                      PI=

**Classification**  
USCS (D 2487)= ML                      AASHTO (M 145)= A-4(0)

**Coefficients**  
D<sub>90</sub>= 0.1206                      D<sub>85</sub>= 0.1063                      D<sub>60</sub>= 0.0657  
D<sub>50</sub>= 0.0549                      D<sub>30</sub>= 0.0339                      D<sub>15</sub>= 0.0139  
D<sub>10</sub>= 0.0099                      C<sub>u</sub>= 6.66                      C<sub>c</sub>= 1.77

**Remarks**  
As received MC = 26.3%

**Date Received:** 10/4/2017                      **Date Tested:** 10/17/2017  
**Tested By:** GW  
**Checked By:** MP  
**Title:** Laboratory Manager

**Source of Sample:** B-8                      **Depth:** 24-26'  
**Sample Number:** S-8

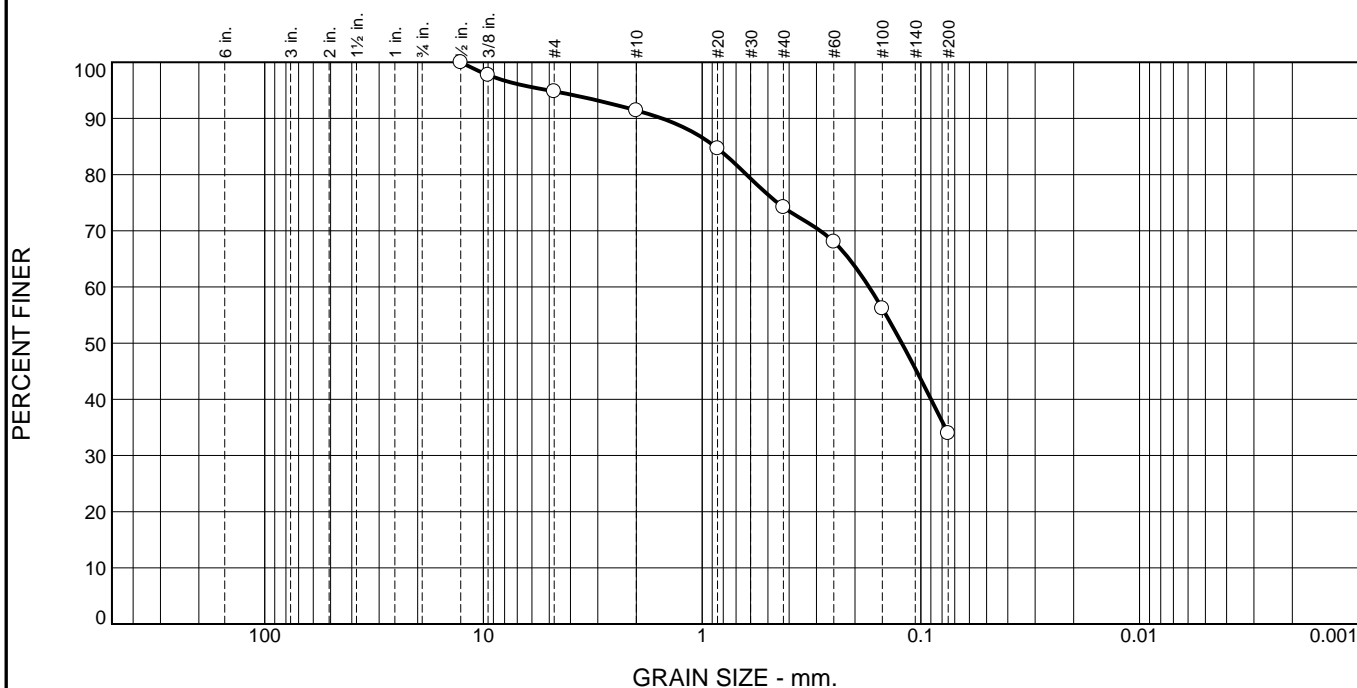
**Date Sampled:** 9/22/2017

**CDM Smith**  
**Boston, Massachusetts**

**Client:** Town of Salem  
**Project:** South Broadway Watermain Replacement  
Salem, NH  
**Project No:** 25972-215427

**Figure**

# Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	5.2	3.4	17.2	40.3	33.9	

Test Results (ASTM D422 & ASTM D1140)			
Opening Size	Percent Finer	Spec.* (Percent)	Pass? (X=Fail)
.5"	100.0		
.375"	97.7		
#4	94.8		
#10	91.4		
#20	84.7		
#40	74.2		
#60	68.0		
#100	56.2		
#200	33.9		

\* (no specification provided)

**Material Description**  
Dark brown silty sand

**Atterberg Limits (ASTM D 4318)**  
 PL= \_\_\_\_\_ LL= \_\_\_\_\_ PI= \_\_\_\_\_

**Classification**  
 USCS (D 2487)= SM AASHTO (M 145)= A-2-4(0)

**Coefficients**  
 D<sub>90</sub>= 1.5251 D<sub>85</sub>= 0.8728 D<sub>60</sub>= 0.1724  
 D<sub>50</sub>= 0.1222 D<sub>30</sub>= \_\_\_\_\_ D<sub>15</sub>= \_\_\_\_\_  
 D<sub>10</sub>= \_\_\_\_\_ C<sub>u</sub>= \_\_\_\_\_ C<sub>c</sub>= \_\_\_\_\_

**Remarks**  
As received MC = 18.4%

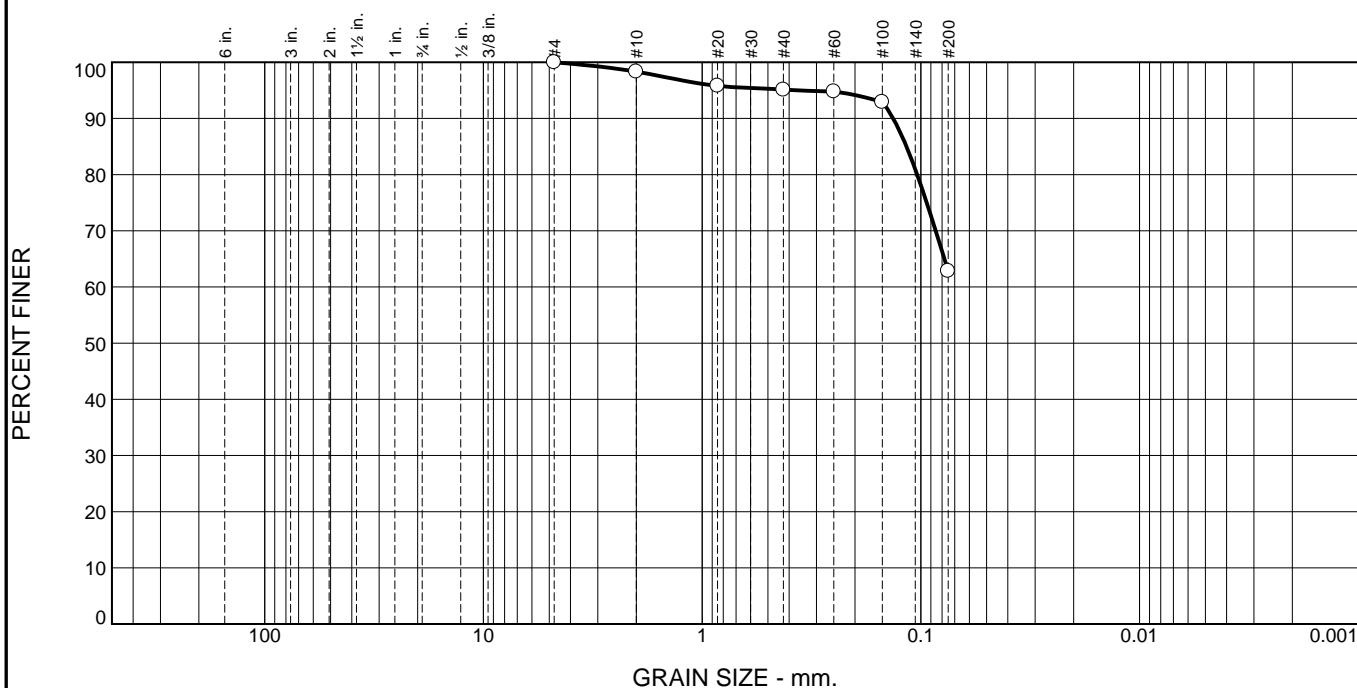
**Date Received:** 10/4/2017 **Date Tested:** 10/16/2017  
**Tested By:** GW  
**Checked By:** MP  
**Title:** Laboratory Manager

**Source of Sample:** B-9 **Depth:** 1-2.5'  
**Sample Number:** S-1A(TOP)

**Date Sampled:** 9/27/2017

<b>CDM Smith</b>  <b>Boston, Massachusetts</b>	<b>Client:</b> Town of Salem <b>Project:</b> South Broadway Watermain Replacement Salem, NH
	<b>Project No:</b> 25972-215427 <b>Figure</b>

# Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	0.0	1.7	3.2	32.3	62.8	

Test Results (ASTM D422 & ASTM D1140)			
Opening Size	Percent Finer	Spec.* (Percent)	Pass? (X=Fail)
#4	100.0		
#10	98.3		
#20	95.8		
#40	95.1		
#60	94.8		
#100	92.9		
#200	62.8		

\* (no specification provided)

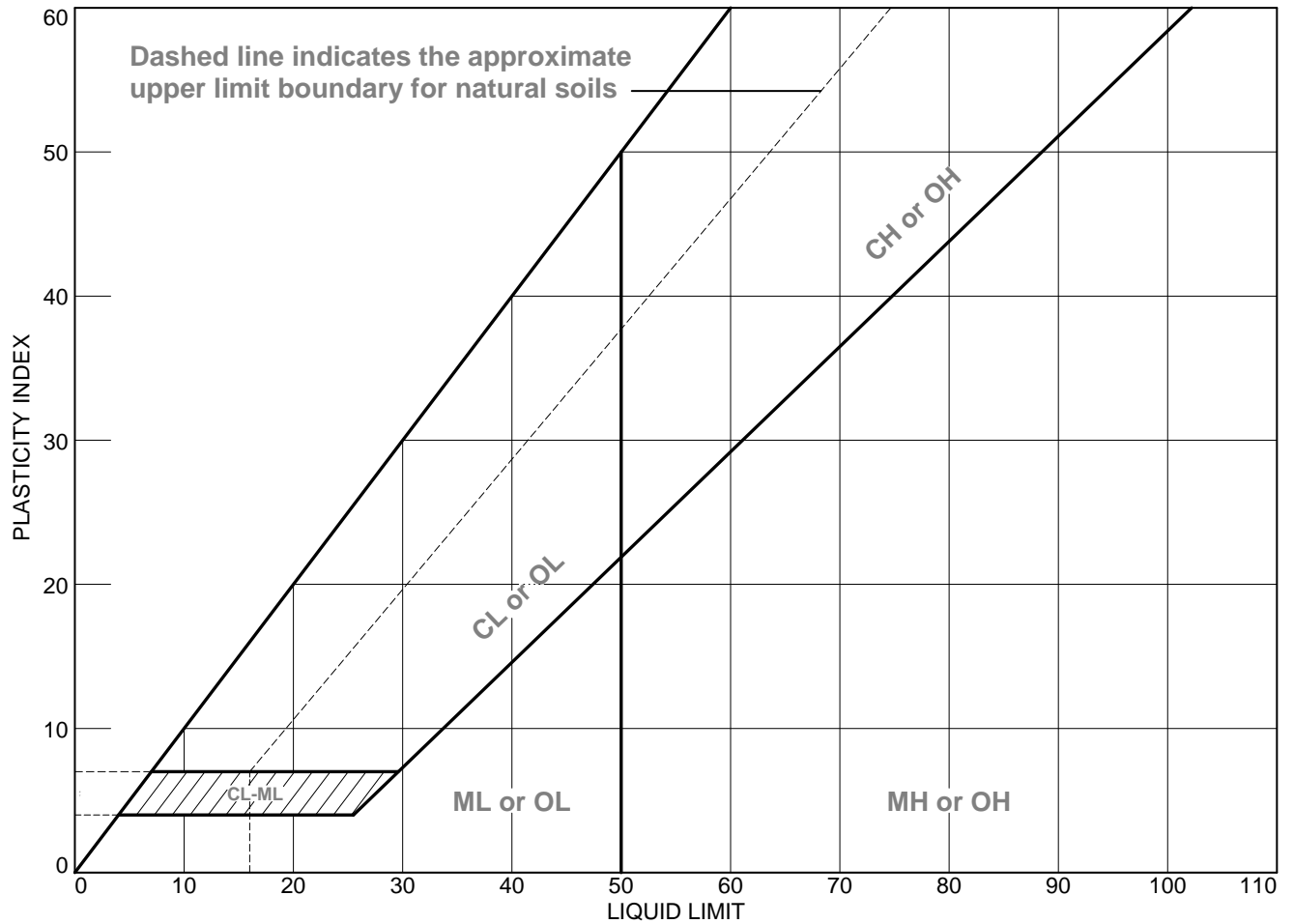
<b>Material Description</b>		
Light brown sandy silt		
<b>Atterberg Limits (ASTM D 4318)</b>		
PL=	LL=	PI=
<b>Classification</b>		
USCS (D 2487)=	ML	AASHTO (M 145)= A-4(0)
<b>Coefficients</b>		
D <sub>90</sub> = 0.1337	D <sub>85</sub> = 0.1163	D <sub>60</sub> =
D <sub>50</sub> =	D <sub>30</sub> =	D <sub>15</sub> =
D <sub>10</sub> =	C <sub>u</sub> =	C <sub>c</sub> =
<b>Remarks</b>		
As received MC = 31.0%		
<b>Date Received:</b> 10/4/2017		<b>Date Tested:</b> 10/17/2017
<b>Tested By:</b> RZ		
<b>Checked By:</b> MP		
<b>Title:</b> Laboratory Manager		

Source of Sample: B-9 Depth: 8-10'  
Sample Number: S-5

Date Sampled: 9/27/2017

<b>CDM Smith</b>  <b>Boston, Massachusetts</b>	<b>Client:</b> Town of Salem <b>Project:</b> South Broadway Watermain Replacement Salem, NH
	<b>Project No:</b> 25972-215427 <b>Figure</b>

# LIQUID AND PLASTIC LIMITS TEST REPORT



SOIL DATA								
SYMBOL	SOURCE	SAMPLE NO.	DEPTH	NATURAL WATER CONTENT (%)	PLASTIC LIMIT (%)	LIQUID LIMIT (%)	PLASTICITY INDEX (%)	USCS
●	B-9	S-6	14-16'	32.2	NP	NV	NP	ML

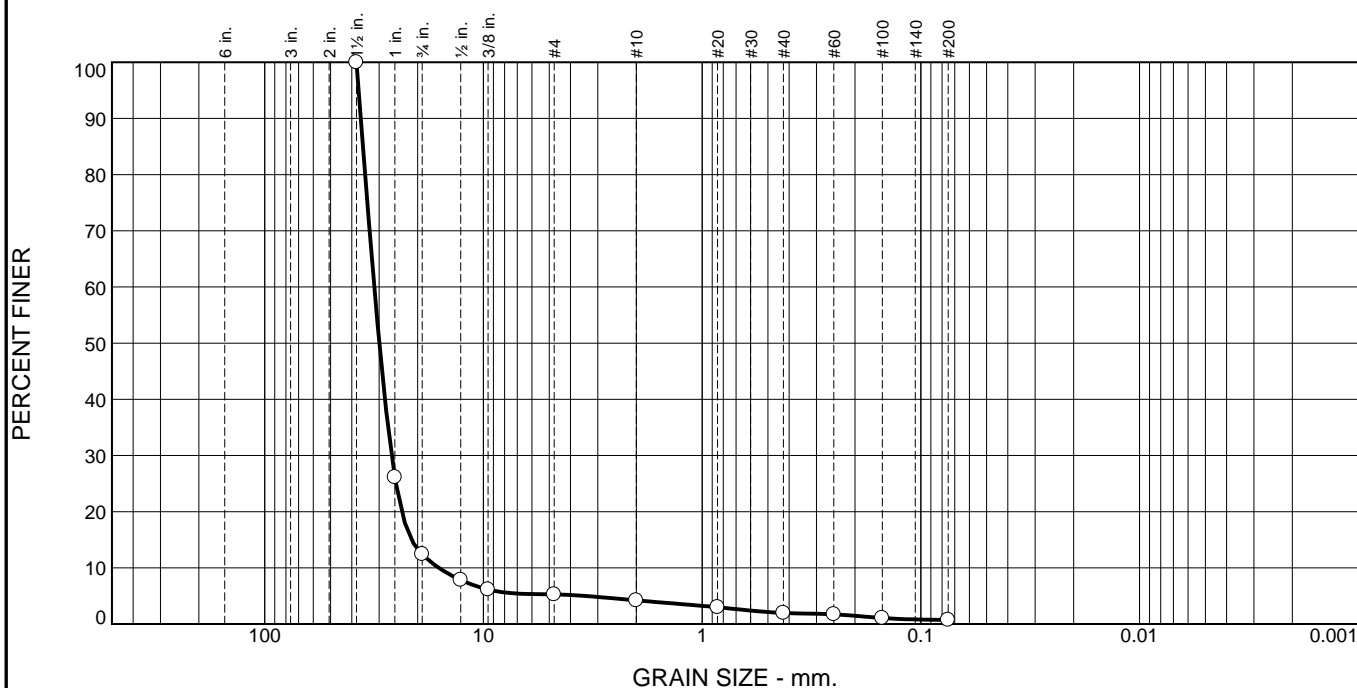
**CDM Smith**  
**Boston, Massachusetts**

**Client:** Town of Salem  
**Project:** South Broadway Watermain Replacement  
Salem, NH  
**Project No.:** 25972-215427

**Figure**

**Tested By:** RZ **Checked By:** MP

# Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	87.6	7.1	1.1	2.2	1.3	0.7	

Test Results (ASTM D422 & ASTM D1140)			
Opening Size	Percent Finer	Spec.* (Percent)	Pass? (X=Fail)
1.5"	100.0		
1"	26.1		
.75"	12.4		
.5"	7.8		
.375"	6.1		
#4	5.3		
#10	4.2		
#20	3.0		
#40	2.0		
#60	1.7		
#100	1.1		
#200	0.7		

\* (no specification provided)

**Material Description**  
Gray poorly graded gravel

**Atterberg Limits (ASTM D 4318)**  
PL=                      LL=                      PI=

**Classification**  
USCS (D 2487)= GP                      AASHTO (M 145)= A-1-a

**Coefficients**  
D<sub>90</sub>= 36.4115                      D<sub>85</sub>= 35.5888                      D<sub>60</sub>= 31.5692  
D<sub>50</sub>= 29.9359                      D<sub>30</sub>= 26.2793                      D<sub>15</sub>= 21.3363  
D<sub>10</sub>= 16.0093                      C<sub>u</sub>= 1.97                      C<sub>c</sub>= 1.37

**Remarks**  
As received MC = 2.8%

**Date Received:** 10/4/2017                      **Date Tested:** 10/17/2017  
**Tested By:** RZ  
**Checked By:** MP  
**Title:** Laboratory Manager

**Source of Sample:** B-10                      **Depth:** 0-1'  
**Sample Number:** BAG (Subbase)

**Date Sampled:** 10/4/2017

**CDM Smith**  
**Boston, Massachusetts**

**Client:** Town of Salem  
**Project:** South Broadway Watermain Replacement  
Salem, NH  
**Project No:** 25972-215427

**Figure**

# CDM Smith

## Geotechnical Engineering Laboratory

### Standard Test Method for Moisture, Ash, and Organic Matter of Peat and Other Organic Soils (ASTM D2974)

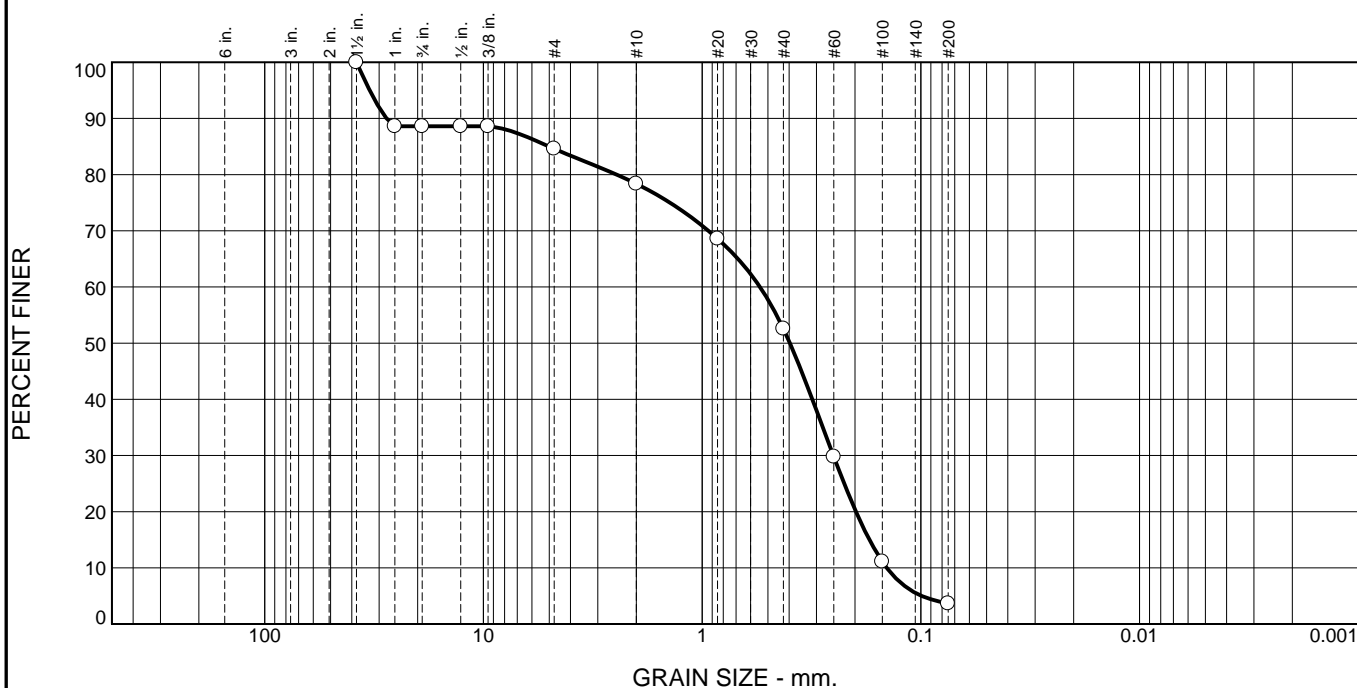
Client:	Town of Salem	Tested By:	MP
Project Name:	South Broadway WM Replacement	Test Date:	10/16/2017
Project Location:	Salem, NH		
Project Number:	25972-215427		
Boring Number:	B-10	Procedure:	C
Sample Number:	S-3	Temperature:	440° C
Sample Depth (ft):	4-6		
Sample Date:	10/4/2017		

AS RECEIVED MOISTURE CONTENT	
Tin Mass (g)	90.23
Wet Mass of Sample & Tin (g)	143.79
Dry Mass of Sample & Tin (g)	132.12
Mass of Water (g)	11.67
Mass of Dry Soil (g)	41.89
<b>Moisture Content (%)</b>	<b>27.9</b>

ASH CONTENT	
Porcelain Dish Mass (g)	90.23
Porcelain Dish + Oven Dried Soil (g)	132.12
Mass of Oven Dried Soil (g)	41.89
Mass of Dish & Burned Soil (g)	130.34
Mass of Burned Soil (g)	40.11
Mass of Organic Material (g)	1.78
Ash Content (%)	95.8
<b>Organic Content (%)</b>	<b>4.2</b>



# Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	11.4	4.0	6.2	25.8	48.9	3.7	

Test Results (ASTM D422 & ASTM D1140)			
Opening Size	Percent Finer	Spec.* (Percent)	Pass? (X=Fail)
1.5"	100.0		
1"	88.6		
.75"	88.6		
.5"	88.6		
.375"	88.6		
#4	84.6		
#10	78.4		
#20	68.6		
#40	52.6		
#60	29.8		
#100	11.1		
#200	3.7		

\* (no specification provided)

**Material Description**  
Brown poorly graded sand with gravel

**Atterberg Limits (ASTM D 4318)**  
 PL=                      LL=                      PI=

**Classification**  
 USCS (D 2487)= SP                      AASHTO (M 145)= A-3

**Coefficients**  
 D<sub>90</sub>= 27.7200                      D<sub>85</sub>= 5.0101                      D<sub>60</sub>= 0.5440  
 D<sub>50</sub>= 0.3968                      D<sub>30</sub>= 0.2513                      D<sub>15</sub>= 0.1718  
 D<sub>10</sub>= 0.1431                      C<sub>u</sub>= 3.80                      C<sub>c</sub>= 0.81

**Remarks**  
As received MC = 14.3%

**Date Received:** 10/4/2017                      **Date Tested:** 10/17/2017  
**Tested By:** RZ  
**Checked By:** MP  
**Title:** Laboratory Manager

**Source of Sample:** B-11                      **Depth:** 8-10'  
**Sample Number:** S-5

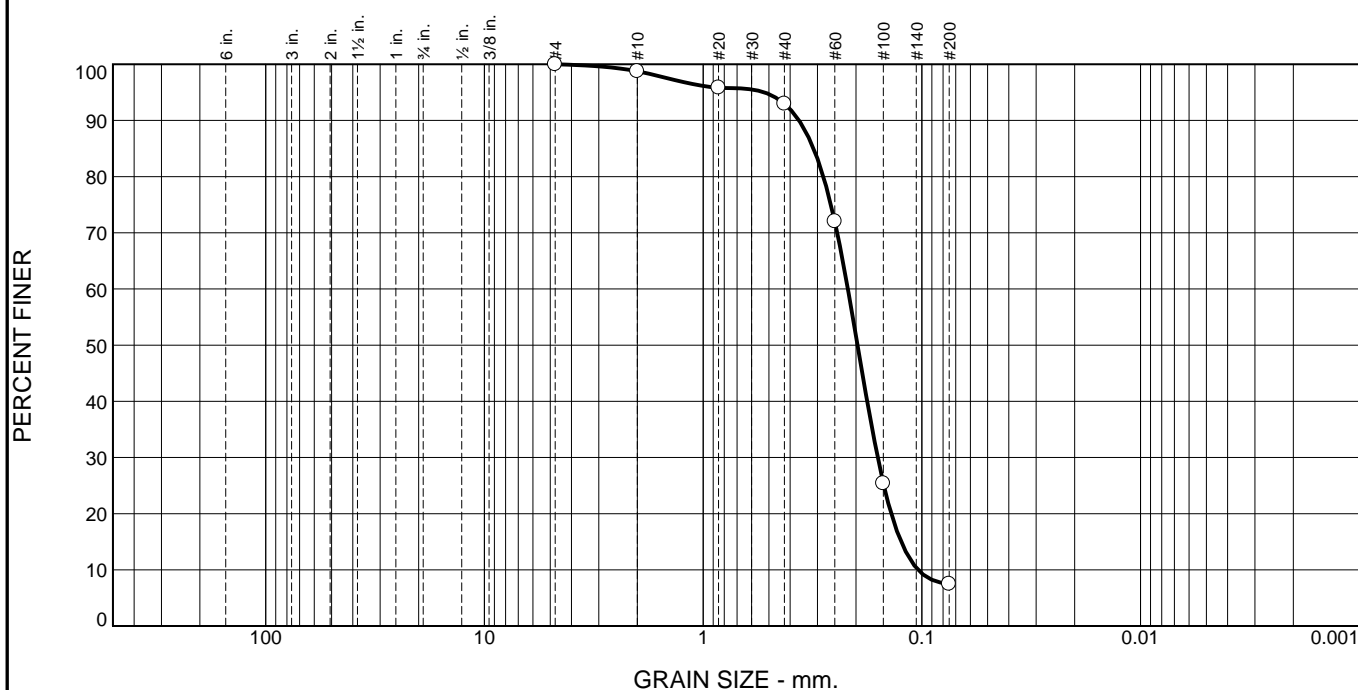
**Date Sampled:** 4/12-13/2017

**CDM Smith**  
  
**Boston, Massachusetts**

**Client:** Town of Salem  
**Project:** South Broadway Watermain Replacement  
 Salem, NH  
**Project No:** 25972-215427

**Figure**

# Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	0.0	1.3	5.8	85.4	7.5	

Test Results (ASTM D422 & ASTM D1140)			
Opening Size	Percent Finer	Spec.* (Percent)	Pass? (X=Fail)
#4	100.0		
#10	98.7		
#20	95.8		
#40	92.9		
#60	72.0		
#100	25.4		
#200	7.5		

\* (no specification provided)

## Material Description

Light brown poorly graded sand with silt

## Atterberg Limits (ASTM D 4318)

PL= LL= PI=

## Classification

USCS (D 2487)= SP-SM AASHTO (M 145)= A-3

## Coefficients

D<sub>90</sub>= 0.3641 D<sub>85</sub>= 0.3126 D<sub>60</sub>= 0.2178  
D<sub>50</sub>= 0.1967 D<sub>30</sub>= 0.1591 D<sub>15</sub>= 0.1243  
D<sub>10</sub>= 0.1041 C<sub>u</sub>= 2.09 C<sub>c</sub>= 1.12

## Remarks

As received MC = 25.0%

Date Received: 10/4/2017 Date Tested: 10/17/2017

Tested By: RZ

Checked By: MP

Title: Laboratory Manager

Source of Sample: B-12 Depth: 8-10'  
Sample Number: S-5

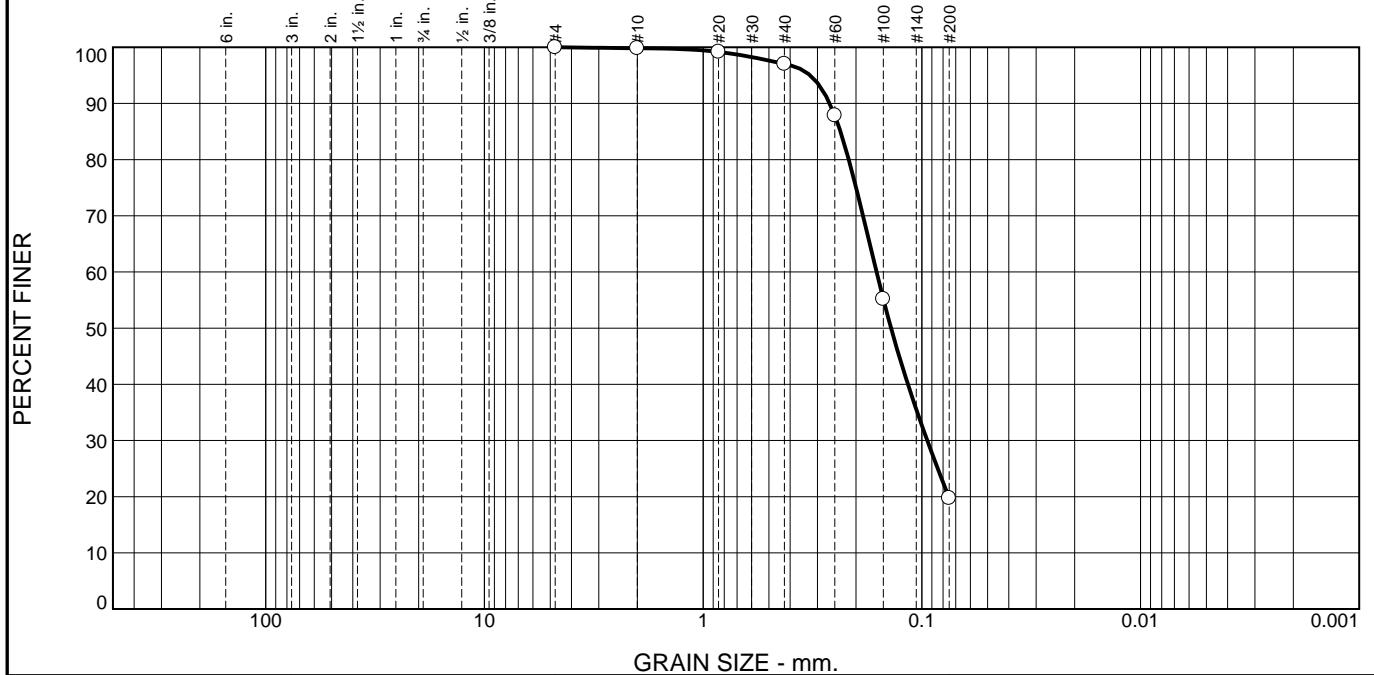
Date Sampled: 9/26/2017

**CDM Smith**  
**Boston, Massachusetts**

Client: Town of Salem  
Project: South Broadway Watermain Replacement  
Salem, NH  
Project No: 25972-215427

Figure

# Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	0.0	0.2	2.8	77.3	19.7	

Test Results (ASTM D422 & ASTM D1140)			
Opening Size	Percent Finer	Spec.* (Percent)	Pass? (X=Fail)
#4	100.0		
#10	99.8		
#20	99.2		
#40	97.0		
#60	87.9		
#100	55.2		
#200	19.7		

\* (no specification provided)

<b>Material Description</b>	
Reddish-brown silty sand	
<b>Atterberg Limits (ASTM D 4318)</b>	
PL=	LL= PI=
<b>Classification</b>	
USCS (D 2487)= SM	AASHTO (M 145)= A-2-4(0)
<b>Coefficients</b>	
D <sub>90</sub> = 0.2637	D <sub>85</sub> = 0.2356 D <sub>60</sub> = 0.1611
D <sub>50</sub> = 0.1382	D <sub>30</sub> = 0.0944 D <sub>15</sub> =
D <sub>10</sub> =	C <sub>u</sub> = C <sub>c</sub> =
<b>Remarks</b>	
As received MC = 24.4%	
<b>Date Received:</b> 10/4/2017 <b>Date Tested:</b> 10/17/2017	
<b>Tested By:</b> RZ	
<b>Checked By:</b> MP	
<b>Title:</b> Laboratory Manager	

Source of Sample: B-12 Depth: 14-16'  
Sample Number: S-6

Date Sampled: 9/26/2017

<b>CDM Smith</b>  <b>Boston, Massachusetts</b>	<b>Client:</b> Town of Salem <b>Project:</b> South Broadway Watermain Replacement Salem, NH
	<b>Project No:</b> 25972-215427 <b>Figure</b>

# CDM Smith

## Geotechnical Engineering Laboratory

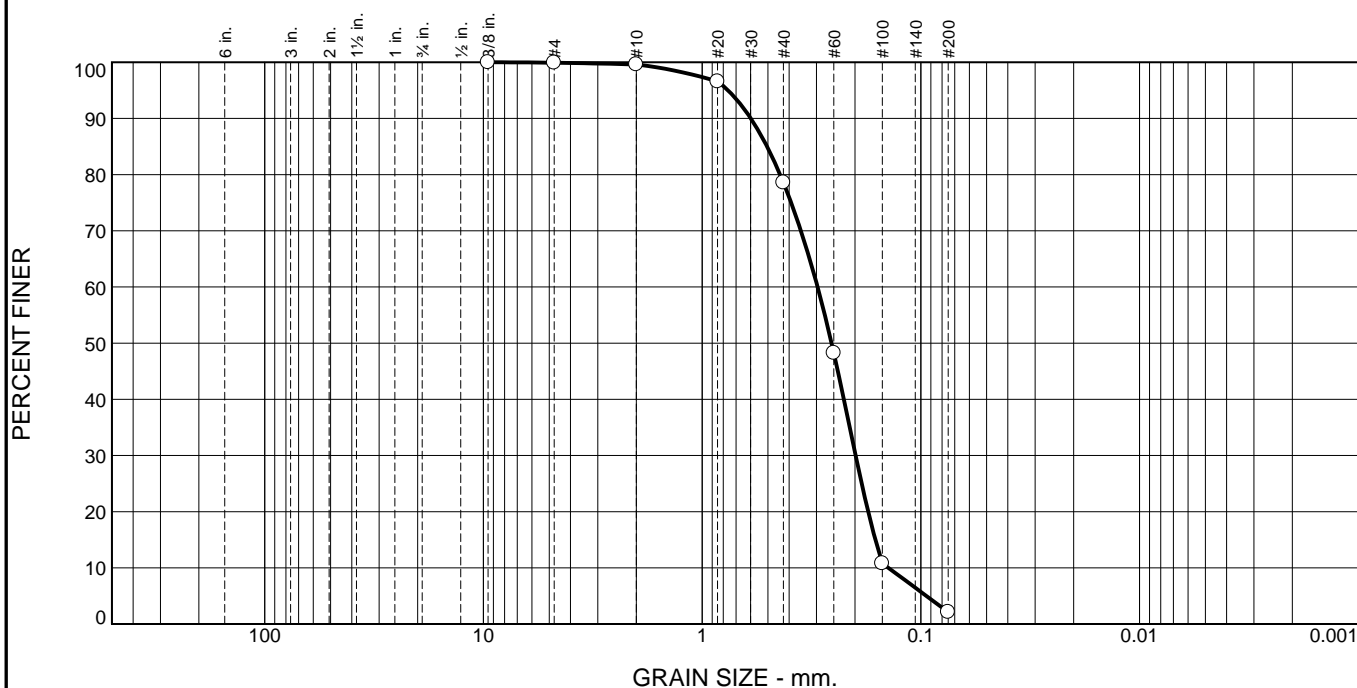
### Standard Test Method for Moisture, Ash, and Organic Matter of Peat and Other Organic Soils (ASTM D2974)

Client:	Town of Salem	Tested By:	MP
Project Name:	South Broadway WM Replacement	Test Date:	10/16/2017
Project Location:	Salem, NH		
Project Number:	25972-215427		
Boring Number:	B-13	Procedure:	C
Sample Number:	S-4	Temperature:	440° C
Sample Depth (ft):	6-8		
Sample Date:	9/19-20/2017		

AS RECEIVED MOISTURE CONTENT	
Tin Mass (g)	96.80
Wet Mass of Sample & Tin (g)	177.54
Dry Mass of Sample & Tin (g)	162.12
Mass of Water (g)	15.42
Mass of Dry Soil (g)	65.32
<b>Moisture Content (%)</b>	<b>23.6</b>

ASH CONTENT	
Porcelain Dish Mass (g)	96.80
Porcelain Dish + Oven Dried Soil (g)	162.12
Mass of Oven Dried Soil (g)	65.32
Mass of Dish & Burned Soil (g)	161.63
Mass of Burned Soil (g)	64.83
Mass of Organic Material (g)	0.49
Ash Content (%)	99.2
<b>Organic Content (%)</b>	<b>0.8</b>

# Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	0.1	0.3	21.0	76.5	2.1	

Test Results (ASTM D422 & ASTM D1140)			
Opening Size	Percent Finer	Spec.* (Percent)	Pass? (X=Fail)
.375"	100.0		
#4	99.9		
#10	99.6		
#20	96.6		
#40	78.6		
#60	48.3		
#100	10.8		
#200	2.1		

\* (no specification provided)

**Material Description**  
 Light brown poorly graded sand

**Atterberg Limits (ASTM D 4318)**  
 PL=                      LL=                      PI=

**Classification**  
 USCS (D 2487)= SP                      AASHTO (M 145)= A-3

**Coefficients**  
 D<sub>90</sub>= 0.5987                      D<sub>85</sub>= 0.5048                      D<sub>60</sub>= 0.2966  
 D<sub>50</sub>= 0.2560                      D<sub>30</sub>= 0.1986                      D<sub>15</sub>= 0.1615  
 D<sub>10</sub>= 0.1407                      C<sub>u</sub>= 2.11                      C<sub>c</sub>= 0.94

**Remarks**  
 As received MC = 23.1%

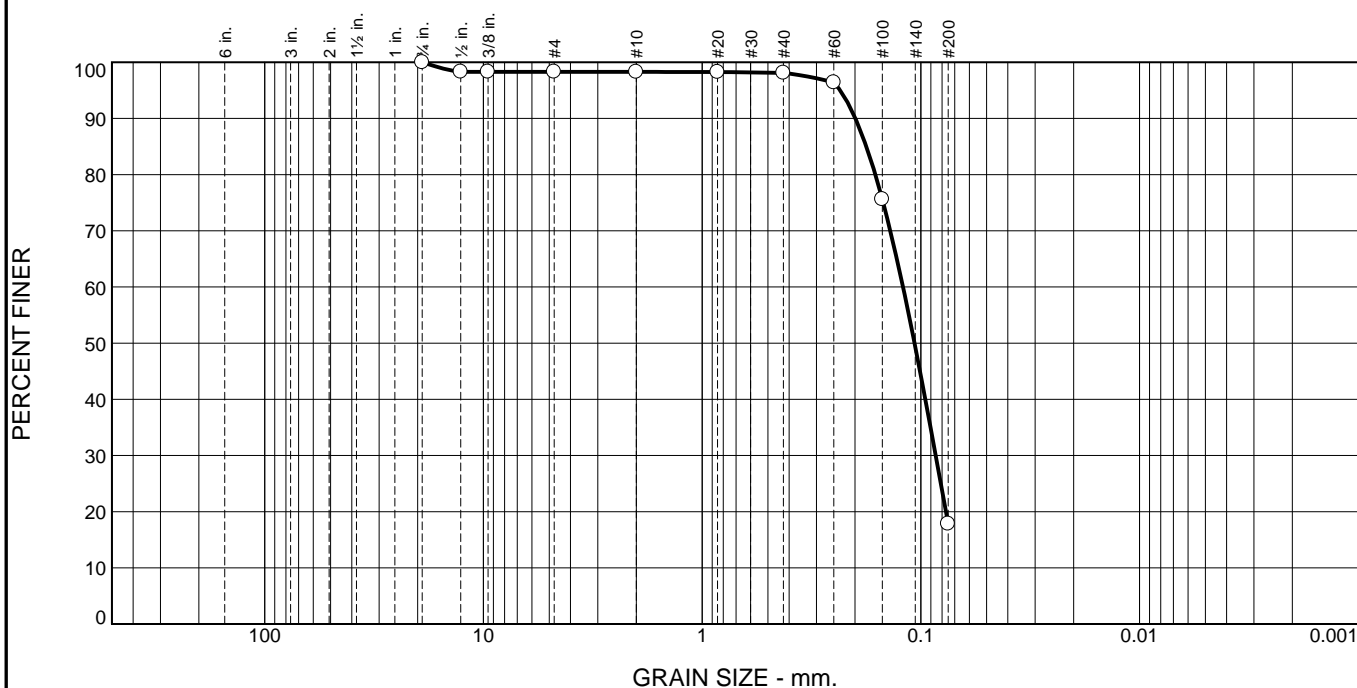
Date Received: 10/4/2017                      Date Tested: 10/17/2017  
 Tested By: RZ  
 Checked By: MP  
 Title: Laboratory Manager

Source of Sample: B-13                      Depth: 8-10'  
 Sample Number: S-5

Date Sampled: 9/19-20/2017

<b>CDM Smith</b>  <b>Boston, Massachusetts</b>	<b>Client:</b> Town of Salem <b>Project:</b> South Broadway Watermain Replacement Salem, NH
	<b>Project No:</b> 25972-215427 <b>Figure</b>

# Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	1.7	0.0	0.2	80.2	17.9	

Test Results (ASTM D422 & ASTM D1140)			
Opening Size	Percent Finer	Spec.* (Percent)	Pass? (X=Fail)
.75"	100.0		
.5"	98.3		
.375"	98.3		
#4	98.3		
#10	98.3		
#20	98.3		
#40	98.1		
#60	96.4		
#100	75.6		
#200	17.9		

\* (no specification provided)

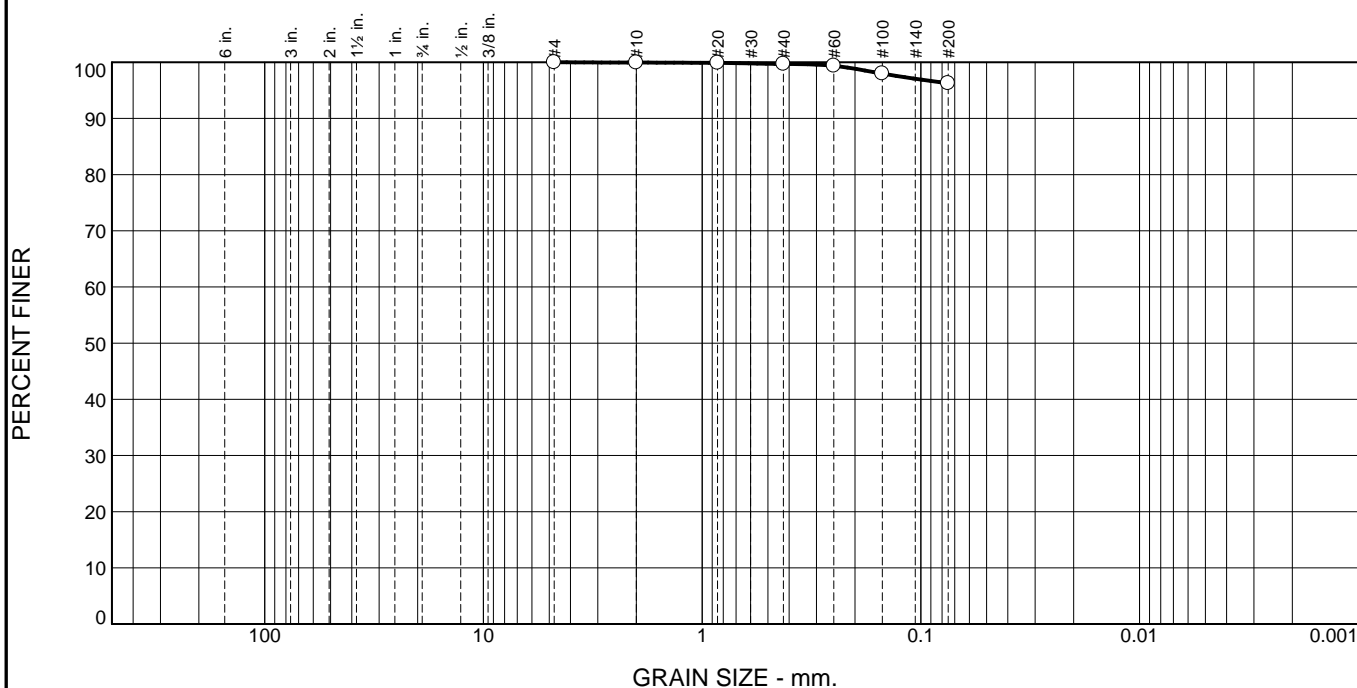
<b>Material Description</b>	
Light brown silty sand	
<b>Atterberg Limits (ASTM D 4318)</b>	
PL=	LL= PI=
<b>Classification</b>	
USCS (D 2487)= SM	AASHTO (M 145)= A-2-4(0)
<b>Coefficients</b>	
D <sub>90</sub> = 0.1991	D <sub>85</sub> = 0.1774 D <sub>60</sub> = 0.1206
D <sub>50</sub> = 0.1069	D <sub>30</sub> = 0.0855 D <sub>15</sub> =
D <sub>10</sub> =	C <sub>u</sub> = C <sub>c</sub> =
<b>Remarks</b>	
As received MC = 25.1%	
Date Received: 10/4/2017	Date Tested: 10/17/2017
Tested By: RZ	
Checked By: MP	
Title: Laboratory Manager	

Source of Sample: B-14 Depth: 8-10'  
Sample Number: S-5

Date Sampled: 9/27/2017

<b>CDM Smith</b>  <b>Boston, Massachusetts</b>	<b>Client:</b> Town of Salem <b>Project:</b> South Broadway Watermain Replacement Salem, NH
	<b>Project No:</b> 25972-215427 <b>Figure</b>

# Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	0.0	0.1	0.2	3.4	96.3	

Test Results (ASTM D422 & ASTM D1140)			
Opening Size	Percent Finer	Spec.* (Percent)	Pass? (X=Fail)
#4	100.0		
#10	99.9		
#20	99.9		
#40	99.7		
#60	99.4		
#100	98.0		
#200	96.3		

\* (no specification provided)

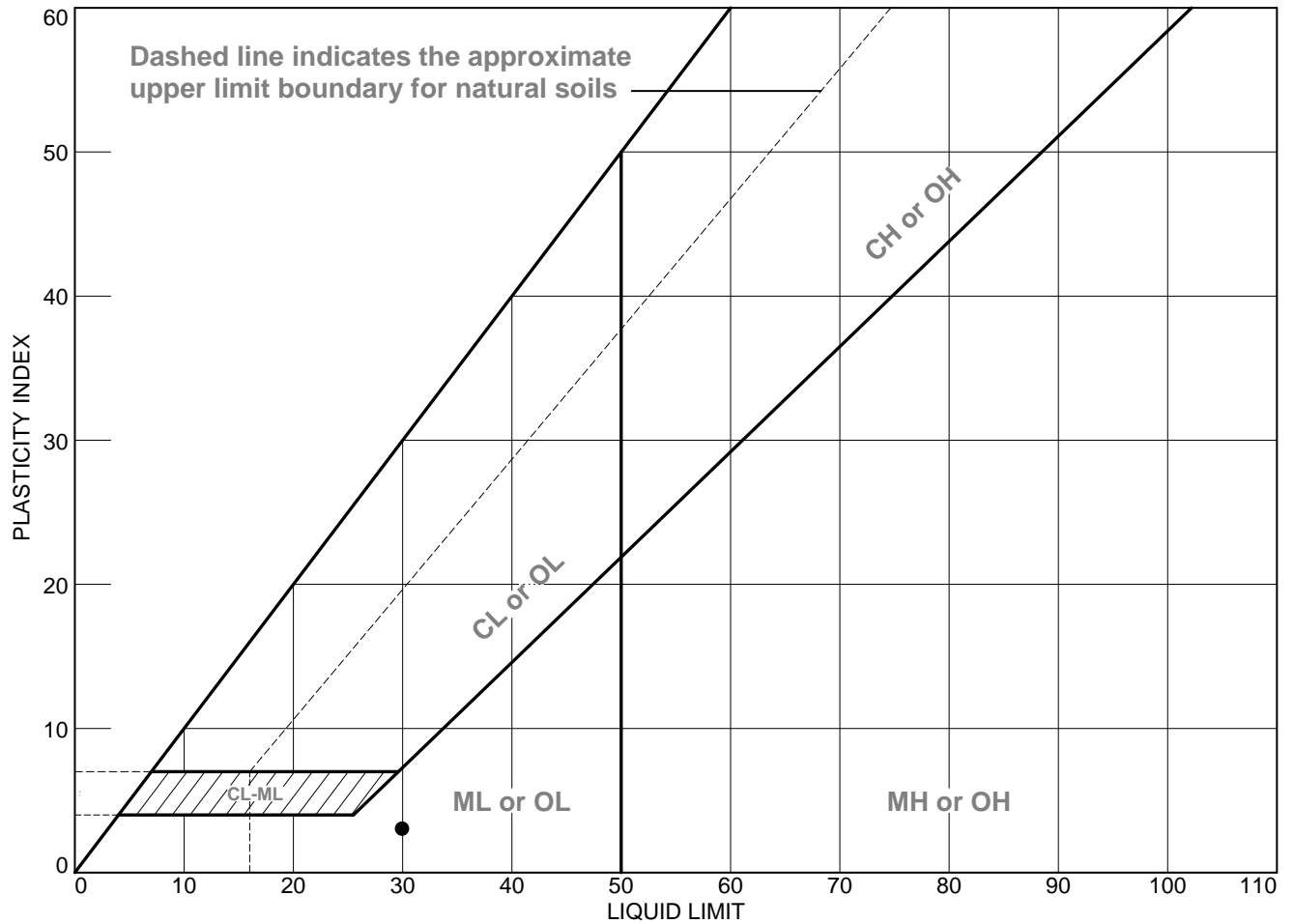
<b>Material Description</b>		
Gray silt		
<b>Atterberg Limits (ASTM D 4318)</b>		
PL= 27	LL= 30	PI= 3
<b>Classification</b>		
USCS (D 2487)= ML	AASHTO (M 145)= A-4(3)	
<b>Coefficients</b>		
D <sub>90</sub> =	D <sub>85</sub> =	D <sub>60</sub> =
D <sub>50</sub> =	D <sub>30</sub> =	D <sub>15</sub> =
D <sub>10</sub> =	C <sub>u</sub> =	C <sub>c</sub> =
<b>Remarks</b>		
As received MC = 31.5%		
<b>Date Received:</b> 10/4/2017		<b>Date Tested:</b> 10/17/2017
<b>Tested By:</b> RZ		
<b>Checked By:</b> MP		
<b>Title:</b> Laboratory Manager		

Source of Sample: B-14      Depth: 19-21'  
Sample Number: S-7

Date Sampled: 9/27/2017

<b>CDM Smith</b>  <b>Boston, Massachusetts</b>		<b>Client:</b> Town of Salem <b>Project:</b> South Broadway Watermain Replacement Salem, NH <b>Project No:</b> 25972-215427
		<b>Figure</b>

# LIQUID AND PLASTIC LIMITS TEST REPORT



SOIL DATA								
SYMBOL	SOURCE	SAMPLE NO.	DEPTH	NATURAL WATER CONTENT (%)	PLASTIC LIMIT (%)	LIQUID LIMIT (%)	PLASTICITY INDEX (%)	USCS
●	B-14	S-7	19-21'	31.5	27	30	3	ML

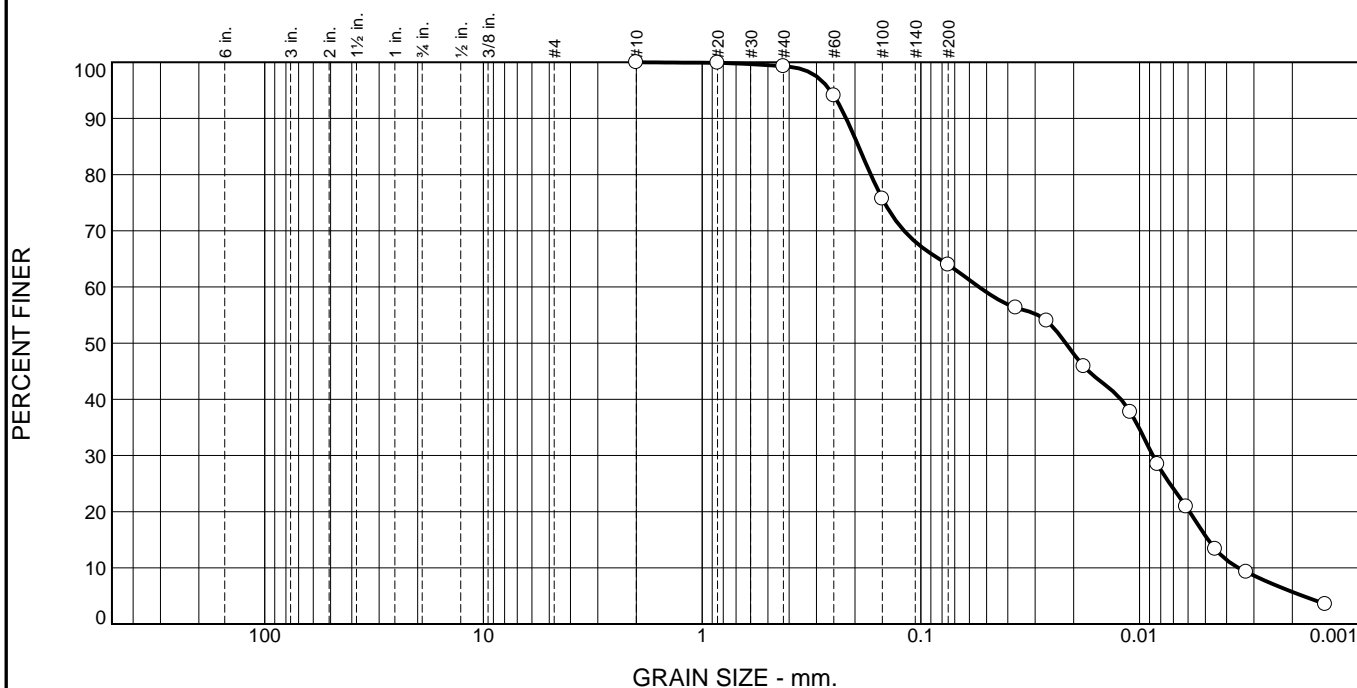
<b>CDM Smith</b>  <b>Boston, Massachusetts</b>		<b>Client:</b> Town of Salem <b>Project:</b> South Broadway Watermain Replacement Salem, NH <b>Project No.:</b> 25972-215427
--	--	---

Figure

Tested By: RZ Checked By: MP



# Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	0.0	0.0	0.7	35.4	48.2	15.7

Test Results (ASTM D 422 & ASTM D 1140)			
Opening Size	Percent Finer	Spec.* (Percent)	Pass? (X=Fail)
#10	100.0		
#20	99.9		
#40	99.3		
#60	94.1		
#100	75.7		
#200	63.9		
0.036 mm.	56.3		
0.026 mm.	54.0		
0.018 mm.	45.9		
0.011 mm.	37.7		
0.008 mm.	28.4		
0.006 mm.	20.9		
0.004 mm.	13.4		
0.003 mm.	9.3		
0.001 mm.	3.5		

\* (no specification provided)

**Material Description**  
Light brown sandy silt

**Atterberg Limits (ASTM D 4318)**  
PL=                      LL=                      PI=

**Classification**  
USCS (D 2487)= ML                      AASHTO (M 145)= A-4(0)

**Coefficients**  
D<sub>90</sub>= 0.2195                      D<sub>85</sub>= 0.1924                      D<sub>60</sub>= 0.0542  
D<sub>50</sub>= 0.0217                      D<sub>30</sub>= 0.0087                      D<sub>15</sub>= 0.0049  
D<sub>10</sub>= 0.0035                      C<sub>u</sub>= 15.37                      C<sub>c</sub>= 0.40

**Remarks**  
As received MC = 23.2%

**Date Received:** 10/4/2017                      **Date Tested:** 10/17/2017  
**Tested By:** GW  
**Checked By:** MP  
**Title:** Laboratory Manager

**Source of Sample:** B-15                      **Depth:** 5-7'  
**Sample Number:** S-1

**Date Sampled:** 4/13/2017

<b>CDM Smith</b>  <b>Boston, Massachusetts</b>	<b>Client:</b> Town of Salem
	<b>Project:</b> South Broadway Watermain Replacement Salem, NH
<b>Project No:</b> 25972-215427	<b>Figure</b>

PERCENT FINER

SIEVE SIZE

Sieve Size	Percent Finer (%)
6 in.	100
3 in.	100
2 in.	100
1½ in.	100
1 in.	100
¾ in.	100
½ in.	95
3/8 in.	94
#4	90
#10	88
#20	87
#30	86
#40	85
#60	80
#100	68
#140	38
#200	38

% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	10.5	2.0	2.5	47.5	37.5	

Test Results (ASTM D422 & ASTM D1140)			
Opening Size	Percent Finer	Spec.* (Percent)	Pass? (X=Fail)
.75"	100.0		
.5"	94.0		
.375"	93.4		
#4	89.5		
#10	87.5		
#20	86.3		
#40	85.0		
#60	79.9		
#100	67.4		
#200	37.5		

### Material Description

Gray silty sand

### Atterberg Limits (ASTM D 4318)

PL=                      LL=                      PI=

USCS (D 2487)= SM Classification AASHTO (M 145)= A-4(0)

### Coefficients

D90= 5.1850	D85= 0.4236	D60= 0.1230
D50= 0.0977	D30=	D15=
D10=	Cu=	Cc=

### Remarks

As received MC = 21.0%

**Date Received:** 10/4/2017      **Date Tested:** 10/17/2017

**Tested By:** RZ

**Checked By:** MP

**Title:** Laboratory Manager

**Source of Sample:** B-15      **Depth:** 9-11'  
**Sample Number:** S-3

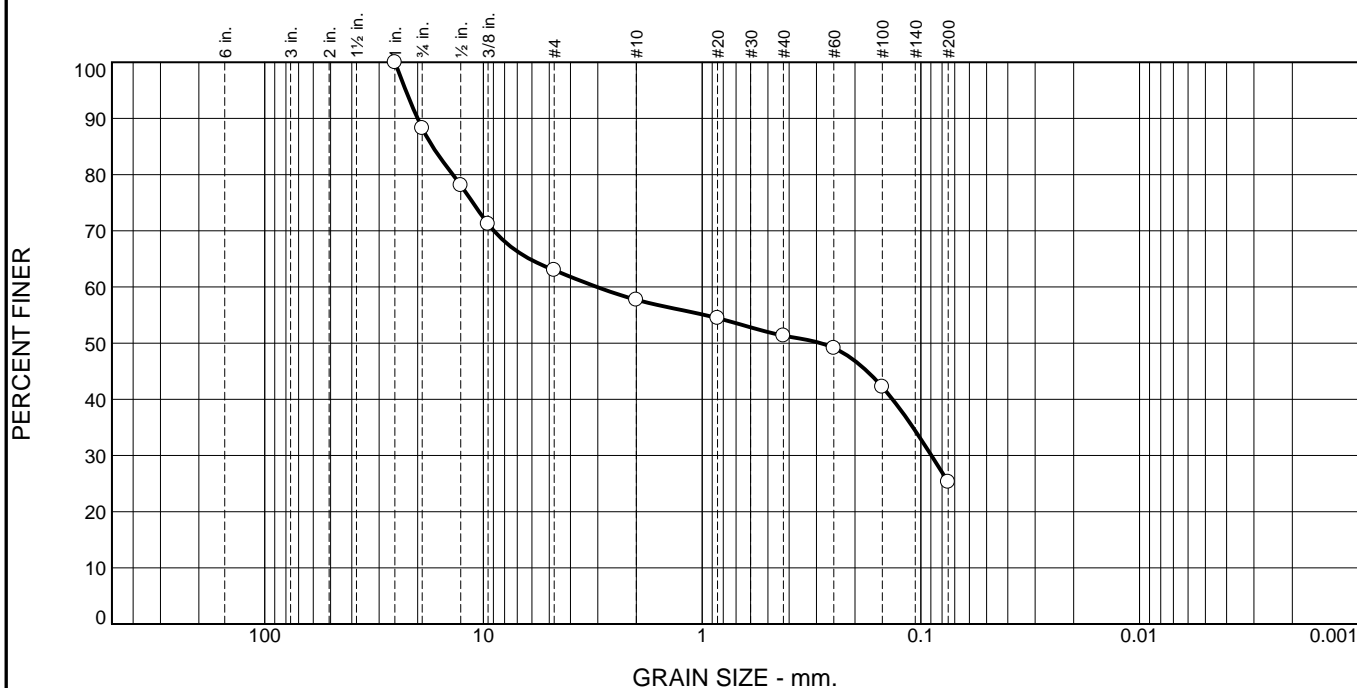
Date Sampled: 4/13/2017

**CDM Smith**  
**Boston, Massachusetts**

**Client:** Town of Salem  
**Project:** South Broadway Watermain Replacement  
Salem, NH  
**Project No:** 25972-215427

### Figure

# Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	11.7	25.3	5.3	6.4	26.0	25.3	

Test Results (ASTM D422 & ASTM D1140)			
Opening Size	Percent Finer	Spec.* (Percent)	Pass? (X=Fail)
1"	100.0		
3/4"	88.3		
.5"	78.1		
.375"	71.2		
#4	63.0		
#10	57.7		
#20	54.5		
#40	51.3		
#60	49.1		
#100	42.2		
#200	25.3		

\* (no specification provided)

**Material Description**  
Gray silty sand with gravel

**Atterberg Limits (ASTM D 4318)**  
 PL=                      LL=                      PI=

**Classification**  
 USCS (D 2487)= SM                      AASHTO (M 145)= A-2-4(0)

**Coefficients**  
 D<sub>90</sub>= 20.0112                      D<sub>85</sub>= 17.0797                      D<sub>60</sub>= 3.0312  
 D<sub>50</sub>= 0.2873                      D<sub>30</sub>= 0.0894                      D<sub>15</sub>=  
 D<sub>10</sub>=                      C<sub>u</sub>=                      C<sub>c</sub>=

**Remarks**  
As received MC = 12.7%

**Date Received:** 10/4/2017                      **Date Tested:** 10/17/2017  
**Tested By:** RZ  
**Checked By:** MP  
**Title:** Laboratory Manager

**Source of Sample:** B-15                      **Depth:** 14-16'  
**Sample Number:** S-4

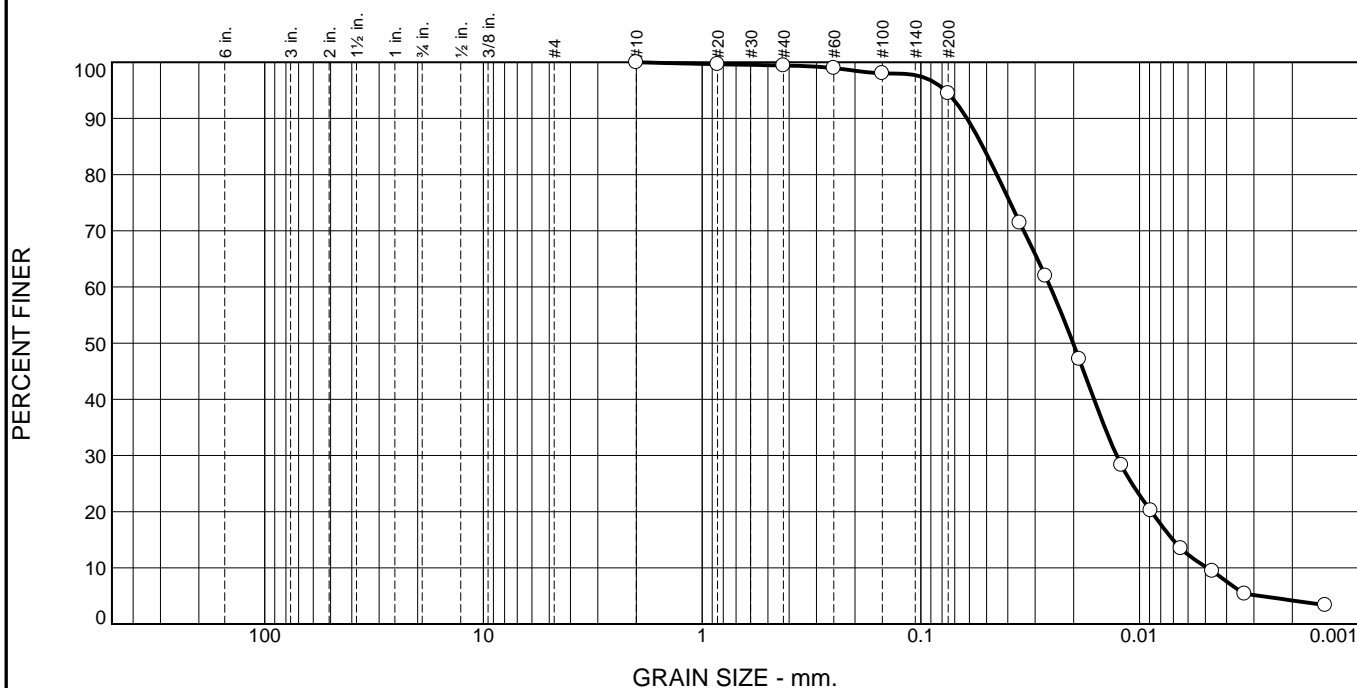
**Date Sampled:** 4/13/2017

**CDM Smith**  
  
**Boston, Massachusetts**

**Client:** Town of Salem  
**Project:** South Broadway Watermain Replacement  
 Salem, NH  
**Project No:** 25972-215427

**Figure**

# Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	0.0	0.0	0.6	4.9	84.3	10.2

Test Results (ASTM D422 & ASTM D1140)			
Opening Size	Percent Finer	Spec.* (Percent)	Pass? (X=Fail)
#10	100.0		
#20	99.6		
#40	99.4		
#60	99.0		
#100	98.0		
#200	94.5		
0.0353 mm.	71.4		
0.0270 mm.	62.0		
0.0189 mm.	47.2		
0.0121 mm.	28.3		
0.0089 mm.	20.2		
0.0065 mm.	13.5		
0.0047 mm.	9.4		
0.0033 mm.	5.4		
0.0014 mm.	3.4		

\* (no specification provided)

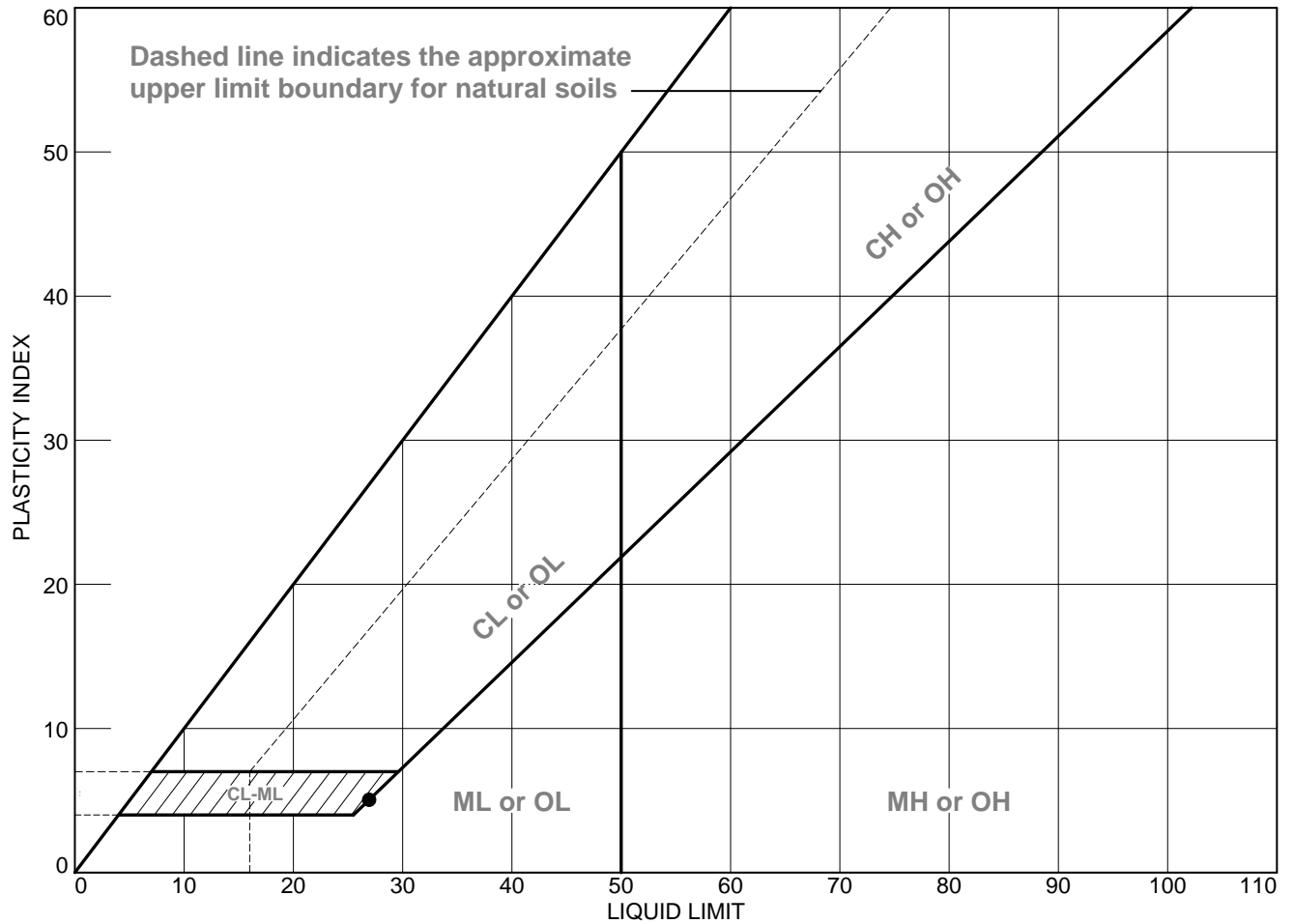
<b>Material Description</b>	
Brown silt	
<b>Atterberg Limits (ASTM D 4318)</b>	
PL=	LL= PI=
<b>Classification</b>	
USCS (D 2487)= ML	AASHTO (M 145)= A-4(0)
<b>Coefficients</b>	
D <sub>90</sub> = 0.0615	D <sub>85</sub> = 0.0521 D <sub>60</sub> = 0.0256
D <sub>50</sub> = 0.0201	D <sub>30</sub> = 0.0127 D <sub>15</sub> = 0.0070
D <sub>10</sub> = 0.0049	C <sub>u</sub> = 5.23 C <sub>c</sub> = 1.29
<b>Remarks</b>	
As received MC = 28.1%	
<b>Date Received:</b> 10/5/2017	<b>Date Tested:</b> 10/18/2017
<b>Tested By:</b> MP/GW	
<b>Checked By:</b> MP	
<b>Title:</b> Laboratory Manager	

Source of Sample: B-16A Depth: 8-10'  
Sample Number: S-1

Date Sampled: 9/18/2017

<b>CDM Smith</b>  <b>Boston, Massachusetts</b>	<b>Client:</b> Town of Salem <b>Project:</b> South Broadway Watermain Replacement Salem, NH
	<b>Project No:</b> 25972-215427 <b>Figure</b>

# LIQUID AND PLASTIC LIMITS TEST REPORT



SOIL DATA								
SYMBOL	SOURCE	SAMPLE NO.	DEPTH	NATURAL WATER CONTENT (%)	PLASTIC LIMIT (%)	LIQUID LIMIT (%)	PLASTICITY INDEX (%)	USCS
●	B-16A	S-4	23-25'	29.0	22	27	5	CL-ML

<b>CDM Smith</b>  <b>Boston, Massachusetts</b>		<b>Client:</b> Town of Salem <b>Project:</b> South Broadway Watermain Replacement Salem, NH <b>Project No.:</b> 25972-215427
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Figure

Tested By: RZ Checked By: MP

# CDM Smith

## Geotechnical Engineering Laboratory

### Standard Test Method for Moisture, Ash, and Organic Matter of Peat and Other Organic Soils (ASTM D2974)

Client: Town of Salem  
Project Name: South Broadway WM Replacement  
Project Location: Salem, NH  
Project Number: 25972-215427  
Boring Number: B-17  
Sample Number: S-2  
Sample Depth (ft): 2.5-4'  
Sample Date: 9/15/2017

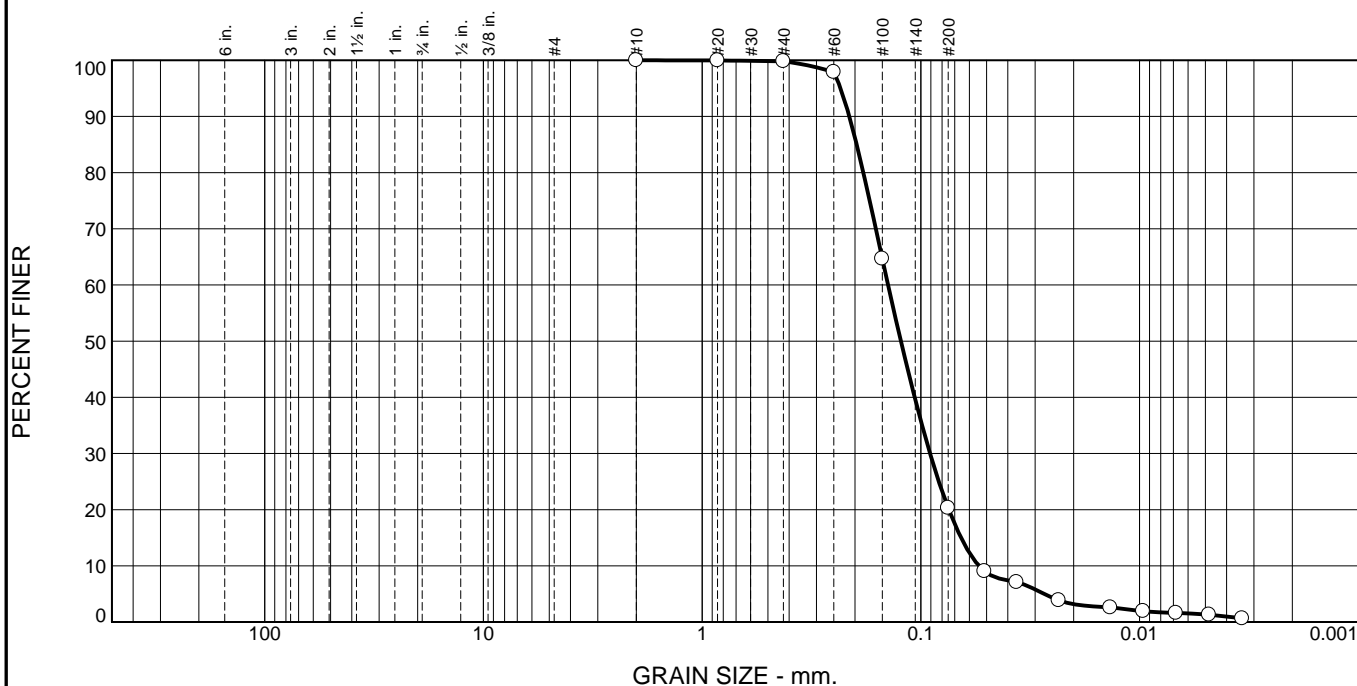
Tested By: MP  
Test Date: 10/17/2017

Procedure: C  
Temperature: 440° C

AS RECEIVED MOISTURE CONTENT	
Tin Mass (g)	105.23
Wet Mass of Sample & Tin (g)	183.81
Dry Mass of Sample & Tin (g)	167.84
Mass of Water (g)	15.97
Mass of Dry Soil (g)	62.61
<b>Moisture Content (%)</b>	<b>25.5</b>

ASH CONTENT	
Porcelain Dish Mass (g)	105.23
Porcelain Dish + Oven Dried Soil (g)	167.84
Mass of Oven Dried Soil (g)	62.61
Mass of Dish & Burned Soil (g)	166.59
Mass of Burned Soil (g)	61.36
Mass of Organic Material (g)	1.25
Ash Content (%)	98.0
<b>Organic Content (%)</b>	<b>2.0</b>

# Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	0.0	0.0	0.2	79.5	19.0	1.3

Test Results (ASTM D422 & ASTM D1140)			
Opening Size	Percent Finer	Spec.* (Percent)	Pass? (X=Fail)
#10	100.0		
#20	100.0		
#40	99.8		
#60	97.9		
#100	64.7		
#200	20.3		
0.0511 mm.	9.0		
0.0365 mm.	7.1		
0.0234 mm.	3.9		
0.0136 mm.	2.6		
0.0096 mm.	1.9		
0.0068 mm.	1.6		
0.0048 mm.	1.3		
0.0034 mm.	0.6		

\* (no specification provided)

**Material Description**  
Light brown and gray silty sand

**Atterberg Limits (ASTM D 4318)**  
PL=                      LL=                      PI=

**Classification**  
USCS (D 2487)= SM                      AASHTO (M 145)= A-2-4(0)

**Coefficients**  
D<sub>90</sub>= 0.2122                      D<sub>85</sub>= 0.1964                      D<sub>60</sub>= 0.1412  
D<sub>50</sub>= 0.1233                      D<sub>30</sub>= 0.0907                      D<sub>15</sub>= 0.0655  
D<sub>10</sub>= 0.0543                      C<sub>u</sub>= 2.60                      C<sub>c</sub>= 1.07

**Remarks**  
As received MC = 25.0%

**Date Received:** 10/5/2017                      **Date Tested:** 10/18/2017  
**Tested By:** GW/MP  
**Checked By:** MP  
**Title:** Laboratory Manager

**Source of Sample:** B-17                      **Depth:** 8-10'  
**Sample Number:** S-5

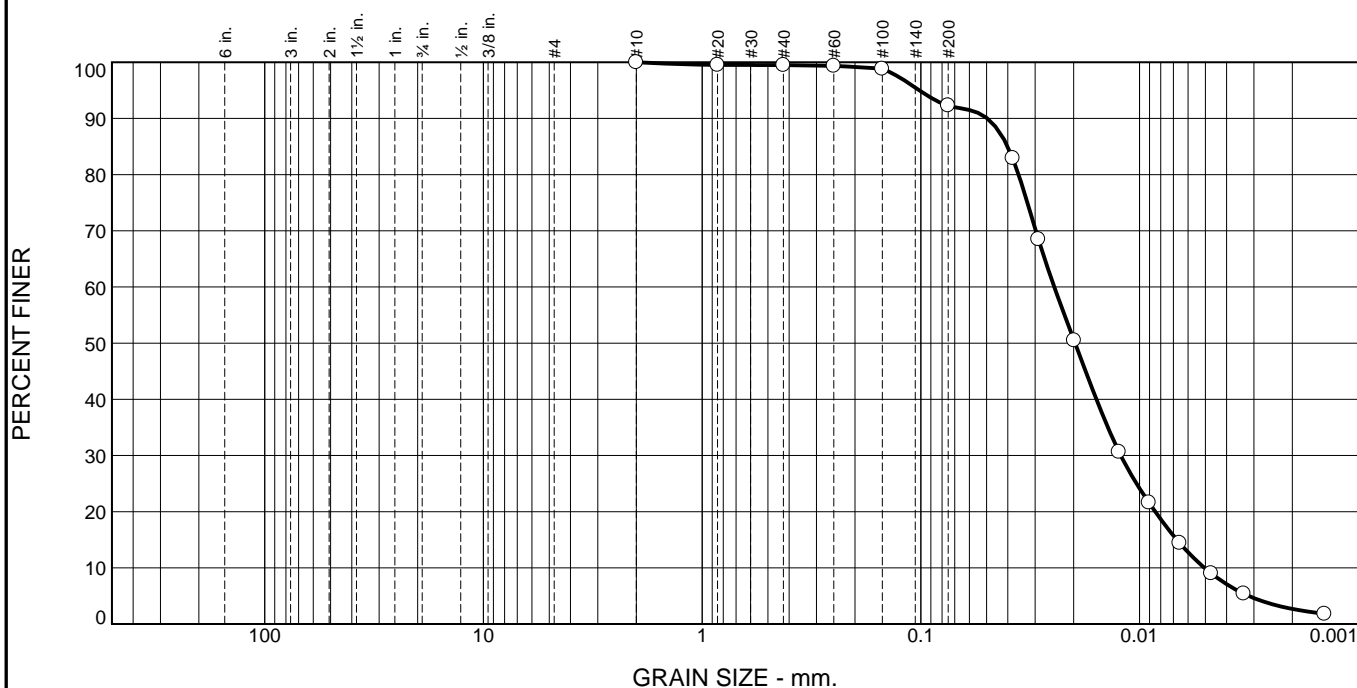
**Date Sampled:** 9/15/2017

**CDM Smith**  
**Boston, Massachusetts**

**Client:** Town of Salem  
**Project:** South Broadway Watermain Replacement  
Salem, NH  
**Project No:** 25972-215427

**Figure**

# Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	0.0	0.0	0.5	7.2	82.4	9.9

Test Results (ASTM D422 & ASTM D1140)			
Opening Size	Percent Finer	Spec.* (Percent)	Pass? (X=Fail)
#10	100.0		
#20	99.5		
#40	99.5		
#60	99.3		
#100	98.8		
#200	92.3		
0.0380 mm.	82.9		
0.0290 mm.	68.5		
0.0199 mm.	50.5		
0.0124 mm.	30.6		
0.0091 mm.	21.6		
0.0066 mm.	14.4		
0.0047 mm.	9.0		
0.0033 mm.	5.4		
0.0014 mm.	1.8		

\* (no specification provided)

<b>Material Description</b>	
Brown silt	
<b>Atterberg Limits (ASTM D 4318)</b>	
PL=	LL= PI=
<b>Classification</b>	
USCS (D 2487)= ML	AASHTO (M 145)= A-4(0)
<b>Coefficients</b>	
D <sub>90</sub> = 0.0496	D <sub>85</sub> = 0.0401 D <sub>60</sub> = 0.0245
D <sub>50</sub> = 0.0197	D <sub>30</sub> = 0.0122 D <sub>15</sub> = 0.0068
D <sub>10</sub> = 0.0050	C <sub>u</sub> = 4.86 C <sub>c</sub> = 1.20
<b>Remarks</b>	
As received MC = 29.7%	
<b>Date Received:</b> 10/5/2017	<b>Date Tested:</b> 10/18/2017
<b>Tested By:</b> GW/MP	
<b>Checked By:</b> MP	
<b>Title:</b> Laboratory Manager	

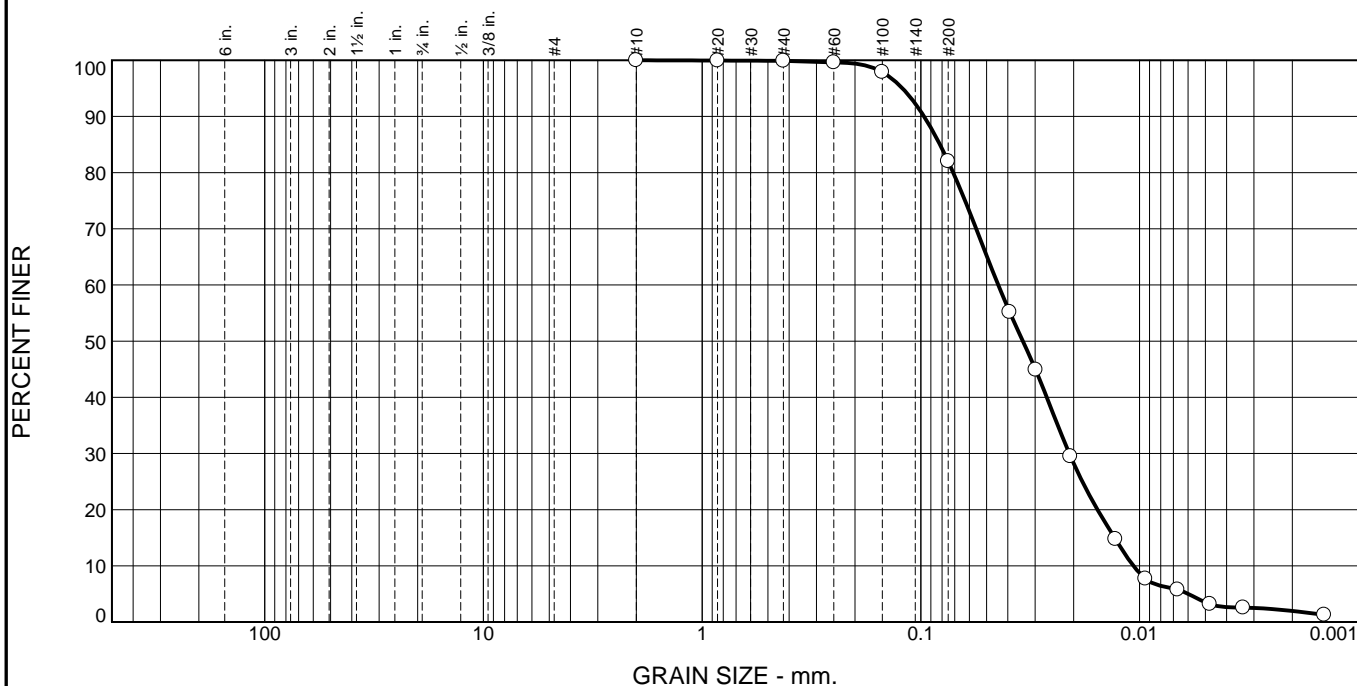
Source of Sample: B-17 Depth: 19-21'  
Sample Number: S-7

Date Sampled: 9/15/2017

<b>CDM Smith</b>  <b>Boston, Massachusetts</b>	<b>Client:</b> Town of Salem <b>Project:</b> South Broadway Watermain Replacement Salem, NH
	<b>Project No:</b> 25972-215427 <b>Figure</b>



# Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	0.0	0.0	0.1	17.9	78.5	3.5

Test Results (ASTM D422 & ASTM D1140)			
Opening Size	Percent Finer	Spec.* (Percent)	Pass? (X=Fail)
#10	100.0		
#20	99.9		
#40	99.9		
#60	99.6		
#100	97.9		
#200	82.0		
0.0393 mm.	55.2		
0.0298 mm.	44.9		
0.0207 mm.	29.5		
0.0129 mm.	14.8		
0.0094 mm.	7.7		
0.0067 mm.	5.8		
0.0048 mm.	3.2		
0.0034 mm.	2.6		
0.0014 mm.	1.3		

\* (no specification provided)

**Material Description**  
Brown silt with sand

**Atterberg Limits (ASTM D 4318)**  
PL=                      LL=                      PI=

**Classification**  
USCS (D 2487)= ML                      AASHTO (M 145)= A-4(0)

**Coefficients**  
D<sub>90</sub>= 0.0967                      D<sub>85</sub>= 0.0817                      D<sub>60</sub>= 0.0443  
D<sub>50</sub>= 0.0342                      D<sub>30</sub>= 0.0210                      D<sub>15</sub>= 0.0130  
D<sub>10</sub>= 0.0107                      C<sub>u</sub>= 4.15                      C<sub>c</sub>= 0.93

**Remarks**  
As received MC = 24.5%

**Date Received:** 10/5/2017                      **Date Tested:** 10/18/2017  
**Tested By:** GW/MP  
**Checked By:** MP  
**Title:** Laboratory Manager

**Source of Sample:** B-18                      **Depth:** 6-8'  
**Sample Number:** S-4

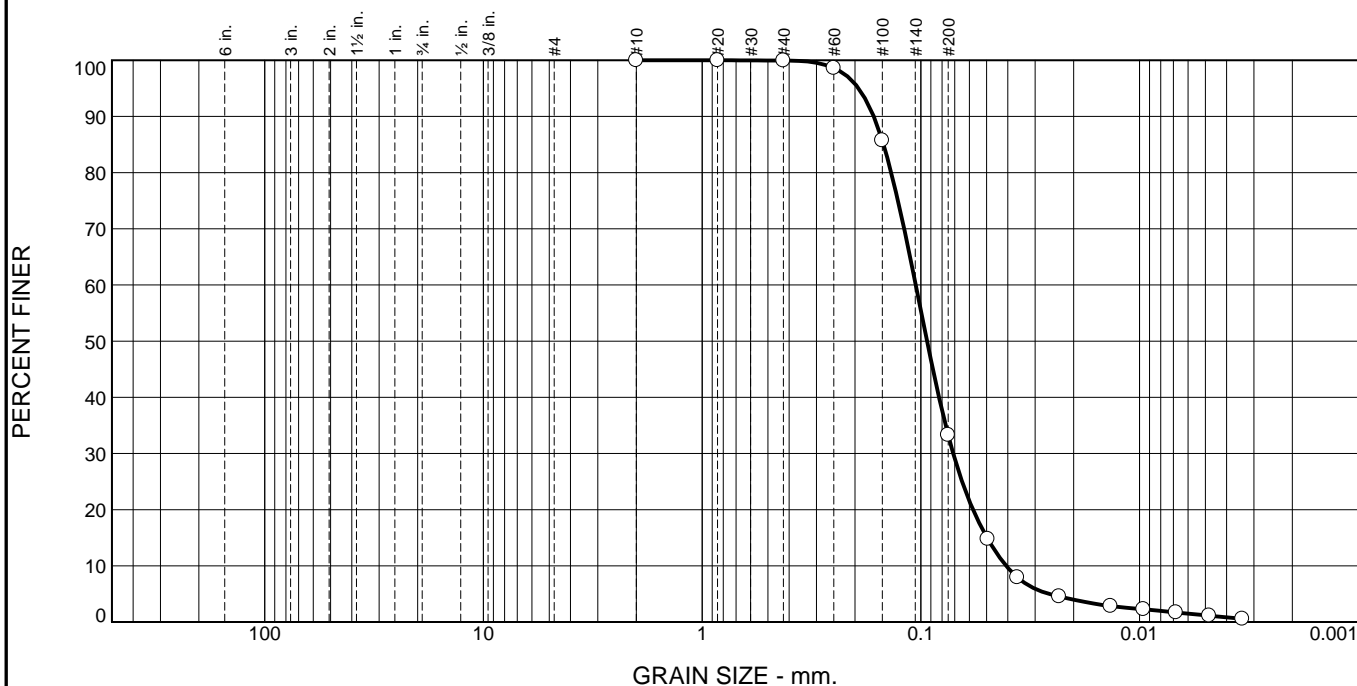
**Date Sampled:** 9/15/2017

**CDM Smith**  
**Boston, Massachusetts**

**Client:** Town of Salem  
**Project:** South Broadway Watermain Replacement  
Salem, NH  
**Project No:** 25972-215427

**Figure**

# Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	0.0	0.0	0.1	66.6	32.1	1.2

Test Results (ASTM D422 & ASTM D1140)			
Opening Size	Percent Finer	Spec.* (Percent)	Pass? (X=Fail)
#10	100.0		
#20	100.0		
#40	99.9		
#60	98.6		
#100	85.7		
#200	33.3		
0.0494 mm.	14.8		
0.0362 mm.	8.0		
0.0233 mm.	4.5		
0.0136 mm.	2.8		
0.0096 mm.	2.3		
0.0068 mm.	1.7		
0.0048 mm.	1.1		
0.0034 mm.	0.6		

\* (no specification provided)

**Material Description**  
Brown silty sand

**Atterberg Limits (ASTM D 4318)**  
PL=                      LL=                      PI=

**Classification**  
USCS (D 2487)= SM                      AASHTO (M 145)= A-2-4(0)

**Coefficients**  
D<sub>90</sub>= 0.1642                      D<sub>85</sub>= 0.1481                      D<sub>60</sub>= 0.1055  
D<sub>50</sub>= 0.0936                      D<sub>30</sub>= 0.0711                      D<sub>15</sub>= 0.0498  
D<sub>10</sub>= 0.0407                      C<sub>u</sub>= 2.59                      C<sub>c</sub>= 1.18

**Remarks**  
As received MC = 25.8%

**Date Received:** 10/5/2017                      **Date Tested:** 10/18/2017  
**Tested By:** GW/MP  
**Checked By:** MP  
**Title:** Laboratory Manager

**Source of Sample:** B-18                      **Depth:** 14-16'  
**Sample Number:** S-6

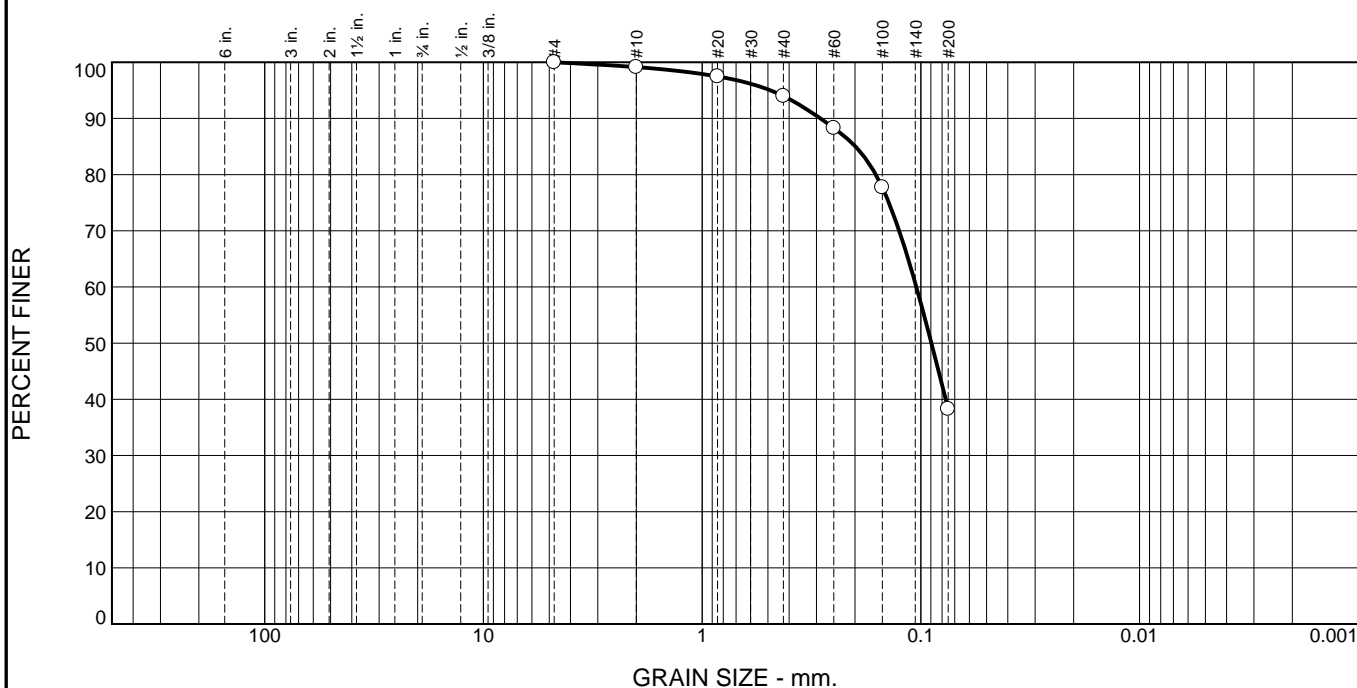
**Date Sampled:** 9/15/2017

**CDM Smith**  
**Boston, Massachusetts**

**Client:** Town of Salem  
**Project:** South Broadway Watermain Replacement  
Salem, NH  
**Project No:** 25972-215427

**Figure**

# Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	0.0	0.9	5.1	55.7	38.3	

Test Results (ASTM D422 & ASTM D1140)			
Opening Size	Percent Finer	Spec.* (Percent)	Pass? (X=Fail)
#4	100.0		
#10	99.1		
#20	97.5		
#40	94.0		
#60	88.3		
#100	77.7		
#200	38.3		

\* (no specification provided)

**Material Description**  
Light brown silty sand

**Atterberg Limits (ASTM D 4318)**  
 PL= \_\_\_\_\_ LL= \_\_\_\_\_ PI= \_\_\_\_\_

**Classification**  
 USCS (D 2487)= SM AASHTO (M 145)= A-4(0)

**Coefficients**  
 D<sub>90</sub>= 0.2879 D<sub>85</sub>= 0.1988 D<sub>60</sub>= 0.1048  
 D<sub>50</sub>= 0.0894 D<sub>30</sub>= \_\_\_\_\_ D<sub>15</sub>= \_\_\_\_\_  
 D<sub>10</sub>= \_\_\_\_\_ C<sub>u</sub>= \_\_\_\_\_ C<sub>c</sub>= \_\_\_\_\_

**Remarks**  
As received MC = 25.5%

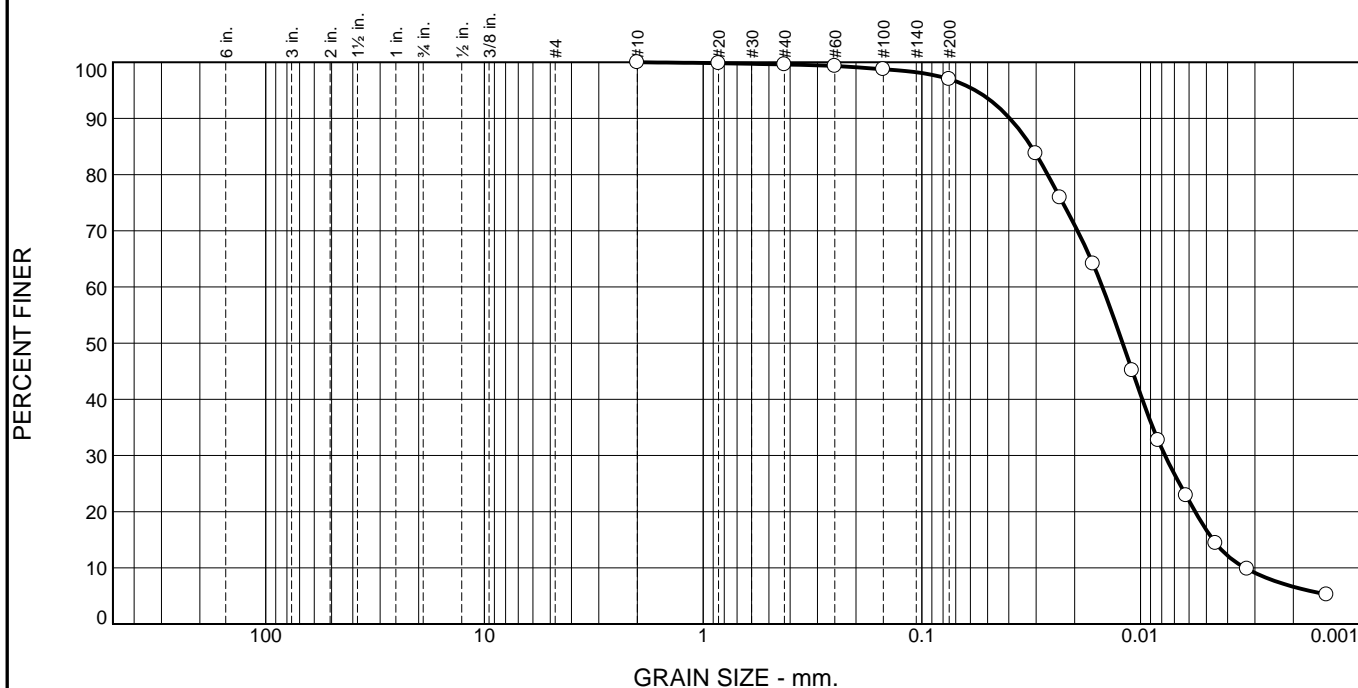
**Date Received:** 10/5/2017 **Date Tested:** 10/17/2017  
**Tested By:** RZ  
**Checked By:** MP  
**Title:** Laboratory Manager

**Source of Sample:** B-19 **Depth:** 4-6'  
**Sample Number:** S-2

**Date Sampled:** 9/14/2017

<b>CDM Smith</b>  <b>Boston, Massachusetts</b>	<b>Client:</b> Town of Salem
	<b>Project:</b> South Broadway Watermain Replacement Salem, NH
<b>Project No:</b> 25972-215427	<b>Figure</b>

# Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	0.0	0.0	0.4	2.6	80.3	16.7

Test Results (ASTM D422 & ASTM D1140)			
Opening Size	Percent Finer	Spec.* (Percent)	Pass? (X=Fail)
#10	100.0		
#20	99.8		
#40	99.6		
#60	99.3		
#100	98.8		
#200	97.0		
0.0302 mm.	83.8		
0.0234 mm.	75.9		
0.0165 mm.	64.1		
0.0109 mm.	45.2		
0.0083 mm.	32.7		
0.0062 mm.	22.9		
0.0045 mm.	14.4		
0.0033 mm.	9.8		
0.0014 mm.	5.2		

\* (no specification provided)

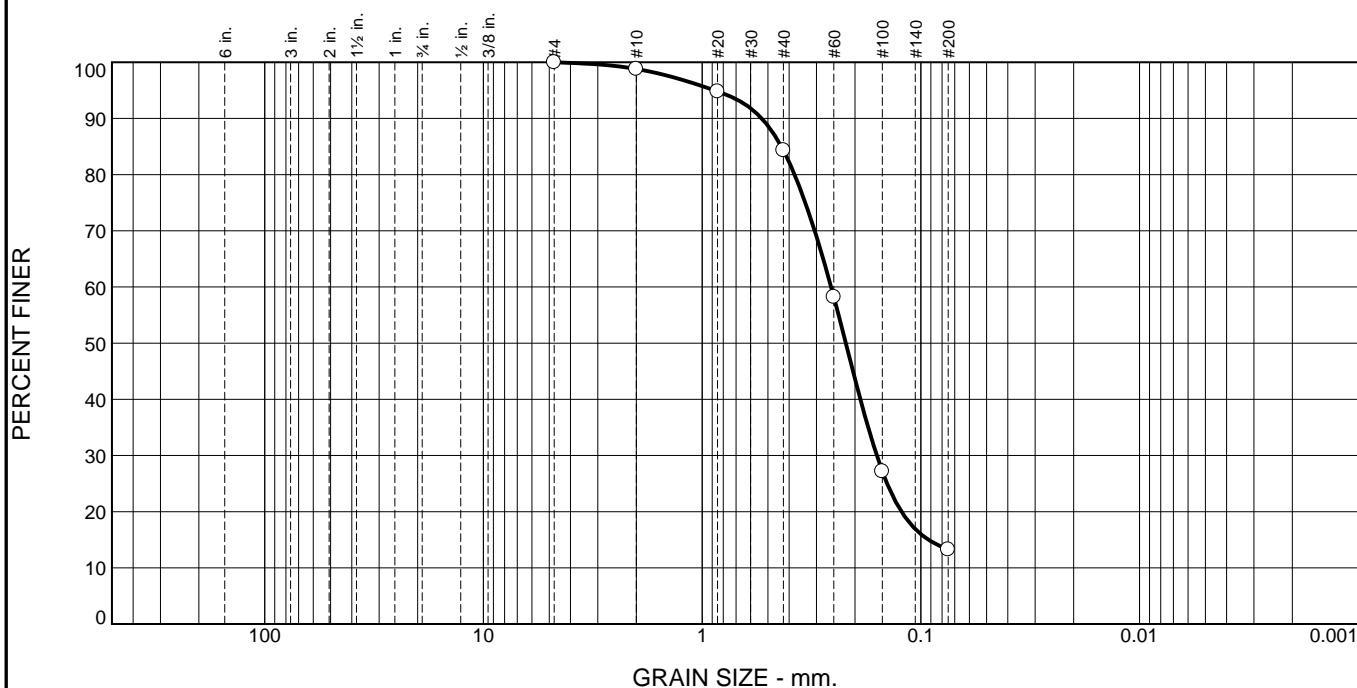
<b>Material Description</b>	
Gray silt	
<b>Atterberg Limits (ASTM D 4318)</b>	
PL=	LL= PI=
<b>Classification</b>	
USCS (D 2487)= ML	AASHTO (M 145)= A-4(0)
<b>Coefficients</b>	
D <sub>90</sub> = 0.0396	D <sub>85</sub> = 0.0316 D <sub>60</sub> = 0.0149
D <sub>50</sub> = 0.0121	D <sub>30</sub> = 0.0077 D <sub>15</sub> = 0.0047
D <sub>10</sub> = 0.0033	C <sub>u</sub> = 4.50 C <sub>c</sub> = 1.21
<b>Remarks</b>	
As received MC = 27.1%	
<b>Date Received:</b> 10/5/2017 <b>Date Tested:</b> 10/15/2017 <b>Tested By:</b> GW/MP <b>Checked By:</b> MP <b>Title:</b> Laboratory Manager	

Source of Sample: B-19      Depth: 14-16'  
Sample Number: S-5

Date Sampled: 9/14/2017

<b>CDM Smith</b>  <b>Boston, Massachusetts</b>	<b>Client:</b> Town of Salem <b>Project:</b> South Broadway Watermain Replacement Salem, NH
	<b>Project No:</b> 25972-215427 <b>Figure</b>

# Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	0.0	1.2	14.5	71.0	13.3	

Test Results (ASTM D 422 & ASTM D 1140)			
Opening Size	Percent Finer	Spec.* (Percent)	Pass? (X=Fail)
#4	100.0		
#10	98.8		
#20	94.8		
#40	84.3		
#60	58.2		
#100	27.2		
#200	13.3		

\* (no specification provided)

**Material Description**  
Black and brown silty sand

**Atterberg Limits (ASTM D 4318)**  
 PL=                      LL=                      PI=

**Classification**  
 USCS (D 2487)= SM                      AASHTO (M 145)= A-2-4(0)

**Coefficients**  
 D<sub>90</sub>= 0.5338                      D<sub>85</sub>= 0.4342                      D<sub>60</sub>= 0.2572  
 D<sub>50</sub>= 0.2203                      D<sub>30</sub>= 0.1590                      D<sub>15</sub>= 0.0922  
 D<sub>10</sub>=                      C<sub>u</sub>=                      C<sub>c</sub>=

**Remarks**  
As received MC = 30.1%

**Date Received:** 10/4/2017                      **Date Tested:** 10/17/2017  
**Tested By:** RZ  
**Checked By:** MP  
**Title:** Laboratory Manager

**Source of Sample:** B-21                      **Depth:** 4-6'  
**Sample Number:** S-3

**Date Sampled:** 9/13/2017

**CDM Smith**  
  
**Boston, Massachusetts**

**Client:** Town of Salem  
**Project:** South Broadway Watermain Replacement  
 Salem, NH  
**Project No:** 25972-215427

**Figure**

# CDM Smith

## Geotechnical Engineering Laboratory

### Standard Test Method for Moisture, Ash, and Organic Matter of Peat and Other Organic Soils (ASTM D2974)

Client: Town of Salem  
Project Name: South Broadway WM Replacement  
Project Location: Salem, NH  
Project Number: 25972-215427  
Boring Number: B-21  
Sample Number: S-3  
Sample Depth (ft): 4-6'  
Sample Date: 9/13/2017

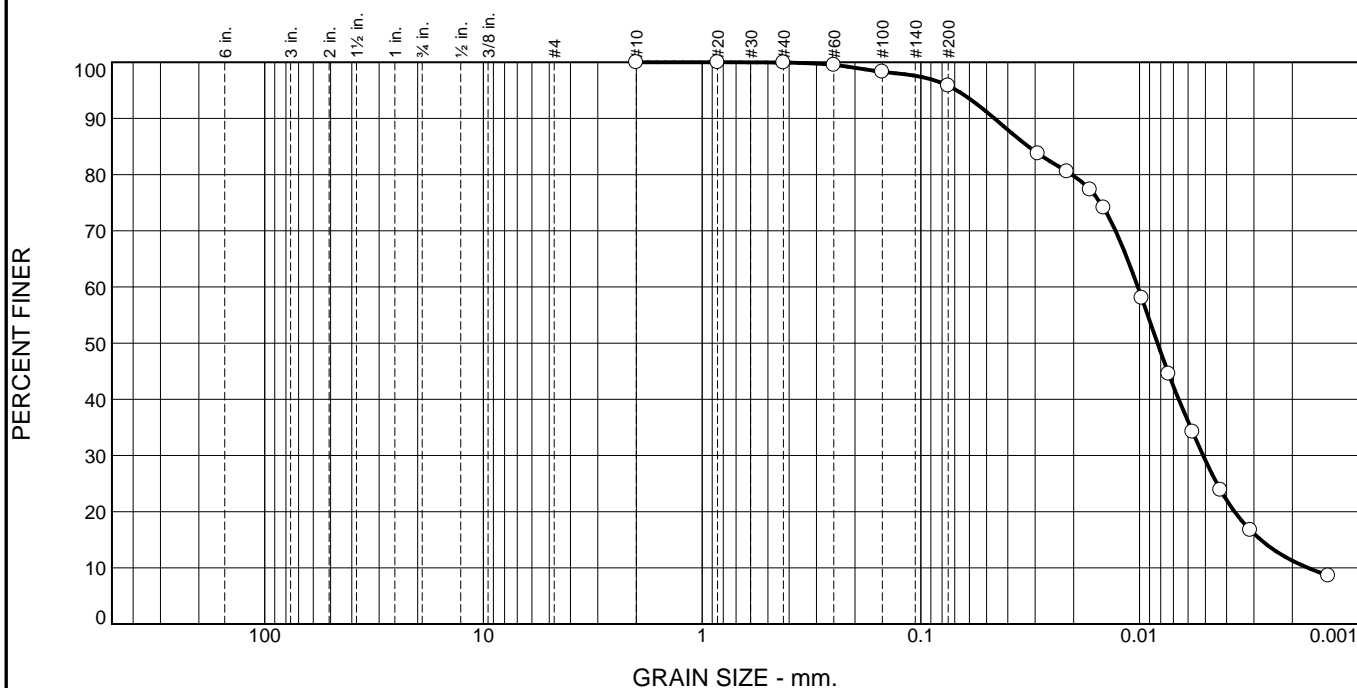
Tested By: MP  
Test Date: 10/17/2017

Procedure: C  
Temperature: 440° C

AS RECEIVED MOISTURE CONTENT	
Tin Mass (g)	78.10
Wet Mass of Sample & Tin (g)	137.76
Dry Mass of Sample & Tin (g)	123.96
Mass of Water (g)	13.80
Mass of Dry Soil (g)	45.86
<b>Moisture Content (%)</b>	<b>30.1</b>

ASH CONTENT	
Porcelain Dish Mass (g)	78.10
Porcelain Dish + Oven Dried Soil (g)	123.96
Mass of Oven Dried Soil (g)	45.86
Mass of Dish & Burned Soil (g)	121.92
Mass of Burned Soil (g)	43.82
Mass of Organic Material (g)	2.04
Ash Content (%)	95.6
<b>Organic Content (%)</b>	<b>4.4</b>

# Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	0.0	0.0	0.1	4.1	66.6	29.2

Test Results (ASTM D422 & ASTM D1140)			
Opening Size	Percent Finer	Spec.* (Percent)	Pass? (X=Fail)
#10	100.0		
#20	100.0		
#40	99.9		
#60	99.5		
#100	98.3		
#200	95.8		
0.0291 mm.	83.8		
0.0215 mm.	80.5		
0.0169 mm.	77.3		
0.0146 mm.	74.1		
0.0098 mm.	58.1		
0.0074 mm.	44.5		
0.0057 mm.	34.2		
0.0043 mm.	23.9		
0.0031 mm.	16.7		
0.0014 mm.	8.6		

\* (no specification provided)

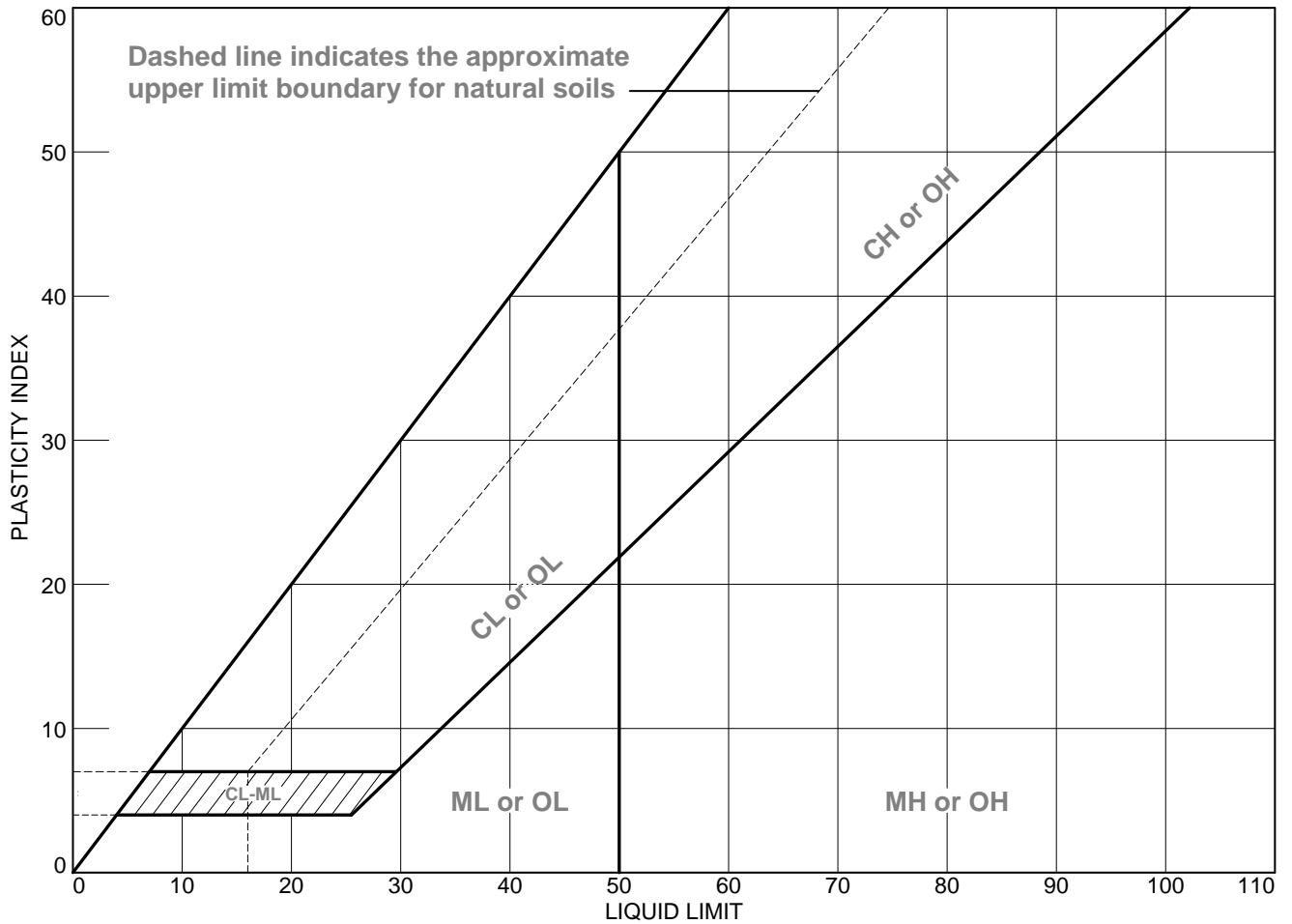
<b>Material Description</b>		
Gray silt		
<b>Atterberg Limits (ASTM D 4318)</b>		
PL= 23	LL= NV	PI= NP
<b>Classification</b>		
USCS (D 2487)= ML	AASHTO (M 145)= A-4(0)	
<b>Coefficients</b>		
D <sub>90</sub> = 0.0461	D <sub>85</sub> = 0.0322	D <sub>60</sub> = 0.0102
D <sub>50</sub> = 0.0083	D <sub>30</sub> = 0.0051	D <sub>15</sub> = 0.0028
D <sub>10</sub> = 0.0017	C <sub>u</sub> = 6.00	C <sub>c</sub> = 1.52
<b>Remarks</b>		
As received MC = 26.8%		
<b>Date Received:</b> 10/5/2017		<b>Date Tested:</b> 10/19/2017
<b>Tested By:</b> RZ/MP		
<b>Checked By:</b> MP		
<b>Title:</b> Laboratory Manager		

Source of Sample: B-21 Depth: 8-10'  
Sample Number: S-5

Date Sampled: 9/13/2017

<b>CDM Smith</b>  <b>Boston, Massachusetts</b>	<b>Client:</b> Town of Salem <b>Project:</b> South Broadway Watermain Replacement Salem, NH
	<b>Project No:</b> 25972-215427 <b>Figure</b>

# LIQUID AND PLASTIC LIMITS TEST REPORT



SOIL DATA								
SYMBOL	SOURCE	SAMPLE NO.	DEPTH	NATURAL WATER CONTENT (%)	PLASTIC LIMIT (%)	LIQUID LIMIT (%)	PLASTICITY INDEX (%)	USCS
●	B-21	S-5	8-10'	26.8	23	NV	NP	ML

**CDM Smith**  
**Boston, Massachusetts**

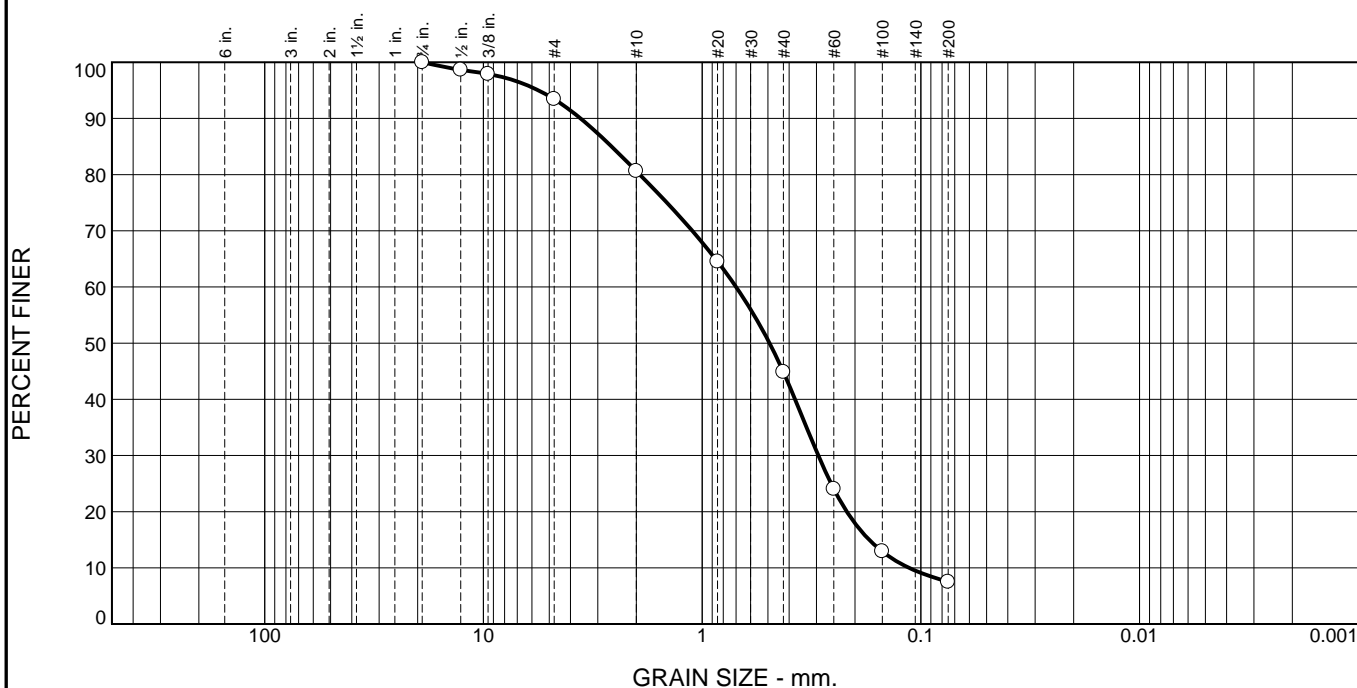
**Client:** Town of Salem  
**Project:** South Broadway Watermain Replacement  
Salem, NH  
**Project No.:** 25972-215427

**Figure**

**Tested By:** RZ **Checked By:** MP



# Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	6.6	12.8	35.7	37.4	7.5	

Test Results (ASTM D422 & ASTM D1140)			
Opening Size	Percent Finer	Spec.* (Percent)	Pass? (X=Fail)
.75"	100.0		
.5"	98.7		
.375"	97.9		
#4	93.4		
#10	80.6		
#20	64.5		
#40	44.9		
#60	24.0		
#100	12.9		
#200	7.5		

\* (no specification provided)

## Material Description

Light brown well-graded sand with silt

## Atterberg Limits (ASTM D 4318)

PL= LL= PI=

## Classification

USCS (D 2487)= SW-SM AASHTO (M 145)= A-1-b

## Coefficients

D<sub>90</sub>= 3.5978 D<sub>85</sub>= 2.5943 D<sub>60</sub>= 0.6995  
D<sub>50</sub>= 0.4917 D<sub>30</sub>= 0.2944 D<sub>15</sub>= 0.1723  
D<sub>10</sub>= 0.1132 C<sub>u</sub>= 6.18 C<sub>c</sub>= 1.09

## Remarks

As received MC = 17.3%

Date Received: 10/5/2017 Date Tested: 10/17/2017

Tested By: RZ

Checked By: MP

Title: Laboratory Manager

Source of Sample: B-21 Depth: 14-16'  
Sample Number: S-6

Date Sampled: 9/13/2017

**CDM Smith**

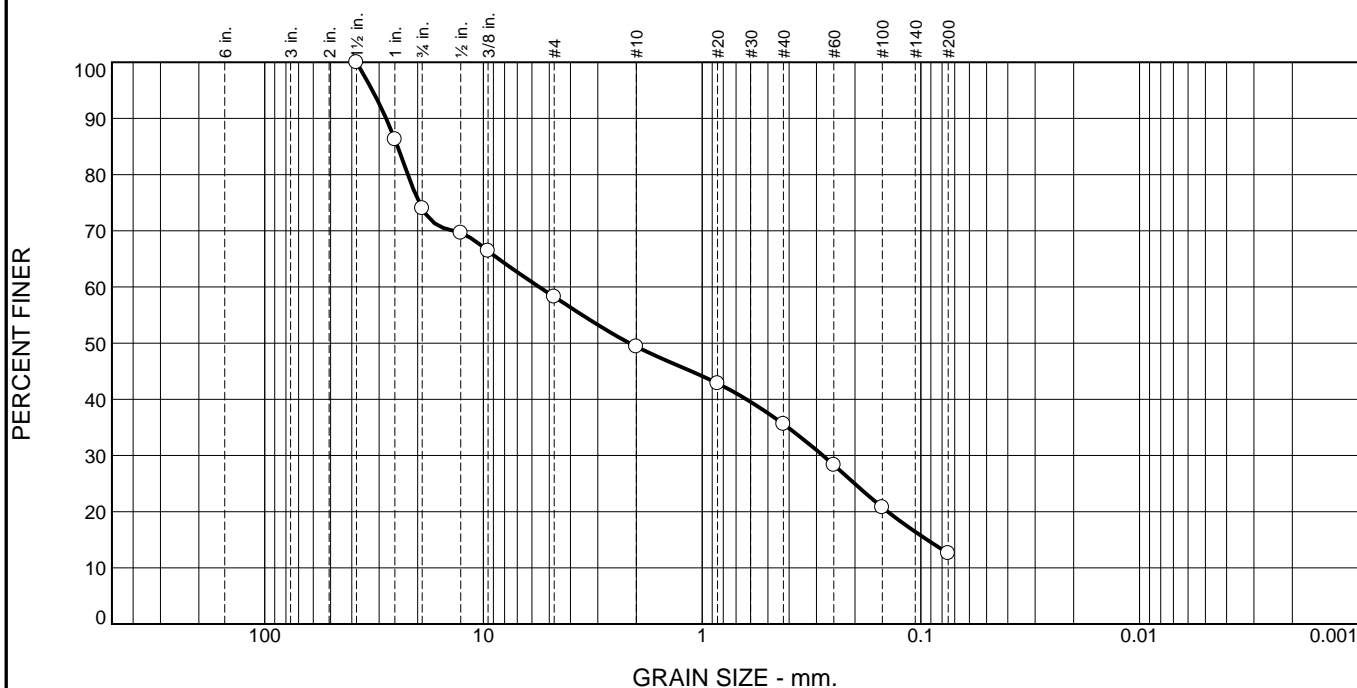
**Boston, Massachusetts**

Client: Town of Salem  
Project: South Broadway Watermain Replacement  
Salem, NH

Project No: 25972-215427

Figure

# Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	26.0	15.7	8.9	13.8	23.0	12.6	

Test Results (ASTM D422 & ASTM D1140)			
Opening Size	Percent Finer	Spec.* (Percent)	Pass? (X=Fail)
1.5"	100.0		
1"	86.3		
.75"	74.0		
.5"	69.6		
.375"	66.4		
#4	58.3		
#10	49.4		
#20	42.8		
#40	35.6		
#60	28.3		
#100	20.8		
#200	12.6		

\* (no specification provided)

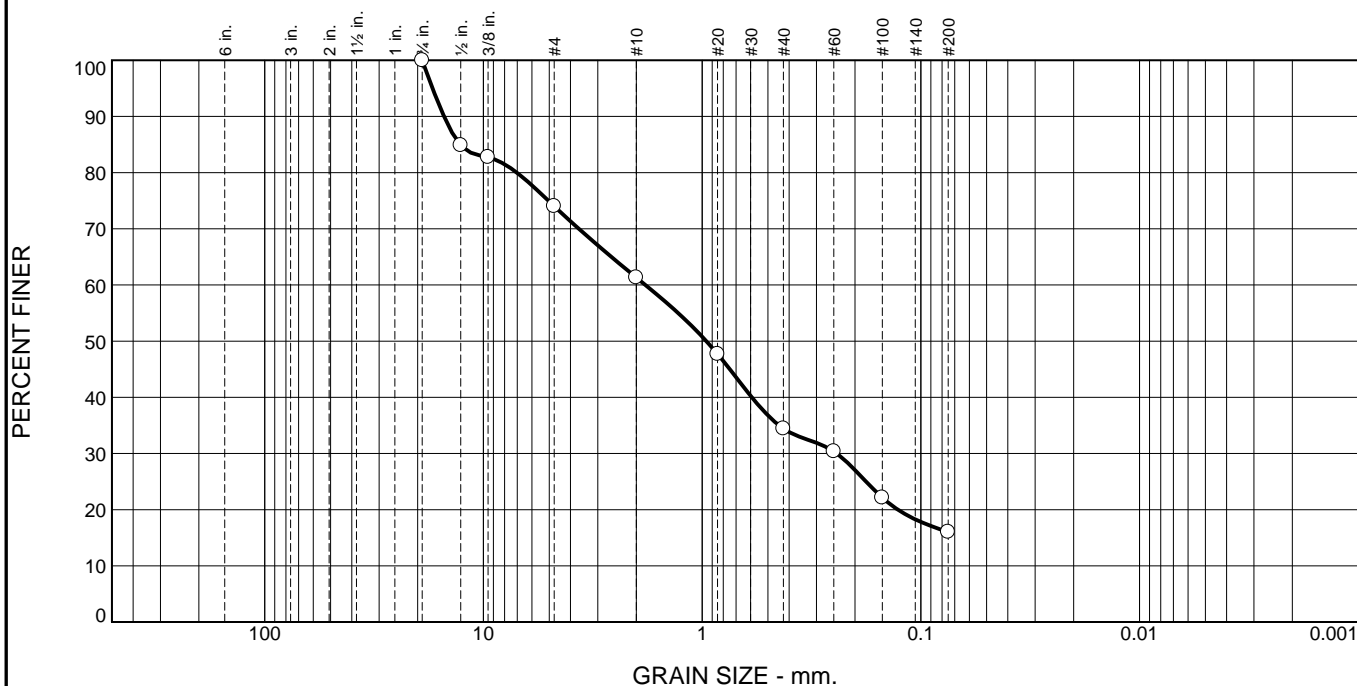
<b>Material Description</b>	
Brown silty sand with gravel	
<b>Atterberg Limits (ASTM D 4318)</b>	
PL=	LL= PI=
<b>Classification</b>	
USCS (D 2487)= SM	AASHTO (M 145)= A-1-b
<b>Coefficients</b>	
D <sub>90</sub> = 27.8495	D <sub>85</sub> = 24.6923 D <sub>60</sub> = 5.5558
D <sub>50</sub> = 2.1456	D <sub>30</sub> = 0.2804 D <sub>15</sub> = 0.0935
D <sub>10</sub> =	C <sub>u</sub> = C <sub>c</sub> =
<b>Remarks</b>	
As received MC = 8.0%	
Date Received: 10/5/2017	Date Tested: 10/17/2017
Tested By: RZ	
Checked By: MP	
Title: Laboratory Manager	

Source of Sample: B-21 Depth: 19-21'  
Sample Number: S-7

Date Sampled: 9/13/2017

<b>CDM Smith</b>  <b>Boston, Massachusetts</b>	<b>Client:</b> Town of Salem <b>Project:</b> South Broadway Watermain Replacement Salem, NH
	<b>Project No:</b> 25972-215427 <b>Figure</b>

# Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	26.0	12.7	26.9	18.4	16.0	

Test Results (ASTM D422 & ASTM D1140)			
Opening Size	Percent Finer	Spec.* (Percent)	Pass? (X=Fail)
.75"	100.0		
.5"	84.9		
.375"	82.8		
#4	74.0		
#10	61.3		
#20	47.7		
#40	34.4		
#60	30.4		
#100	22.1		
#200	16.0		

\* (no specification provided)

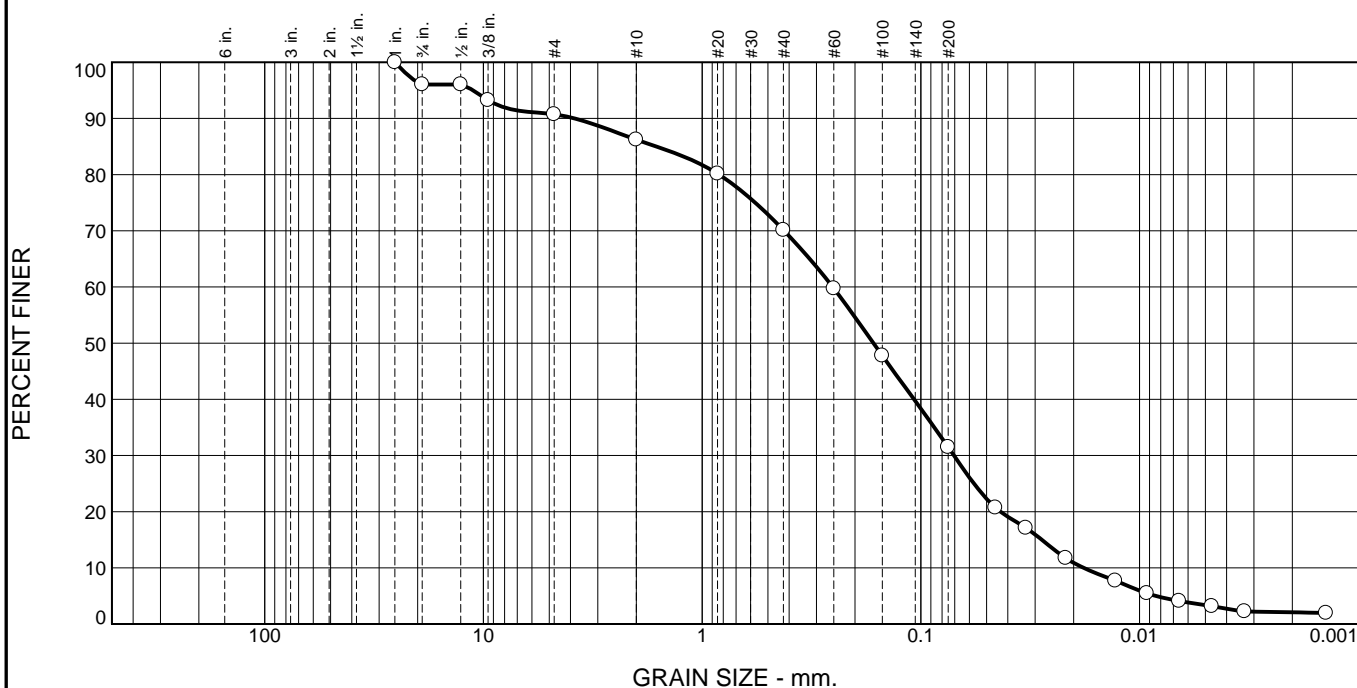
<b>Material Description</b>	
Brown silty sand with gravel	
<b>Atterberg Limits (ASTM D 4318)</b>	
PL=	LL= PI=
<b>Classification</b>	
USCS (D 2487)= SM	AASHTO (M 145)= A-1-b
<b>Coefficients</b>	
D <sub>90</sub> = 15.1470	D <sub>85</sub> = 12.7840 D <sub>60</sub> = 1.8179
D <sub>50</sub> = 0.9565	D <sub>30</sub> = 0.2424 D <sub>15</sub> =
D <sub>10</sub> =	C <sub>u</sub> = C <sub>c</sub> =
<b>Remarks</b>	
As received MC = 11.8%	
<b>Date Received:</b> 10/5/2017 <b>Date Tested:</b> 10/17/2017	
<b>Tested By:</b> RZ	
<b>Checked By:</b> MP	
<b>Title:</b> Laboratory Manager	

Source of Sample: B-22 Depth: 0-1'  
Sample Number: BAG (Subbase)

Date Sampled: 10/4/2017

<b>CDM Smith</b>  <b>Boston, Massachusetts</b>	<b>Client:</b> Town of Salem
	<b>Project:</b> South Broadway Watermain Replacement Salem, NH
<b>Project No:</b> 25972-215427	<b>Figure</b>

# Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	4.0	5.3	4.5	16.1	38.6	28.1	3.4

Test Results (ASTM D422 & ASTM D1140)			
Opening Size	Percent Finer	Spec.* (Percent)	Pass? (X=Fail)
1"	100.0		
.75"	96.0		
.5"	96.0		
.375"	93.2		
#4	90.7		
#10	86.2		
#20	80.2		
#40	70.1		
#60	59.7		
#100	47.8		
#200	31.5		
0.045 mm.	20.7		
0.031 mm.	17.1		
0.021 mm.	11.7		
0.012 mm.	7.7		
0.009 mm.	5.5		
0.006 mm.	4.1		
0.004 mm.	3.2		
0.003 mm.	2.2		
0.001 mm.	2.0		

\* (no specification provided)

**Material Description**  
Light brown silty sand

**Atterberg Limits (ASTM D 4318)**  
PL=                      LL=                      PI=

**Classification**  
USCS (D 2487)= SM                      AASHTO (M 145)= A-2-4(0)

**Coefficients**  
D<sub>90</sub>= 3.8364                      D<sub>85</sub>= 1.6187                      D<sub>60</sub>= 0.2529  
D<sub>50</sub>= 0.1647                      D<sub>30</sub>= 0.0705                      D<sub>15</sub>= 0.0280  
D<sub>10</sub>= 0.0180                      C<sub>u</sub>= 14.03                      C<sub>c</sub>= 1.09

**Remarks**  
As received MC = 16.6%

**Date Received:** 10/5/2017                      **Date Tested:** 10/19/2017  
**Tested By:** GW/MP  
**Checked By:** MP  
**Title:** Laboratory Manager

**Source of Sample:** B-22                      **Depth:** 8-10'  
**Sample Number:** S-5

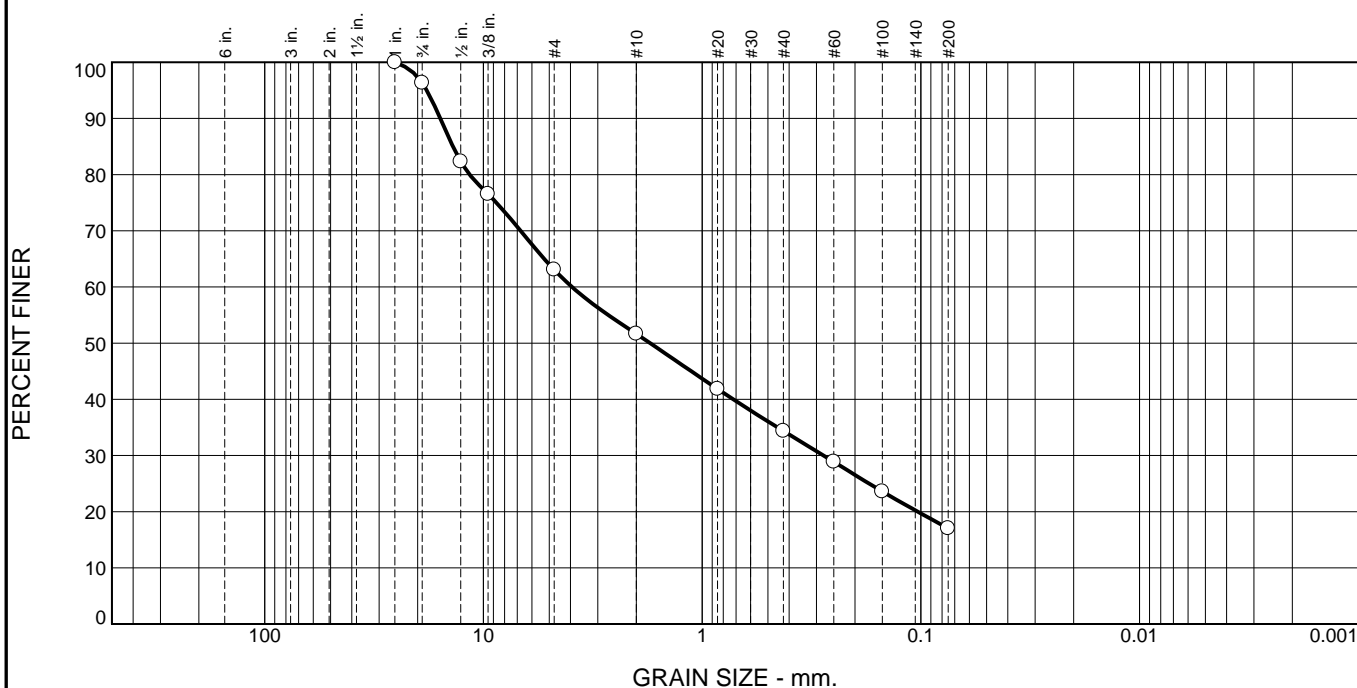
**Date Sampled:** 10/4/2017

**CDM Smith**  
**Boston, Massachusetts**

**Client:** Town of Salem  
**Project:** South Broadway Watermain Replacement  
Salem, NH  
**Project No:** 25972-215427

**Figure**

# Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	3.7	33.2	11.4	17.3	17.4	17.0	

Test Results (ASTM D422 & ASTM D1140)			
Opening Size	Percent Finer	Spec.* (Percent)	Pass? (X=Fail)
1"	100.0		
.75"	96.3		
.5"	82.3		
.375"	76.6		
#4	63.1		
#10	51.7		
#20	41.8		
#40	34.4		
#60	28.9		
#100	23.6		
#200	17.0		

\* (no specification provided)

## Material Description

Brown silty sand with gravel

## Atterberg Limits (ASTM D 4318)

PL= LL= PI=

## Classification

USCS (D 2487)= SM/SC AASHTO (M 145)= A-1-b

## Coefficients

D<sub>90</sub>= 15.7423 D<sub>85</sub>= 13.7887 D<sub>60</sub>= 3.9340  
D<sub>50</sub>= 1.7293 D<sub>30</sub>= 0.2786 D<sub>15</sub>=  
D<sub>10</sub>= C<sub>u</sub>= C<sub>c</sub>=

## Remarks

As received MC = 11.4%

Date Received: 10/5/2017 Date Tested: 10/17/2017

Tested By: RZ

Checked By: MP

Title: Laboratory Manager

Source of Sample: B-22  
Sample Number: S-6

Depth: 14-16'

Date Sampled: 10/4/2017

**CDM Smith**

**Boston, Massachusetts**

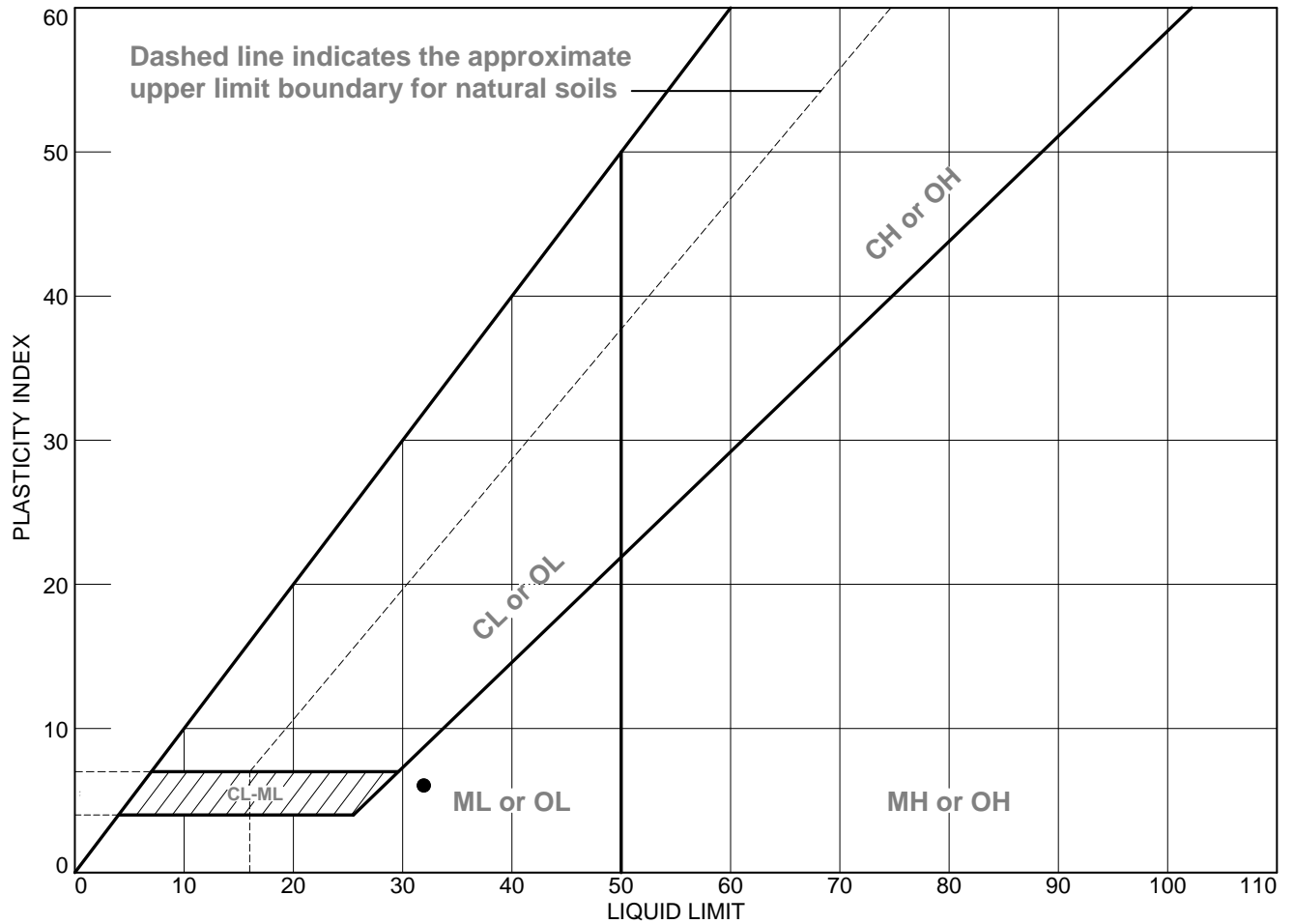
Client: Town of Salem

Project: South Broadway Watermain Replacement  
Salem, NH

Project No: 25972-215427

Figure

# LIQUID AND PLASTIC LIMITS TEST REPORT



SOIL DATA								
SYMBOL	SOURCE	SAMPLE NO.	DEPTH	NATURAL WATER CONTENT (%)	PLASTIC LIMIT (%)	LIQUID LIMIT (%)	PLASTICITY INDEX (%)	USCS
●	B-23	S-5	8-10'	32.5	26	32	6	ML

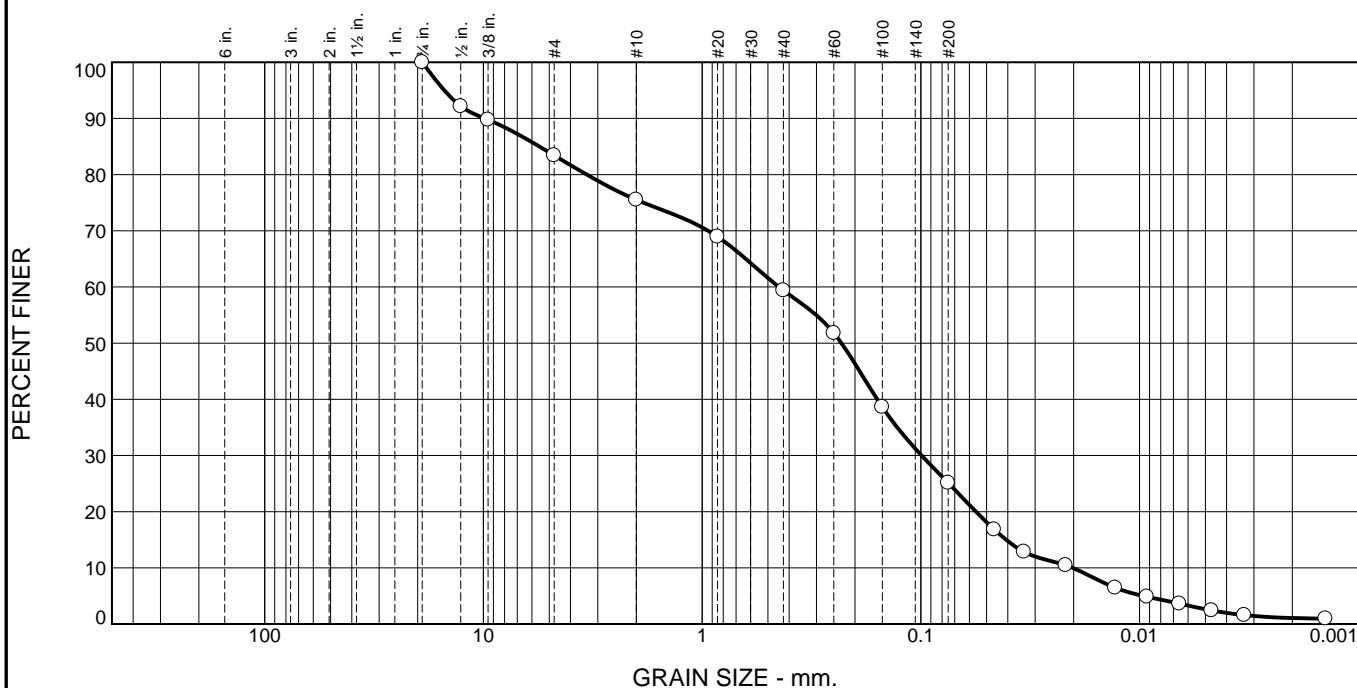
**CDM Smith**  
**Boston, Massachusetts**

**Client:** Town of Salem  
**Project:** South Broadway Watermain Replacement  
Salem, NH  
**Project No.:** 25972-215427

**Figure**

**Tested By:** RZ **Checked By:** MP

# Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	16.6	7.9	16.1	34.3	22.5	2.6

Test Results (ASTM D422 & ASTM D1140)			
Opening Size	Percent Finer	Spec.* (Percent)	Pass? (X=Fail)
.75"	100.0		
.5"	92.2		
.375"	89.8		
#4	83.4		
#10	75.5		
#20	68.9		
#40	59.4		
#60	51.8		
#100	38.6		
#200	25.1		
0.0462 mm.	16.8		
0.0337 mm.	12.9		
0.0217 mm.	10.5		
0.0129 mm.	6.5		
0.0092 mm.	4.9		
0.0066 mm.	3.6		
0.0047 mm.	2.4		
0.0033 mm.	1.6		
0.0014 mm.	1.0		

\* (no specification provided)

**Material Description**  
Brown silty sand with gravel

**Atterberg Limits (ASTM D 4318)**  
PL=                      LL=                      PI=

**Classification**  
USCS (D 2487)= SM                      AASHTO (M 145)= A-2-4(0)

**Coefficients**  
D<sub>90</sub>= 9.8642                      D<sub>85</sub>= 5.5485                      D<sub>60</sub>= 0.4454  
D<sub>50</sub>= 0.2307                      D<sub>30</sub>= 0.0992                      D<sub>15</sub>= 0.0408  
D<sub>10</sub>= 0.0202                      C<sub>u</sub>= 22.00                      C<sub>c</sub>= 1.09

**Remarks**  
As received MC = 13.7%

**Date Received:** 10/5/2017                      **Date Tested:** 10/19/2017  
**Tested By:** MP/GW  
**Checked By:** MP  
**Title:** Laboratory Manager

**Source of Sample:** B-23                      **Depth:** 14-16'  
**Sample Number:** S-6

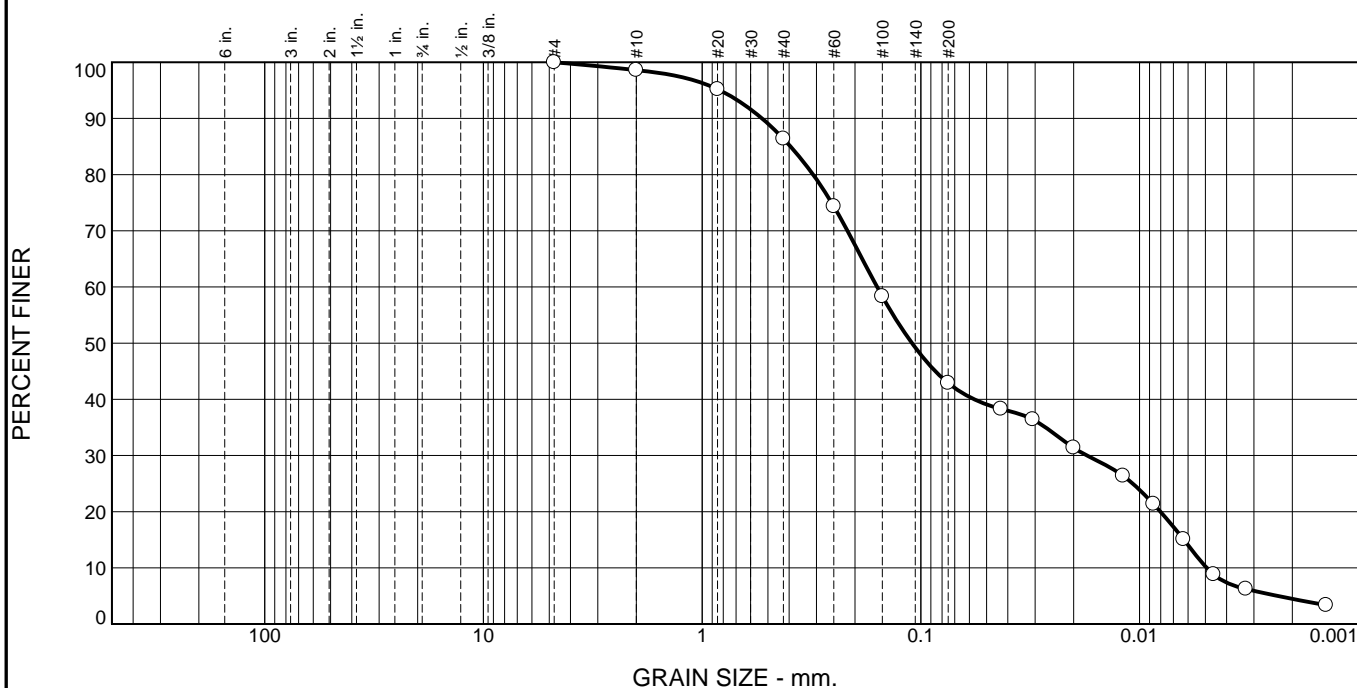
**Date Sampled:** 9/12/2017

**CDM Smith**  
**Boston, Massachusetts**

**Client:** Town of Salem  
**Project:** South Broadway Watermain Replacement  
Salem, NH  
**Project No:** 25972-215427

**Figure**

# Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	0.0	1.4	12.2	43.5	32.6	10.3

Test Results (ASTM D422 & ASTM D1140)			
Opening Size	Percent Finer	Spec.* (Percent)	Pass? (X=Fail)
#4	100.0		
#10	98.6		
#20	95.2		
#40	86.4		
#60	74.3		
#100	58.3		
#200	42.9		
0.0430 mm.	38.3		
0.0308 mm.	36.4		
0.0200 mm.	31.4		
0.0119 mm.	26.4		
0.0086 mm.	21.4		
0.0063 mm.	15.1		
0.0046 mm.	8.8		
0.0033 mm.	6.3		
0.0014 mm.	3.4		

\* (no specification provided)

<b>Material Description</b>	
Brown silty sand	
<b>Atterberg Limits (ASTM D 4318)</b>	
PL=	LL= PI=
<b>Classification</b>	
USCS (D 2487)= SM	AASHTO (M 145)= A-4(0)
<b>Coefficients</b>	
D <sub>90</sub> = 0.5324	D <sub>85</sub> = 0.3940 D <sub>60</sub> = 0.1584
D <sub>50</sub> = 0.1098	D <sub>30</sub> = 0.0174 D <sub>15</sub> = 0.0063
D <sub>10</sub> = 0.0049	C <sub>u</sub> = 32.16 C <sub>c</sub> = 0.39
<b>Remarks</b>	
As received MC = 22.3%	
<b>Date Received:</b> 10/5/2017	<b>Date Tested:</b> 10/19/2017
<b>Tested By:</b> MP/GW	
<b>Checked By:</b> MP	
<b>Title:</b> Laboratory Manager	

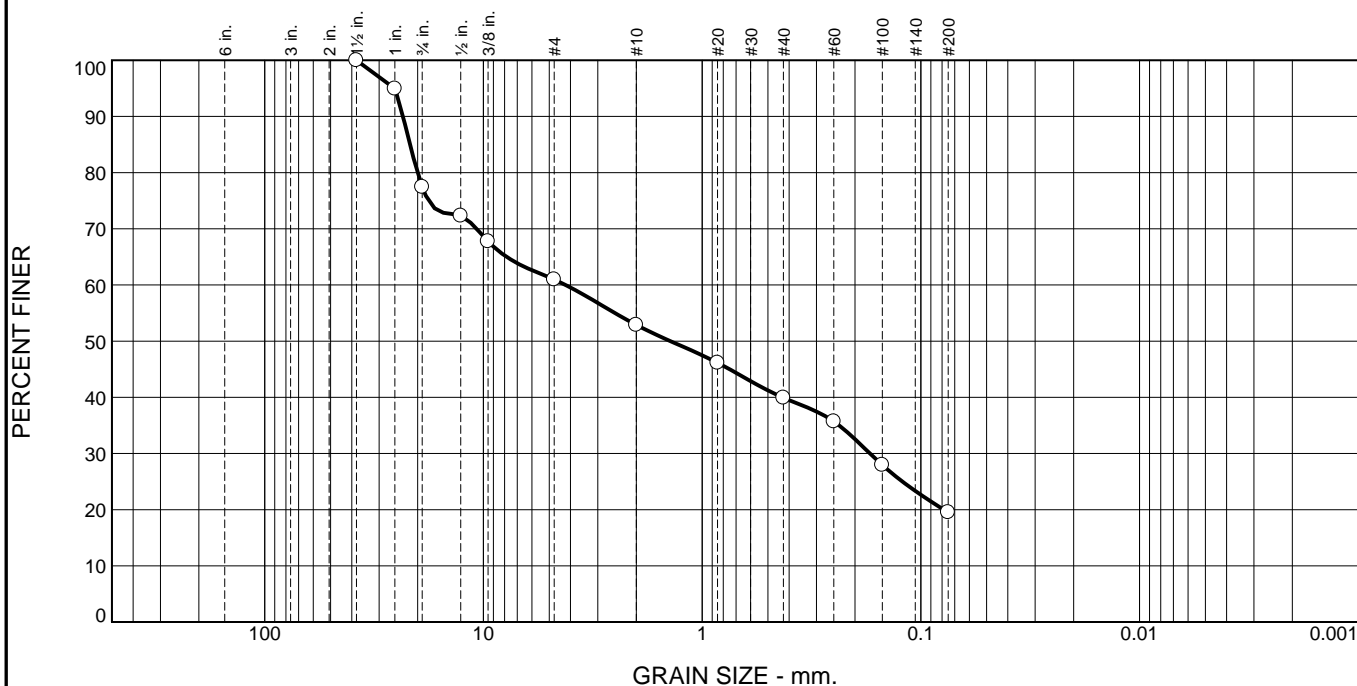
Source of Sample: B-24 Depth: 8-10'  
Sample Number: S-5

Date Sampled: 9/12/2017

<b>CDM Smith</b>  <b>Boston, Massachusetts</b>	<b>Client:</b> Town of Salem <b>Project:</b> South Broadway Watermain Replacement Salem, NH
	<b>Project No:</b> 25972-215427 <b>Figure</b>



# Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	22.6	16.4	8.1	13.0	20.4	19.5	

Test Results (ASTM D 422 & ASTM D 1140)			
Opening Size	Percent Finer	Spec.* (Percent)	Pass? (X=Fail)
1.5"	100.0		
1"	94.9		
3/4"	77.4		
1/2"	72.3		
3/8"	67.8		
#4	61.0		
#10	52.9		
#20	46.1		
#40	39.9		
#60	35.7		
#100	27.9		
#200	19.5		

\* (no specification provided)

**Material Description**  
Gray silty sand with gravel

**Atterberg Limits (ASTM D 4318)**  
PL=                      LL=                      PI=

**Classification**  
USCS (D 2487)= SM                      AASHTO (M 145)= A-1-b

**Coefficients**  
D<sub>90</sub>= 23.3370                      D<sub>85</sub>= 21.6582                      D<sub>60</sub>= 4.2093  
D<sub>50</sub>= 1.3954                      D<sub>30</sub>= 0.1708                      D<sub>15</sub>=  
D<sub>10</sub>=                      C<sub>u</sub>=                      C<sub>c</sub>=

**Remarks**  
As received MC = 8.7%

**Date Received:** 10/4/2017                      **Date Tested:** 10/17/2017  
**Tested By:** RZ  
**Checked By:** MP  
**Title:** Laboratory Manager

**Source of Sample:** B-24                      **Depth:** 14-16'  
**Sample Number:** S-6

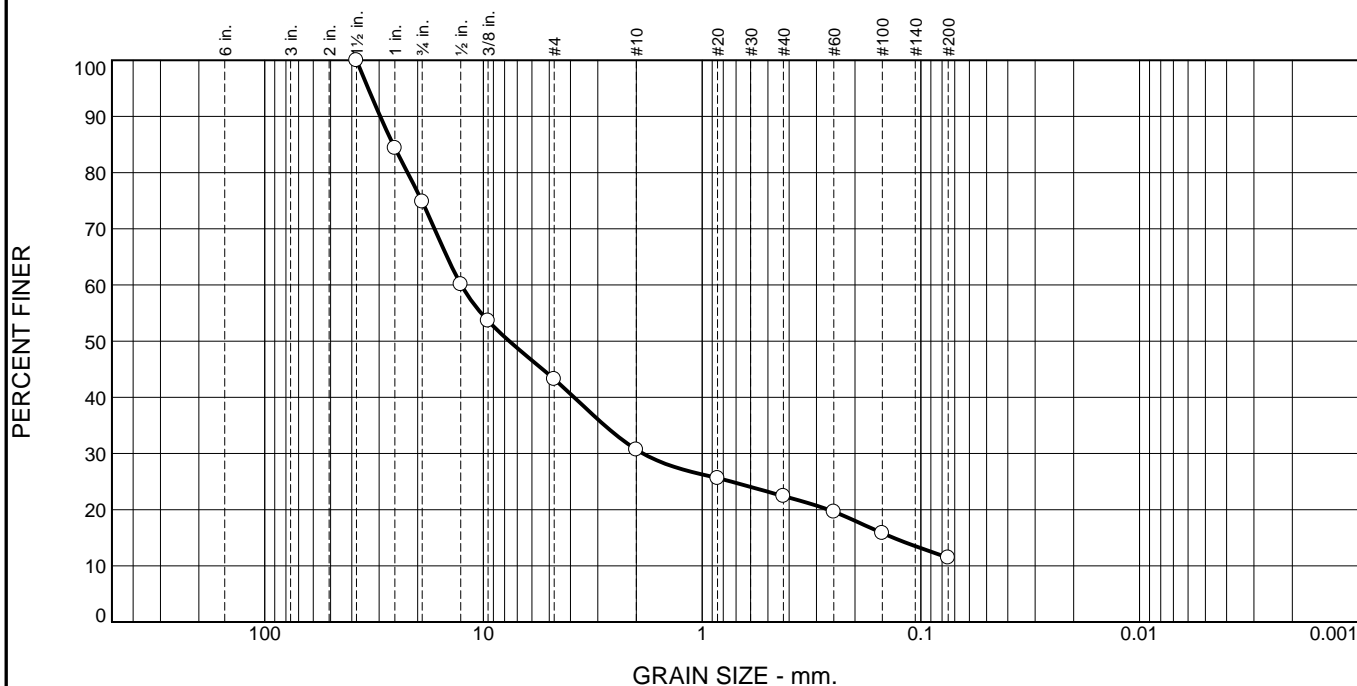
**Date Sampled:** 9/12/2017

**CDM Smith**  
**Boston, Massachusetts**

**Client:** Town of Salem  
**Project:** South Broadway Watermain Replacement  
Salem, NH  
**Project No:** 25972-215427

**Figure**

# Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	25.2	31.6	12.5	8.3	10.9	11.5	

Test Results (ASTM D 422 & ASTM D 1140)			
Opening Size	Percent Finer	Spec.* (Percent)	Pass? (X=Fail)
1.5"	100.0		
1"	84.4		
3/4"	74.8		
1/2"	60.1		
3/8"	53.6		
#4	43.2		
#10	30.7		
#20	25.6		
#40	22.4		
#60	19.6		
#100	15.8		
#200	11.5		

\* (no specification provided)

**Material Description**  
Brown poorly graded gravel with silt and sand

**Atterberg Limits (ASTM D 4318)**  
 PL=                      LL=                      PI=

**Classification**  
 USCS (D 2487)= GP-GM    AASHTO (M 145)= A-1-a

**Coefficients**  
 D<sub>90</sub>= 29.6476    D<sub>85</sub>= 25.8668    D<sub>60</sub>= 12.6451  
 D<sub>50</sub>= 7.6160    D<sub>30</sub>= 1.8652    D<sub>15</sub>= 0.1335  
 D<sub>10</sub>=              C<sub>u</sub>=              C<sub>c</sub>=

**Remarks**  
As received MC = 7.0%

**Date Received:** 10/4/2017    **Date Tested:** 10/17/2017  
**Tested By:** RZ  
**Checked By:** MP  
**Title:** Laboratory Manager

**Source of Sample:** B-24    **Depth:** 19-21'  
**Sample Number:** S-7

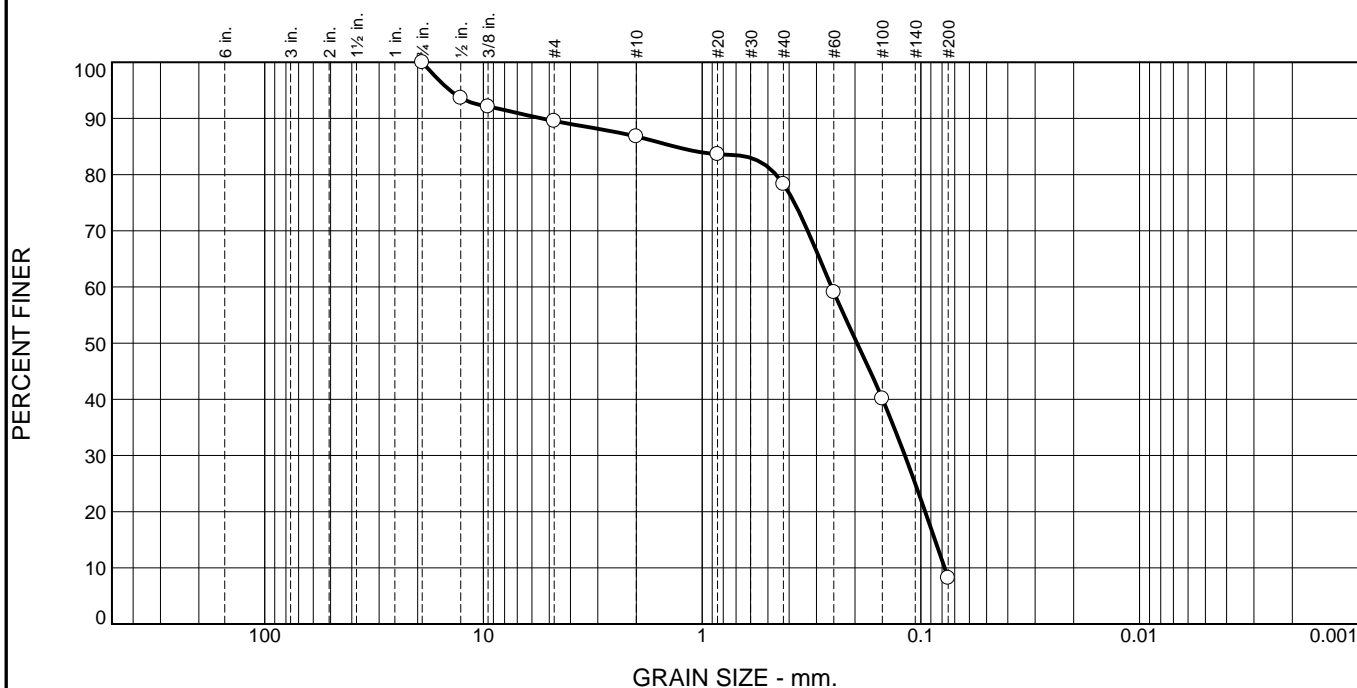
**Date Sampled:** 9/12/2017

**CDM Smith**  
  
**Boston, Massachusetts**

**Client:** Town of Salem  
**Project:** South Broadway Watermain Replacement  
 Salem, NH  
**Project No:** 25972-215427

**Figure**

# Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	10.5	2.7	8.5	70.1	8.2	

Test Results (ASTM D 422 & ASTM D 1140)			
Opening Size	Percent Finer	Spec.* (Percent)	Pass? (X=Fail)
3/4"	100.0		
1/2"	93.7		
3/8"	92.1		
#4	89.5		
#10	86.8		
#20	83.6		
#40	78.3		
#60	59.1		
#100	40.1		
#200	8.2		

\* (no specification provided)

**Material Description**  
Black and reddish brown poorly graded sand with silt

**Atterberg Limits (ASTM D 4318)**  
 PL=                      LL=                      PI=

**Classification**  
 USCS (D 2487)= SP-SM      AASHTO (M 145)= A-3

**Coefficients**  
 D<sub>90</sub>= 5.4079      D<sub>85</sub>= 1.3352      D<sub>60</sub>= 0.2560  
 D<sub>50</sub>= 0.1956      D<sub>30</sub>= 0.1183      D<sub>15</sub>= 0.0861  
 D<sub>10</sub>= 0.0778      C<sub>u</sub>= 3.29      C<sub>c</sub>= 0.70

**Remarks**  
As received MC = 17.6%

**Date Received:** 10/4/2017      **Date Tested:** 10/17/2017  
**Tested By:** RZ  
**Checked By:** MP  
**Title:** Laboratory Manager

**Source of Sample:** B-25      **Depth:** 4-6'  
**Sample Number:** S-3

**Date Sampled:** 9/11/2017

**CDM Smith**  
  
**Boston, Massachusetts**

**Client:** Town of Salem  
**Project:** South Broadway Watermain Replacement  
 Salem, NH  
**Project No:** 25972-215427

**Figure**

# CDM Smith

## Geotechnical Engineering Laboratory

### Standard Test Method for Moisture, Ash, and Organic Matter of Peat and Other Organic Soils (ASTM D2974)

Client: Town of Salem  
Project Name: South Broadway WM Replacement  
Project Location: Salem, NH  
Project Number: 25972-215427  
Boring Number: B-25  
Sample Number: S-3  
Sample Depth (ft): 4-6'  
Sample Date: 9/11/2017

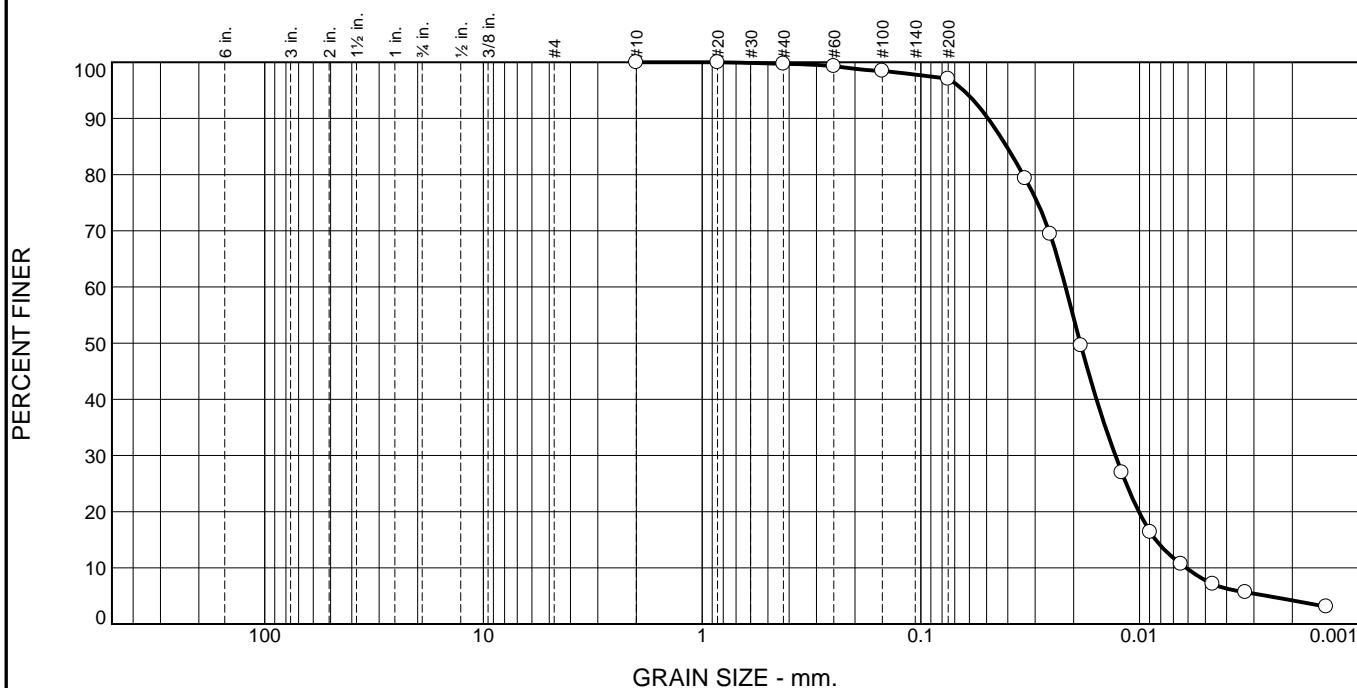
Tested By: MP  
Test Date: 10/17/2017

Procedure: C  
Temperature: 440° C

AS RECEIVED MOISTURE CONTENT	
Tin Mass (g)	90.27
Wet Mass of Sample & Tin (g)	164.43
Dry Mass of Sample & Tin (g)	153.35
Mass of Water (g)	11.08
Mass of Dry Soil (g)	63.08
<b>Moisture Content (%)</b>	<b>17.6</b>

ASH CONTENT	
Porcelain Dish Mass (g)	90.27
Porcelain Dish + Oven Dried Soil (g)	153.35
Mass of Oven Dried Soil (g)	63.08
Mass of Dish & Burned Soil (g)	151.86
Mass of Burned Soil (g)	61.59
Mass of Organic Material (g)	1.49
Ash Content (%)	97.6
<b>Organic Content (%)</b>	<b>2.4</b>

# Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	0.0	0.0	0.3	2.6	89.3	7.8

Test Results (ASTM D422 & ASTM D1140)			
Opening Size	Percent Finer	Spec.* (Percent)	Pass? (X=Fail)
#10	100.0		
#20	100.0		
#40	99.7		
#60	99.3		
#100	98.5		
#200	97.1		
0.0334 mm.	79.3		
0.0256 mm.	69.4		
0.0185 mm.	49.6		
0.0121 mm.	27.0		
0.0089 mm.	16.4		
0.0065 mm.	10.7		
0.0046 mm.	7.1		
0.0033 mm.	5.7		
0.0014 mm.	3.1		

\* (no specification provided)

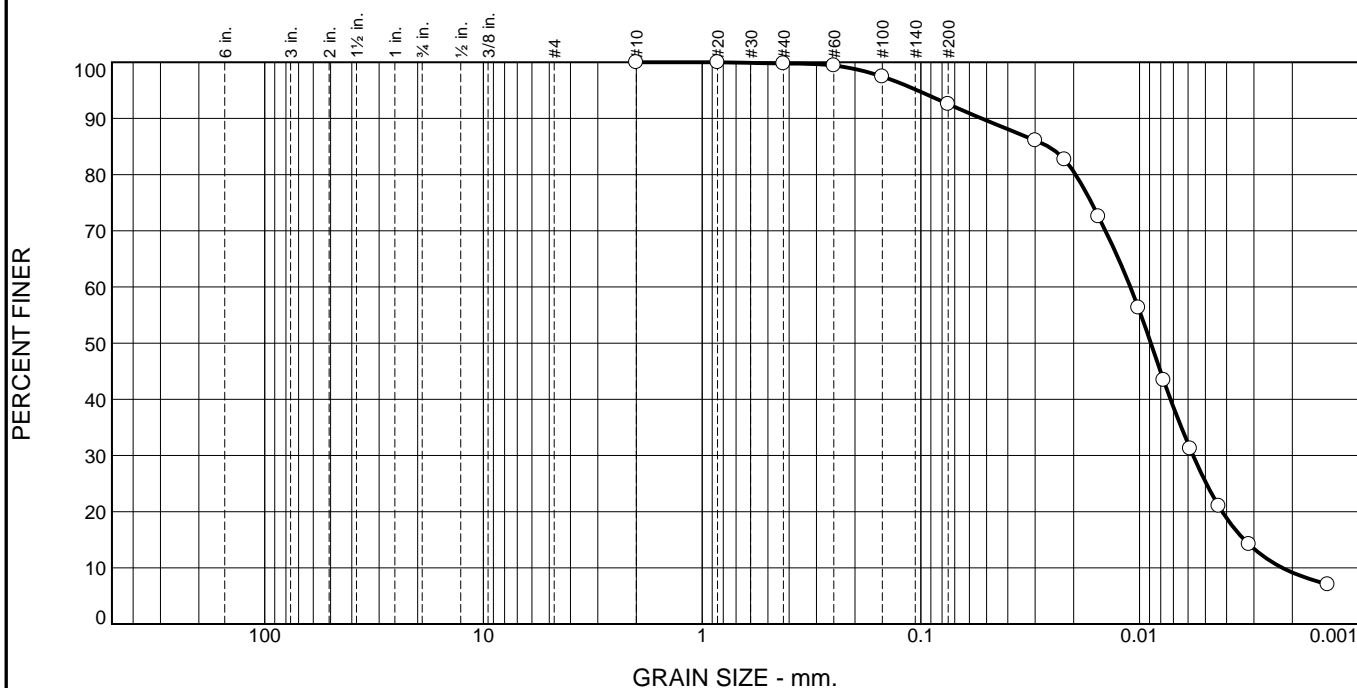
<b>Material Description</b>	
Gray silt	
<b>Atterberg Limits (ASTM D 4318)</b>	
PL=	LL= PI=
<b>Classification</b>	
USCS (D 2487)= ML	AASHTO (M 145)= A-4(0)
<b>Coefficients</b>	
D <sub>90</sub> = 0.0493	D <sub>85</sub> = 0.0405 D <sub>60</sub> = 0.0218
D <sub>50</sub> = 0.0186	D <sub>30</sub> = 0.0129 D <sub>15</sub> = 0.0084
D <sub>10</sub> = 0.0061	C <sub>u</sub> = 3.56 C <sub>c</sub> = 1.25
<b>Remarks</b>	
As received MC = 28.8%	
<b>Date Received:</b> 10/5/2017	<b>Date Tested:</b> 10/19/2017
<b>Tested By:</b> GW/MP	
<b>Checked By:</b> MP	
<b>Title:</b> Laboratory Manager	

Source of Sample: B-25 Depth: 14-16'  
Sample Number: S-6

Date Sampled: 9/11/2017

<b>CDM Smith</b>  <b>Boston, Massachusetts</b>	<b>Client:</b> Town of Salem
	<b>Project:</b> South Broadway Watermain Replacement Salem, NH
<b>Project No:</b> 25972-215427	<b>Figure</b>

# Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	0.0	0.0	0.2	7.2	67.3	25.3

Test Results (ASTM D422 & ASTM D1140)			
Opening Size	Percent Finer	Spec.* (Percent)	Pass? (X=Fail)
#10	100.0		
#20	100.0		
#40	99.8		
#60	99.4		
#100	97.5		
#200	92.6		
0.0300 mm.	86.1		
0.0220 mm.	82.7		
0.0154 mm.	72.5		
0.0101 mm.	56.3		
0.0078 mm.	43.4		
0.0059 mm.	31.2		
0.0043 mm.	21.0		
0.0032 mm.	14.2		
0.0014 mm.	7.0		

\* (no specification provided)

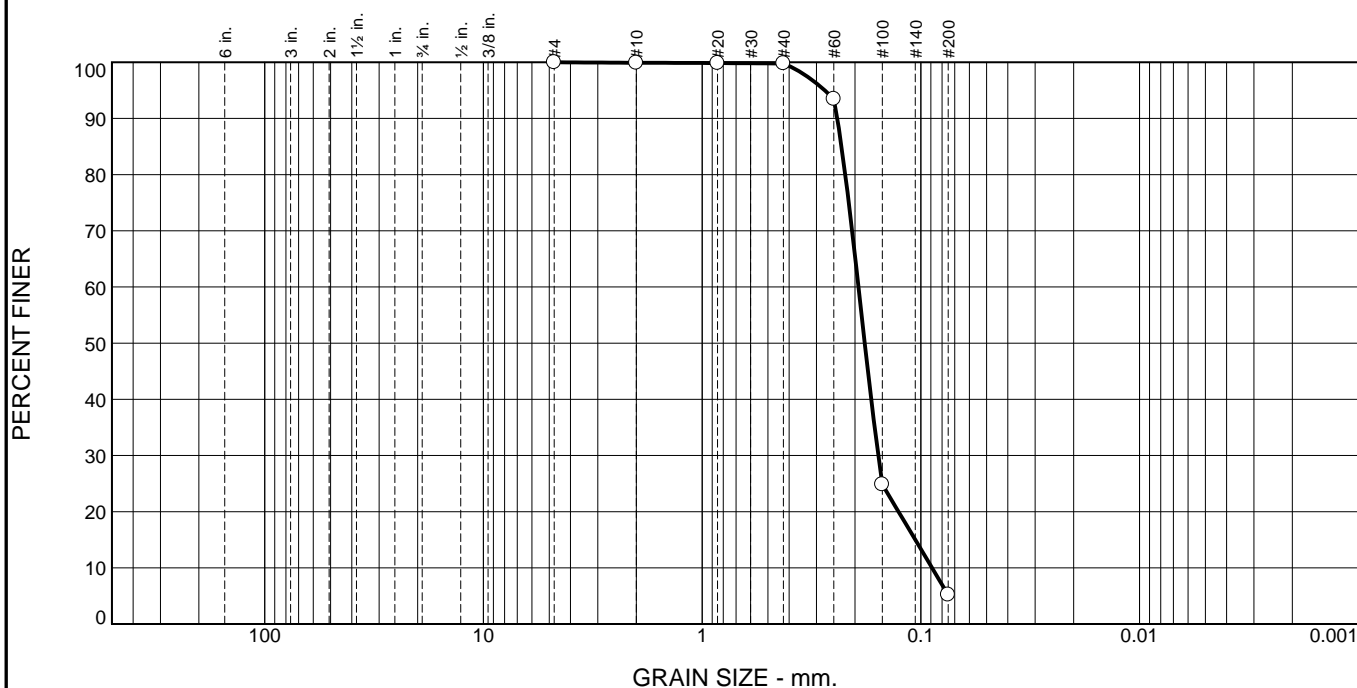
<b>Material Description</b>	
Gray silt	
<b>Atterberg Limits (ASTM D 4318)</b>	
PL=	LL= PI=
<b>Classification</b>	
USCS (D 2487)= ML	AASHTO (M 145)= A-4(0)
<b>Coefficients</b>	
D <sub>90</sub> = 0.0527	D <sub>85</sub> = 0.0261 D <sub>60</sub> = 0.0110
D <sub>50</sub> = 0.0089	D <sub>30</sub> = 0.0057 D <sub>15</sub> = 0.0033
D <sub>10</sub> = 0.0022	C <sub>u</sub> = 4.95 C <sub>c</sub> = 1.32
<b>Remarks</b>	
As received MC = 29.1%	
Date Received: 10/5/2017	Date Tested: 10/19/2017
Tested By: GW/MP	
Checked By: MP	
Title: Laboratory Manager	

Source of Sample: B-25 Depth: 19-21'  
Sample Number: S-7

Date Sampled: 9/11/2017

<b>CDM Smith</b>  <b>Boston, Massachusetts</b>	<b>Client:</b> Town of Salem <b>Project:</b> South Broadway Watermain Replacement Salem, NH
	<b>Project No:</b> 25972-215427 <b>Figure</b>

# Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	0.0	0.1	0.1	94.6	5.2	

Test Results (ASTM D 422 & ASTM D 1140)			
Opening Size	Percent Finer	Spec.* (Percent)	Pass? (X=Fail)
#4	100.0		
#10	99.9		
#20	99.9		
#40	99.8		
#60	93.5		
#100	24.8		
#200	5.2		

\* (no specification provided)

## Material Description

Light brown poorly graded sand with silt

## Atterberg Limits (ASTM D 4318)

PL= LL= PI=

## Classification

USCS (D 2487)= SP-SM AASHTO (M 145)= A-3

## Coefficients

D<sub>90</sub>= 0.2410 D<sub>85</sub>= 0.2304 D<sub>60</sub>= 0.1927  
D<sub>50</sub>= 0.1804 D<sub>30</sub>= 0.1566 D<sub>15</sub>= 0.1060  
D<sub>10</sub>= 0.0888 C<sub>u</sub>= 2.17 C<sub>c</sub>= 1.43

## Remarks

As received MC = 24.8%

Date Received: 10/4/2017 Date Tested: 10/17/2017

Tested By: RZ

Checked By: MP

Title: Laboratory Manager

Source of Sample: B-26 Depth: 8-10'  
Sample Number: S-5

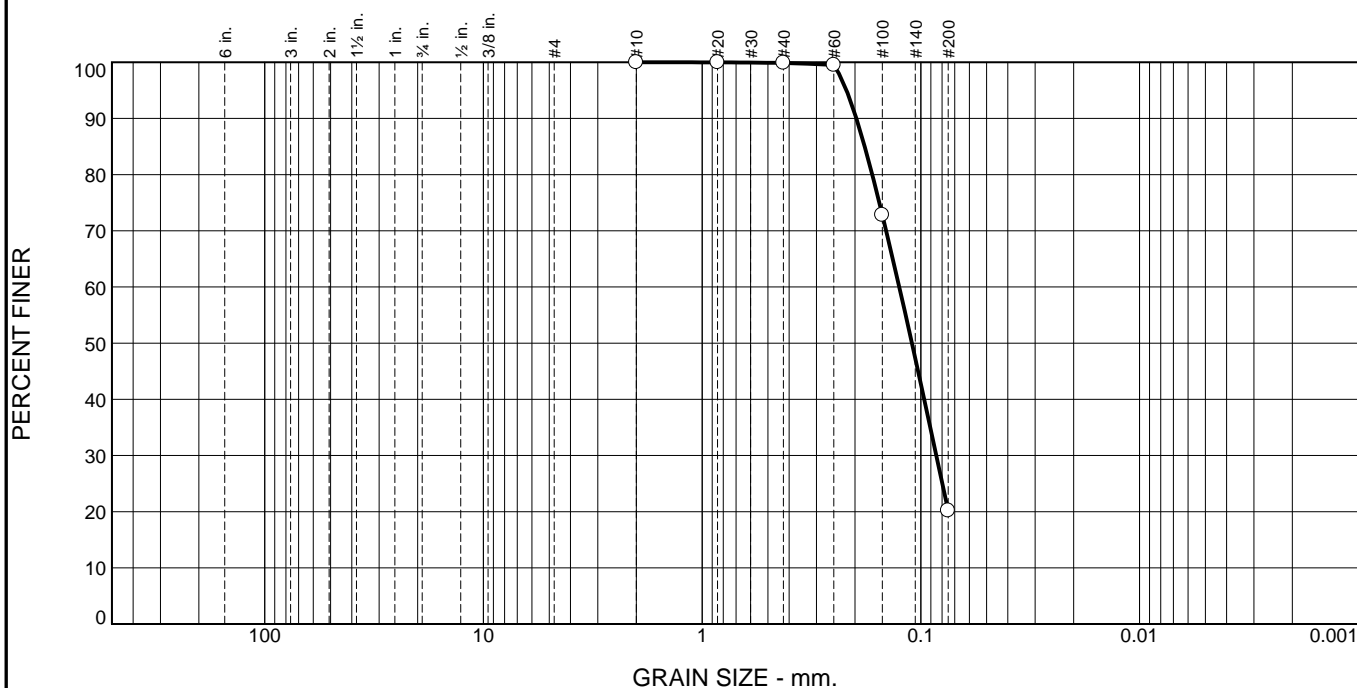
Date Sampled: 9/11/2017

**CDM Smith**  
**Boston, Massachusetts**

Client: Town of Salem  
Project: South Broadway Watermain Replacement  
Salem, NH  
Project No: 25972-215427

Figure

# Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	0.0	0.0	0.1	79.7	20.2	

Test Results (ASTM D 422 & ASTM D 1140)			
Opening Size	Percent Finer	Spec.* (Percent)	Pass? (X=Fail)
#10	100.0		
#20	100.0		
#40	99.9		
#60	99.5		
#100	72.8		
#200	20.2		

\* (no specification provided)

<b>Material Description</b>	
Brown silty sand	
<b>Atterberg Limits (ASTM D 4318)</b>	
PL=	LL= PI=
<b>Classification</b>	
USCS (D 2487)= SM	AASHTO (M 145)= A-2-4(0)
<b>Coefficients</b>	
D <sub>90</sub> = 0.1970	D <sub>85</sub> = 0.1805 D <sub>60</sub> = 0.1255
D <sub>50</sub> = 0.1099	D <sub>30</sub> = 0.0849 D <sub>15</sub> =
D <sub>10</sub> =	C <sub>u</sub> = C <sub>c</sub> =
<b>Remarks</b>	
As received MC = 27.5%	
<b>Date Received:</b> 10/4/2017 <b>Date Tested:</b> 10/17/2017	
<b>Tested By:</b> RZ	
<b>Checked By:</b> MP	
<b>Title:</b> Laboratory Manager	

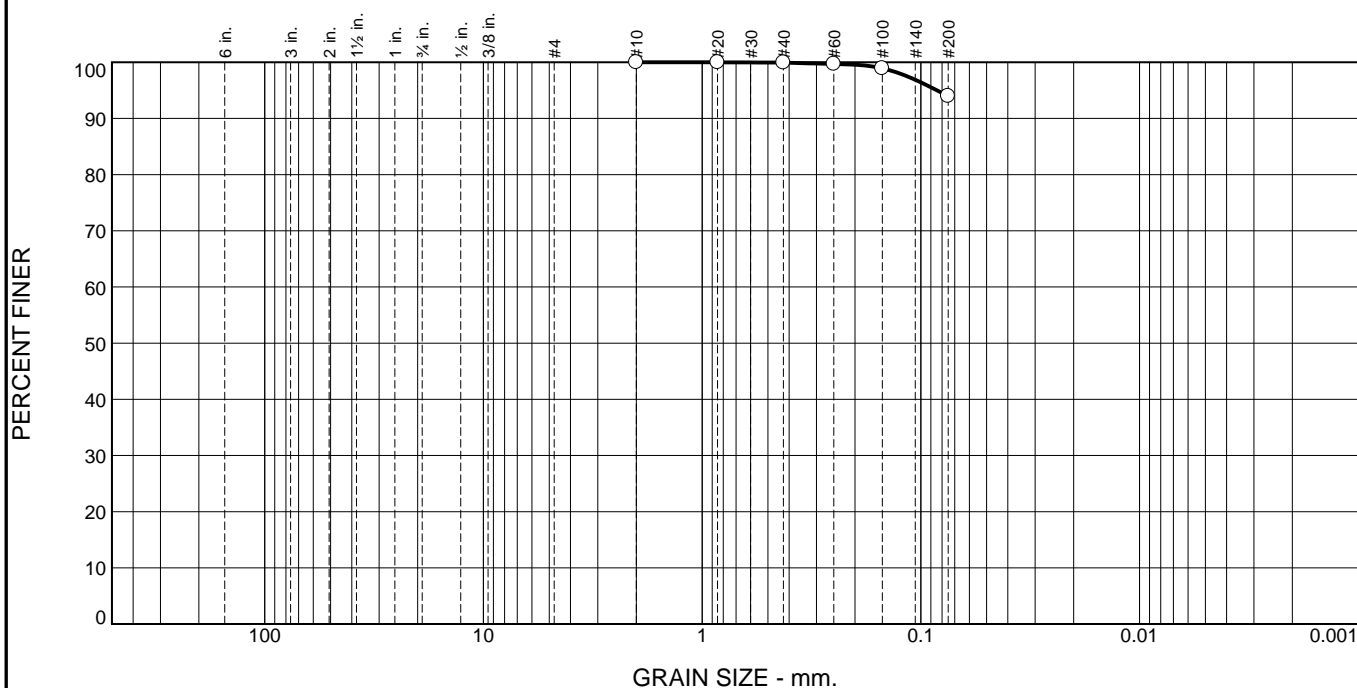
Source of Sample: B-26 Depth: 14-16'  
Sample Number: S-6

Date Sampled: 9/11/2017

<b>CDM Smith</b>  <b>Boston, Massachusetts</b>	<b>Client:</b> Town of Salem <b>Project:</b> South Broadway Watermain Replacement Salem, NH
	<b>Project No:</b> 25972-215427 <b>Figure</b>



# Particle Size Distribution Report



% +3"	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
0.0	0.0	0.0	0.0	0.1	5.9	94.0	

Test Results (ASTM D 422 & ASTM D 1140)			
Opening Size	Percent Finer	Spec.* (Percent)	Pass? (X=Fail)
#10	100.0		
#20	100.0		
#40	99.9		
#60	99.7		
#100	98.9		
#200	94.0		

\* (no specification provided)

<b>Material Description</b>		
Brown and gray silt		
<b>Atterberg Limits (ASTM D 4318)</b>		
PL=	LL=	PI=
<b>Classification</b>		
USCS (D 2487)= ML	AASHTO (M 145)=	A-4(0)
<b>Coefficients</b>		
D <sub>90</sub> =	D <sub>85</sub> =	D <sub>60</sub> =
D <sub>50</sub> =	D <sub>30</sub> =	D <sub>15</sub> =
D <sub>10</sub> =	C <sub>u</sub> =	C <sub>c</sub> =
<b>Remarks</b>		
As received MC = 26.5%		
<b>Date Received:</b> 10/4/2017		<b>Date Tested:</b> 10/17/2017
<b>Tested By:</b> RZ		
<b>Checked By:</b> MP		
<b>Title:</b> Laboratory Manager		

Source of Sample: B-26 Depth: 19-21'  
Sample Number: S-7

Date Sampled: 9/11/2017

<b>CDM Smith</b>  <b>Boston, Massachusetts</b>	<b>Client:</b> Town of Salem <b>Project:</b> South Broadway Watermain Replacement Salem, NH
	<b>Project No:</b> 25972-215427 <b>Figure</b>

## **APPENDIX G**

### **ENVIRONMENTAL TESTING RESULTS AND MONITORING WELL WATER LEVELS**

## TEST BORING SOIL ANALYTICAL RESULTS

TOWN OF SALEM, NEW HAMPSHIRE  
SOUTH BROADWAY WATER & SEWER REHABILITATION

LOCATION						B-2_S-1_0'-10'	B-3_S-1_0'-10'		B-4_S-1_0'-10'		B-11_S-1_0'-10'		B-15_S-1_4'-5'		
SAMPLING DATE						4/11/2017	4/11/2017		4/10/2017		4/12/2017		4/13/2017		
LAB SAMPLE ID						L1711282-02, L1711580-01	L1711282-01		L1711074-01 L1711576-01		L1711530-01		L1711656-01		
	CasNum	Units	COMM-97 <sup>2</sup>		Table 600-2 Soil Remediation Standards <sup>1</sup>	Results		Results		Results		Results		Results	
			Unlined Landfill	Lined Landfill											
General Chemistry															
Solids, Total	NONE	%	NE	NE	NE	73.7		74.8		81.8		86.6		72.6	
Total Metals															
Arsenic, Total	7440-38-2	mg/kg	40	40	11	9		7.1		3.9		2.2		2.7	
Barium, Total	7440-39-3	mg/kg	NE	NE	1000	63		38		23		13		8.7	
Cadmium, Total	7440-43-9	mg/kg	NE	NE	33	0.57		0.53 U		0.48 U		0.45 U		0.54 U	
Chromium, Total	7440-47-3	mg/kg	1000	1000	NE	26		21		18		8.7		6.8	
Lead, Total	7439-92-1	mg/kg	1000	2000	400	240		50		120		21		9.7	
Mercury, Total	7439-97-6	mg/kg	10	10	7	0.19		0.1		0.08 U		0.07 U		0.09 U	
Selenium, Total	7782-49-2	mg/kg	NE	NE	180	1 U		1.1 U		0.96 U		0.89 U		1.1 U	
Silver, Total	7440-22-4	mg/kg	NE	NE	89	0.53 U		0.53 U		0.48 U		0.45 U		0.54 U	
TCLP Metals by EPA 1311															
Lead, TCLP	7439-92-1	mg/l	5	5	NE	0.5 U		-		0.5 U		-		-	
PCB by GC															
Aroclor 1016	12674-11-2	mg/kg	NE	NE	NE	0.0448 U		0.0428 U		0.0385 U		0.0376 U		0.0432 U	
Aroclor 1221	11104-28-2	mg/kg	NE	NE	NE	0.0448 U		0.0428 U		0.0385 U		0.0376 U		0.0432 U	
Aroclor 1232	11141-16-5	mg/kg	NE	NE	NE	0.0448 U		0.0428 U		0.0385 U		0.0376 U		0.0432 U	
Aroclor 1242	53469-21-9	mg/kg	NE	NE	NE	0.0448 U		0.0428 U		0.0385 U		0.0376 U		0.0432 U	
Aroclor 1248	12672-29-6	mg/kg	NE	NE	NE	0.0448 U		0.0428 U		0.0385 U		0.0376 U		0.0432 U	
Aroclor 1254	11097-69-1	mg/kg	NE	NE	NE	0.0448 U		0.0428 U		0.0385 U		0.0376 U		0.0432 U	
Aroclor 1260	11096-82-5	mg/kg	NE	NE	NE	0.0448 U		0.0428 U		0.0385 U		0.0376 U		0.0432 U	
Aroclor 1262	37324-23-5	mg/kg	NE	NE	NE	0.0448 U		0.0428 U		0.0385 U		0.0376 U		0.0432 U	
Aroclor 1268	11100-14-4	mg/kg	NE	NE	NE	0.0448 U		0.0428 U		0.0385 U		0.0376 U		0.0432 U	
PCBs, Total	1336-36-3	mg/kg	NE	NE	1	0.0448 U		0.0428 U		0.0385 U		0.0376 U		0.0432 U	
Petroleum Hydrocarbon Quantitation															
TPH	NONE	mg/kg	2500	5000	10000	121		74.1		39 U		75.4		44.2 U	
Semivolatile Organics by GC/MS															
1,2,4-Trichlorobenzene	120-82-1	mg/kg	NE	NE	19	0.22 U		0.22 U		0.2 U		0.19 U		0.23 U	
1,2-Dichlorobenzene	95-50-1	mg/kg	NE	NE	88	0.22 U		0.22 U		0.2 U		0.19 U		0.23 U	
1,3-Dichlorobenzene	541-73-1	mg/kg	NE	NE	150	0.22 U		0.22 U		0.2 U		0.19 U		0.23 U	
1,4-Dichlorobenzene	106-46-7	mg/kg	NE	NE	7	0.22 U		0.22 U		0.2 U		0.19 U		0.23 U	
1-Methylnaphthalene	90-12-0	mg/kg	NE	NE	NE	0.22 U		0.22 U		0.2 U		0.19 U		0.23 U	
2,4,5-Trichlorophenol	95-95-4	mg/kg	NE	NE	24	0.22 U		0.22 U		0.2 U		0.19 U		0.23 U	
2,4,6-Trichlorophenol	88-06-2	mg/kg	NE	NE	0.7	0.13 U		0.13 U		0.12 U		0.11 U		0.14 U	
2,4-Dichlorophenol	120-83-2	mg/kg	NE	NE	0.7	0.2 U		0.2 U		0.18 U		0.17 U		0.2 U	
2,4-Dimethylphenol	105-67-9	mg/kg	NE	NE	4	0.22 U		0.22 U		0.2 U		0.19 U		0.23 U	
2,4-Dinitrophenol	51-28-5	mg/kg	NE	NE	0.7	1.1 U		1 U		0.97 U		0.9 U		1.1 U	
2,4-Dinitrotoluene	121-14-2	mg/kg	NE	NE	0.7	0.22 U		0.22 U		0.2 U		0.19 U		0.23 U	
2,6-Dinitrotoluene	606-20-2	mg/kg	NE	NE	NE	0.22 U		0.22 U		0.2 U		0.19 U		0.23 U	
2-Chloronaphthalene	91-58-7	mg/kg	NE	NE	NE	0.22 U		0.22 U		0.2 U		0.19 U		0.23 U	
2-Chlorophenol	95-57-8	mg/kg	NE	NE	2	0.22 U		0.22 U		0.2 U		0.19 U		0.23 U	
2-Methylnaphthalene	91-57-6	mg/kg	NE	NE	96	0.27 U		0.26 U		0.24 U		0.22 U		0.27 U	
2-Methylphenol	95-48-7	mg/kg	NE	NE	0.9	0.22 U		0.22 U		0.2 U		0.19 U		0.23 U	
2-Nitroaniline	88-74-4	mg/kg	NE	NE	NE	0.22 U		0.22 U		0.2 U		0.19 U		0.23 U	
2-Nitrophenol	88-75-5	mg/kg	NE	NE	NE	0.48 U		0.48 U		0.44 U		0.4 U		0.49 U	
3,3'-Dichlorobenzidine	91-94-1	mg/kg	NE	NE	0.7	0.22 U		0.22 U		0.2 U		0.19 U		0.23 U	
3-Methylphenol/4-Methylphenol	108-39-4	mg/kg	NE	NE	NE	0.32 U		0.32 U		0.29 U		0.27 U		0.33 U	
3-Nitroaniline	99-09-2	mg/kg	NE	NE	NE	0.22 U		0.22 U		0.2 U		0.19 U		0.23 U	
4,6-Dinitro-o-cresol	534-52-1	mg/kg	NE	NE	NE	0.58 U		0.57 U		0.53 U		0.48 U		0.59 U	
4-Bromophenyl phenyl ether	101-55-3	mg/kg	NE	NE	NE	0.22 U		0.22 U		0.2 U		0.19 U		0.23 U	
4-Chloroaniline	108-47-8	mg/kg	NE	NE	1.3	0.22 U		0.22 U		0.2 U		0.19 U		0.23 U	
4-Chlorophenyl phenyl ether	7005-72-3	mg/kg	NE	NE	NE	0.22 U		0.22 U		0.2 U		0.19 U		0.23 U	
4-Nitroaniline	100-01-6	mg/kg	NE	NE	NE	0.22 U		0.22 U		0.2 U		0.19 U		0.23 U	
4-Nitrophenol	100-02-7	mg/kg	NE	NE	NE	0.31 U		0.31 U		0.28 U		0.26 U		0.32 U	
Acenaphthene	83-32-9	mg/kg	NE	NE	340	0.18 U		0.18 U		0.16 U		0.15 U		0.18 U	
Acenaphthylene	208-96-8	mg/kg	NE	NE	490	0.36		0.18 U		0.16 U		0.22 U		0.18 U	
Aniline	62-53-3	mg/kg	NE	NE	NE	0.27 U		0.26 U		0.24 U		0.22 U		0.27 U	
Anthracene	120-12-7	mg/kg	NE	NE	1000	0.23		0.17		0.12 U		0.11 U		0.14 U	
Azobenzene	103-33-3	mg/kg	NE	NE	NE	0.22 U		0.22 U		0.2 U		0.19 U		0.23 U	
Benzidine	92-87-5	mg/kg	NE	NE	0.01	0.74 U		0.73 U		0.67 U		0.62 U		0.76 U	
Benzo(a)anthracene	56-55-3	mg/kg	NE	NE	1	0.76		0.72		0.62 U		0.25		0.14 U	
Benzo(a)pyrene	50-32-8	mg/kg	NE	NE	NE	0.92		1.5		0.16 U		0.38		0.18 U	
Benzo(b)fluoranthene	205-99-2	mg/kg	NE	NE	1	0.12		0.12		0.12 U		0.46		0.14 U	
Benzo(g,h,i)perylene	191-24-2	mg/kg	NE	NE	NE	0.62		1.8		0.16 U		0.28		0.18 U	
Benzo(k)fluoranthene	207-08-9	mg/kg	NE	NE	12	0.48		0.31		0.12 U		0.15		0.14 U	
Benzoic Acid	65-85-0	mg/kg	NE	NE	350	0.73 U		0.71 U		0.66 U		0.6 U		0.74 U	
Benzyl Alcohol	100-51-6	mg/kg	NE	NE	NE	0.22 U		0.22 U		0.2 U		0.19 U		0.23 U	
Biphenyl	92-52-4	mg/kg	NE	NE	125	0.51 U		0.5 U		0.46 U		0.43 U		0.52 U	
Bis(2-chloroethoxy)methane	111-91-1	mg/kg	NE	NE	NE	0.24 U		0.24 U		0.22 U		0.2 U		0.25 U	
Bis(2-chloroethyl)ether	111-44-4	mg/kg	NE	NE	0.7	0.2 U		0.2 U		0.18 U		0.17 U		0.2 U	
Bis(2-chloroisopropyl)ether	108-60-1	mg/kg	NE	NE	5	0.27 U		0.26 U		0.24 U		0.22 U		0.27 U	
Bis(2-ethylhexyl)phthalate	117-81-7	mg/kg	NE	NE	NE	0.22 U		0.22 U		0.2 U		0.19 U		0.23 U	
Butyl benzyl phthalate	85-68-7	mg/kg	NE	NE	NE	0.22 U		0.22 U		0.2 U		0.19 U		0.23 U	
Carbazole	86-74-8	mg/kg	NE	NE	NE	0.22 U		0.22 U		0.2 U		0.19 U		0.23 U	
Chrysene	218-01-9	mg/kg	NE	NE	120	1		0.83		0.12 U		0.26		0.14 U	
Di-n-butylphthalate	84-74-2	mg/kg	NE	NE	2600	0.22 U		0.22 U		0.2 U		0.19 U		0.23 U	
Di-n-octylphthalate	117-84-0	mg/kg	NE	NE	NE	0.22 U		0.22 U		0.2 U		0.19 U		0.23 U	
Dibenzo(a,h)anthracene	53-70-3	mg/kg	NE	NE	0.7	0.16		0.28		0.12 U		0.11 U		0.14 U	
Dibenzofuran	132-64-9	mg/kg	NE	NE	NE	0.22 U		0.22 U		0.2 U		0.19 U		0.23 U	
Dibutyl phthalate	84-66-2	mg/kg	NE	NE	1000	0.22 U		0.22 U		0.2 U		0.19 U		0.23 U	
Dimethyl phthalate	131-11-3	mg/kg	NE	NE	702	0.22 U		0.22 U		0.2 U		0.19 U		0.23 U	
Fluoranthene	206-44-0	mg/kg	NE	NE	960	1.1		1.1		0.12 U		0.35		0.14 U	
Fluorene	86-73-7	mg/kg	NE	NE	77	0.22 U		0.22 U		0.2 U		0.19 U		0.23 U	
Hexachlorobenzene	118-74-1	mg/kg	NE	NE	0.8	0.13 U		0.13 U		0.12 U		0.11 U		0.14 U	
Hexachlorobutadiene	87-68-3	mg/kg	NE	NE	17	0.22 U		0.22 U		0.2 U		0.19 U		0.23 U	
Hexachlorocyclopentadiene	77-47-4	mg/kg	NE	NE	200	0.64 U		0.63 U		0.58 U		0.53 U		0.65 U	
Hexachloroethane	67-72-1	mg/kg	NE	NE	0.7	0.18 U		0.18 U		0.16 U		0.15 U		0.18 U	
Indeno(1,2,3-cd)pyrene	193-39-5	mg/kg	NE	NE	1	0.66		1.2		0.16 U		0.27		0.18 U	
Isophorone	78-59-1	mg/kg	NE	NE	1	0.2 U		0.2 U		0.18 U		0.17 U		0.2 U	
n-Nitrosod-n-propylamine	621-64-7	mg/kg	NE	NE	NE	0.22 U		0.22 U		0.2 U		0.19 U		0.23 U	
n-Nitrosodimethylamine	62-75-9	mg/kg	NE	NE	NE	0.45 U		0.44 U		0.4 U		0.37 U		0.46 U	
Naphthalene	91-20-3	mg/kg	NE	NE	5	0.22 U		0.22 U		0.2 U		0.19 U		0.23 U	
NDPA/DPA	86-30-6	mg/kg	NE	NE	NE	0.18 U		0.18 U		0.16 U		0.15 U		0.18 U	
Nitrobenzene	98-95-3	mg/kg	NE	NE	NE	0.2 U		0.2 U		0.18 U		0.17 U		0.2 U	
p-Chloro-m-cresol	59-50-7	mg/kg	NE	NE	NE	0.22 U		0.22 U		0.2 U		0.19 U		0.23 U	
Pentachlorophenol	87-86-5	mg/kg	NE	NE	1	0.18 U		0.18 U		0.16 U		0.15 U		0.18 U	
Phenanthrene	85-01-8	mg/kg	NE	NE	3	0.91		0.6		0.12 U		0.11 U		0.14 U	
Phenol	108-95-2	mg/kg	NE	NE	56	0.22 U		0.22 U		0.2 U		0.19 U		0.23 U	
Pyrene	129-00-0	mg/kg	NE	NE	720	1.4		1.4		0.12 U		0.35		0.14 U	
Pyridine	110-86-1	mg/kg	NE	NE	NE	0.24 U		0.24 U		0.22 U		0.2 U		0.25 U	
Total VOCs	-	mg/kg	100	100	NE	10.5		11.11		ND		2.95		ND	
Volatile Organics by GC/MS-5035															
1,1,1,2-Tetrachloroethane	630-20-6	mg/kg	NE	NE	0.8	0.04 U		0.047 U		0.028 U		0.025 U		0.029 U	
1,1,1-Trichloroethane	71-55-6	mg/kg	NE	NE	78	0.04 U		0.047 U		0.028 U		0.025 U		0.029 U	
1,1,2,2-Tetrachloroethane	79-34-5														

STOCKPILE SOIL SAMPLING RESULTS

TOWN OF SALEM, NEW HAMPSHIRE

SOUTH BROADWAY WATER & SEWER REHABILITATION PROGRAM

LOCATION		ANGELAS STOCKPILE, COMPOSITE									
SAMPLING DATE		9/13/2017									
LAB SAMPLE ID		L1732389-01									
		COMM-97									
		Unlined Landfill	Lined Landfill	NH-S1	NH-S2	NH-S3	NH-SRS	Units	Results	Qual	
Extractable Petroleum Hydrocarbons											
	C9-C18 Aliphatics	C9-C18-ALPHA-UJ						mg/kg	6.58	U	
	C19-C36 Aliphatics	C19-C36-ALPHA-UJ						mg/kg	19.8		
	C11-C22 Aromatics	C11-C22-ALPHA-UJ						mg/kg	30.3		
	C11-C22 Aromatics, Adjusted	C11-C22-ALPHA-J						mg/kg	21.9		
General Chemistry											
	Solids, Total	NONE						%	98.6		
PCB by GC											
	Aroclor 1016	12674-11-2			1	10	25	1	mg/kg	0.0333	U
	Aroclor 1221	11104-28-2			1	10	25	1	mg/kg	0.0333	U
	Aroclor 1232	11141-16-5			1	10	25	1	mg/kg	0.0333	U
	Aroclor 1242	53469-21-9			1	10	25	1	mg/kg	0.0333	U
	Aroclor 1248	12672-29-6			1	10	25	1	mg/kg	0.0333	U
	Aroclor 1250	11097-69-1			1	10	25	1	mg/kg	0.0333	U
	Aroclor 1260	11096-82-5			1	10	25	1	mg/kg	0.0333	U
	Aroclor 1262	37324-23-5			1	10	25	1	mg/kg	0.0333	U
	Aroclor 1268	11100-14-4			1	10	25	1	mg/kg	0.0333	U
	PCBs, Total	1336-36-3			1	10	25	1	mg/kg	0.0333	U
Petroleum Hydrocarbon Quantitation											
	TPH	NONE	2500	5000	10000	10000	10000	10000	mg/kg	64.2	
Semivolatile Organics by GC/MS											
	Acenaphthene	83-32-9			340	340	340	340	mg/kg	0.13	U
	Benzidine	92-87-5			0.01	0.01	0.01	0.01	mg/kg	0.55	U
	1,2,4-Trichlorobenzene	120-82-1			19	19	19	19	mg/kg	0.17	U
	Hexachlorobenzene	118-74-1			0.8	3	27	0.8	mg/kg	0.1	U
	Bis(2-chloroethyl)ether	111-44-4			0.7	0.7	0.7	0.7	mg/kg	0.15	U
	2-Chloronaphthalene	91-58-7							mg/kg	0.17	U
	1,2-Dichlorobenzene	95-50-1			88	88	88	88	mg/kg	0.17	U
	1,3-Dichlorobenzene	541-73-1			150	150	150	150	mg/kg	0.17	U
	1,4-Dichlorobenzene	106-46-7			7	7	7	7	mg/kg	0.17	U
	3,3'-Dichlorobenzidine	91-94-1			0.7	0.7	0.7	0.7	mg/kg	0.17	U
	2,4-Dinitrotoluene	121-14-2			0.7	0.7	0.7	0.7	mg/kg	0.17	U
	2,6-Dinitrotoluene	606-20-2							mg/kg	0.17	U
	Azobenzene	103-33-3			1	4	4	1	mg/kg	0.17	U
	Fluoranthene	206-44-0			960	2500	5000	960	mg/kg	0.81	
	4-Chlorophenyl phenyl ether	7005-72-3							mg/kg	0.17	U
	4-Bromophenyl phenyl ether	101-55-3							mg/kg	0.17	U
	Bis(2-chloroisopropyl)ether	108-60-1			5	5	5	5	mg/kg	0.2	U
	Bis(2-chloroethoxy)methane	111-91-1							mg/kg	0.18	U
	Hexachlorobutadiene	87-68-3			17	53	120	17	mg/kg	0.17	U
	Hexachlorocyclopentadiene	77-47-4			200	1800	1800	200	mg/kg	0.48	U
	Hexachloroethane	67-72-1			0.7	0.7	0.7	0.7	mg/kg	0.13	U
	Isophorone	78-59-1			1	1	1	1	mg/kg	0.15	U
	Naphthalene	91-20-3			5	5	5	5	mg/kg	0.17	U
	Nitrobenzene	98-95-3							mg/kg	0.15	U
	NDPA/DPA	86-30-6							mg/kg	0.13	U
	n-Nitrosodi-n-propylamine	621-64-7							mg/kg	0.17	U
	Bis(2-ethylhexyl)phthalate	117-81-7			72	220	2300	72	mg/kg	0.17	U
	Butyl benzyl phthalate	85-68-7							mg/kg	0.17	U
	Di-n-butylphthalate	84-74-2							mg/kg	0.17	U
	Di-n-octylphthalate	117-84-0							mg/kg	0.17	U
	Diethyl phthalate	84-66-2			1000	2500	5000	1000	mg/kg	0.17	U
	Dimethyl phthalate	131-11-3			700	700	700	700	mg/kg	0.17	U
	Benzo(a)anthracene	56-55-3			1	4	52	1	mg/kg	0.91	
	Benzo(a)pyrene	50-32-8			0.7	0.7	5	0.7	mg/kg	1.6	
	Benzo(b)fluoranthene	205-99-2			1	4	52	1	mg/kg	2.3	
	Benzo(k)fluoranthene	207-08-9			12	36	520	12	mg/kg	0.55	
	Chrysene	218-01-9			120	360	5200	120	mg/kg	1.1	
	Acenaphthylene	208-96-8			490	490	490	490	mg/kg	0.13	U
	Anthracene	120-12-7			1000	2500	5000	1000	mg/kg	0.1	U
	Benzo(ghi)perylene	191-24-2							mg/kg	1.3	
	Fluorene	86-73-7			77	77	77	77	mg/kg	0.17	U
	Phenanthrene	85-01-8							mg/kg	0.21	
	Dibenz(a,h)anthracene	53-70-3			0.7	0.7	5	0.7	mg/kg	0.29	
	Indeno(1,2,3-cd)pyrene	193-39-5			1	4	52	1	mg/kg	1.2	
	Pyrene	129-00-0			720	2500	5000	720	mg/kg	0.86	
	Biphenyl	92-52-4			125	174	174	125	mg/kg	0.38	U
	Aniline	62-53-3							mg/kg	0.2	U
	4-Chloroaniline	106-47-8			1.3	1.3	1.3	1.3	mg/kg	0.17	U
	1-Methylnaphthalene	90-12-0							mg/kg	0.17	U
	2-Nitroaniline	88-74-4							mg/kg	0.17	U
	3-Nitroaniline	99-09-2							mg/kg	0.17	U
	4-Nitroaniline	100-01-6							mg/kg	0.17	U
	Dibenzofuran	132-64-9							mg/kg	0.17	U
	2-Methylnaphthalene	91-57-6			96	100	100	96	mg/kg	0.2	U
	n-Nitrosodimethylamine	62-75-9							mg/kg	0.33	U
	2,4,6-Trichlorophenol	88-06-2			0.7	0.7	0.7	0.7	mg/kg	0.1	U
	p-Chloro-m-cresol	59-50-7							mg/kg	0.17	U
	2-Chlorophenol	95-57-8			2	2	2	2	mg/kg	0.17	U
	2,4-Dichlorophenol	120-83-2			0.7	0.7	0.7	0.7	mg/kg	0.15	U
	2,4-Dimethylphenol	105-67-9			4	4	4	4	mg/kg	0.17	U
	2-Nitrophenol	88-75-5							mg/kg	0.36	U
	4-Nitrophenol	100-02-7							mg/kg	0.23	U
	2,4-Dinitrophenol	51-28-5			0.7	0.7	0.7	0.7	mg/kg	0.8	U
	4,6-Dinitro-m-cresol	534-52-1							mg/kg	0.43	U
	Pentachlorophenol	87-86-5			3	3	3	3	mg/kg	0.13	U
	Phenol	108-95-2			56	56	56	56	mg/kg	0.17	U
	2-Methylphenol	95-48-7			0.9	0.9	0.9	0.9	mg/kg	0.17	U
	3-Methylphenol/4-Methylphenol	108-39-4			0.7	0.7	0.7	0.7	mg/kg	0.24	U
	2,4,5-Trichlorophenol	95-95-4			24	24	24	24	mg/kg	0.17	U
	Benzoic Acid	65-85-0			350	350	350	350	mg/kg	0.54	U
	Benzyl Alcohol	100-51-6							mg/kg	0.17	U
	Carbazole	86-74-8							mg/kg	0.17	U
	Pyridine	110-86-1							mg/kg	0.18	U
	Total VOCs		100	100			NE		mg/kg	11.13	
Total Metals											
	Antimony, Total	7440-36-0			9	74	74	9	mg/kg	1.97	U
	Arsenic, Total	7440-38-2	40	40	11	11	47	11	mg/kg	3.4	
	Barium, Total	7440-39-3			1000	2500	5000	1000	mg/kg	53.5	
	Beryllium, Total	7440-41-7			12	89	100	12	mg/kg	0.197	U
	Cadmium, Total	7440-43-9			33	280	280	33	mg/kg	1.08	
	Chromium, Total	7440-47-3	1000	1000				NE	mg/kg	67.8	
	Lead, Total	7439-92-1	1000	2000	400	400	400	400	mg/kg	37.4	
	Mercury, Total	7439-97-6	10	10	7	52	52	7	mg/kg	0.07	U
	Nickel, Total	7440-02-0			400	2500	3100	400	mg/kg	63.5	
	Selenium, Total	7782-49-2			180	1600	1600	180	mg/kg	0.789	U
	Silver, Total	7440-22-4			89	690	690	89	mg/kg	0.395	U
	Thallium, Total	7440-28-0			10	10	10	10	mg/kg	0.789	U
	Vanadium, Total	7440-62-2							mg/kg	14.2	
	Zinc, Total	7440-66-6			1000	2500	5000	1000	mg/kg	35.5	
Volatile Organics by GC/MS-5035											
	Methylene chloride	75-09-2			0.1	0.1	0.1	0.1	mg/kg	0.0074	U
	1,1-Dichloroethane	75-34-3			3	3	3	3	mg/kg	0.0011	U
	Chloroform	67-66-3			3	3	3	3	mg/kg	0.0011	U
	Carbon tetrachloride	56-23-5			12	12	12	12	mg/kg	0.00074	U
	1,2-Dichloropropane	78-87-5			0.1	0.1	0.1	0.1	mg/kg	0.0026	U
	Dibromochloromethane	124-48-1			1	1	1	1	mg/kg	0.00074	U
	1,1,2-Trichloroethane	79-00-5			0.1	0.1	0.1	0.1	mg/kg	0.0011	U
	Tetrachloroethene	127-18-4			2	2	2	2	mg/kg	0.00074	U
	Chlorobenzene	108-90-7			6	6	6	6	mg/kg	0.00074	U
	Trichlorofluoromethane	75-69-4			1000	2500	5000	1000	mg/kg	0.0037	U
	1,2-Dichloroethane	107-06-2			0.1	0.1	0.1	0.1	mg/kg	0.00074	U
	1,1,1-Trichloroethane	71-55-6			78	78	78	78	mg/kg	0.00074	U
	Bromodichloromethane	75-27-4			0.1	0.1	0.1	0.1	mg/kg	0.00074	U
	trans-1,3-Dichloropropene	10061-02-6			0.1	0.1	0.1	0.1	mg/kg	0.00074	U
	cis-1,3-Dichloropropene	10061-01-5			0.1	0.1	0.1	0.1	mg/kg	0.00074	U
	1,3-Dichloropropene, Total	542-75-6			0.1	0.1	0.1	0.1	mg/kg	0.00074	U
	1,1-Dichloropropene	563-58-6							mg/kg	0.0037	U
	Bromoform	75-25-2			0.1	0.1	0.1	0.1	mg/kg	0.003	U
	1,1,2,2-Tetrachloroethane	79-34-5			4	4	4	4	mg/kg	0.00074	U

**TOWN OF SALEM, NEW HAMPSHIRE**  
**SOUTH BROADWAY WATER & SEWER REHABILITATION PROGRAM**

Notes:  
U = Not detected  
ug/L = micrograms per liter

\*NH-AGQS: NH Ambient GW Quality Standards Criteria per NH Ambient Groundwater Quality Standards, Effective June 1, 2015.

\*NHGW-1: NH GW-1 Method 1 Groundwater Standards Criteria per NH Risk Characterization & Management Policy, Revised February 2013.

\*NHGW-2: NH GW-2 Method 1 Groundwater Standards Criteria per NH Risk Characterization & Management Policy, Revised February 2013.

\*MA GW-3: MassDEP MCP Method 1 Groundwater Standard, accessed online 7/17/2017

**GROUNDWATER QUALITY DATA -- PERFLUORINATED COMPOUNDS**

**TOWN OF SALEM, NEW HAMPSHIRE**

**SOUTH BROADWAY WATER & SEWER REHABILITATION PROGRAM**

LOCATION							B-2		B-3		B-4		B-11		B-15		FIELD BLANK		DUP-1		
SAMPLING DATE							6/29/2017		6/29/2017		6/29/2017		6/29/2017		6/29/2017		6/29/2017		6/29/2017		
LAB SAMPLE ID							L1722254-01		L1722254-02		L1722254-03		L1722254-04		L1722254-05		L1722254-06		L1722254-07		
		CasNum	NH-AGQS	NHGW-1	NHGW-2	MA GW-3	Units	Results	Qual	Results	Qual	Results	Qual	Results	Qual	Results	Qual	Results	Qual	Results	Qual
Perfluorinated Alkyl Acids by Isotope Dilution																					
	Perfluorobutanoic Acid (PFBA)	375-22-4					ug/l	0.00596		0.00899		0.00367		0.00185	U	0.00459		0.00192	U	0.002	U
	Perfluoropentanoic Acid (PFPeA)	2706-90-3					ug/l	0.00465		0.00392		0.00373		0.0023		0.00309		0.00192	U	0.0024	
	Perfluorobutanesulfonic Acid (PFBS)	375-73-5					ug/l	0.00353		0.00398		0.00348		0.00185	U	0.00185	U	0.00192	U	0.002	U
	Perfluorohexanoic Acid (PFHxA)	307-24-4					ug/l	0.00406		0.00503		0.00378		0.00185	U	0.00294		0.00192	U	0.002	U
	Perfluoroheptanoic Acid (PFHpA)	375-85-9					ug/l	0.00272		0.00379		0.00358		0.00185	U	0.0023		0.00192	U	0.002	U
	Perfluorohexanesulfonic Acid (PFHxS)	355-46-4					ug/l	0.00185	U	0.00509		0.00524		0.00208		0.00185	U	0.00192	U	0.002	U
	Perfluorooctanoic Acid (PFOA)	335-67-1	0.07				ug/l	0.00877		0.0115		0.0202		0.0022		0.00514		0.00192	U	0.00277	
	Perfluorononanoic Acid (PFNA)	375-95-1					ug/l	0.00185	U	0.00185	U	0.00185	U	0.00185	U	0.00185	U	0.00192	U	0.002	U
	Perfluorooctanesulfonic Acid (PFOS)	1763-23-1	0.07				ug/l	0.00567		0.00526		0.00732		0.00412		0.00185	U	0.00192	U	0.00236	

\*NH-AGQS: NH Ambient GW Quality Standards Criteria per NH Ambient Groundwater Quality Standards, Effective October 22, 2016.

\*NHGW-1: NH GW-1 Method 1 Groundwater Standards Criteria per NH Risk Characterization & Management Policy, Revised February 2013.

\*NHGW-2: NH GW-2 Method 1 Groundwater Standards Criteria per NH Risk Characterization & Management Policy, Revised February 2013.

\*MA GW-3: MassDEP MCP Method 1 Groundwater Standard, access online 10/3/2017.

**TOWN OF SALEM, NEW HAMPSHIRE**  
**SOUTH BROADWAY WATER MAIN REPLACEMENT**

**pH READINGS**

Location	6/8/2017	6/29/2017	7/24/2017
B-2	6.36	6.50	5.74
B-3	6.00	6.58	5.74
B-4	6.39	6.46	6.07
B-11	6.31	6.43	4.79
B-15	6.43	6.68	6.92

**Notes:**

The GLSD discharge standard is for the pH to be between 5.5 and 10.5. One of the readings from B-11 was outside that range. GLSD was willing to let the discharge proceed for the Post Office intersection work, on the basis of the average of the three readings being within the range.

**TOWN OF SALEM, NEW HAMPSHIRE  
SOUTH BROADWAY WATER MAIN REPLACEMENT**

**WATER LEVEL READINGS IN MONITORING WELLS**

(All readings in feet below road surface)

Well:	B-2	B-3	B-4	B-11	B-15
Development:	4/12/2017	4/11/2017	4/10/2017	4/13/2017	4/13/2017
4/13/2017	3.90	4.60	3.80	3.30	5.00
6/8/2017	5.68	4.53	4.08	4.00	3.21
6/29/2017	6.20	5.10	4.60	4.79	4.30
7/24/2017	6.09	5.14	4.62	5.07	4.61
10/4/2017	6.50	5.80	5.30	6.20	5.30

**Locations:**

B-2 is on the north side of East Main Street, near the Depot intersection.

B-3 is on the east side of S. Broadway, near the Depot intersection.

B-4 is on the east side of S. Broadway, downgradient of Daisy Cleaners.

B-11 is on the east side of S. Broadway, downgradient of Crown Cleaners site.

B-15 is on the east side of S. Broadway, downgradient of Nissan site.

**Notes:**

Water level may still have been recovering in some of the 4/13/2017 readings.



## **APPENDIX H**

### **SUMMARY OF MEETING WITH GLSD**



## Memorandum

*To: Dan Hudson, Town of Salem*

*From: Jeff Diercks and Steve Carey*

*Date: July 21, 2017*

*Subject: Minutes of Meeting with GLSD*

These notes summarize a meeting held on July 18, 2017, at the Greater Lawrence Sanitary District (GLSD) headquarters in North Andover, MA.

The three of us met with Cheri Cousens, Executive Director, and Colleen Spero, Monitoring Manager, to discuss the potential discharge to the sewer of water from dewatering operations during the South Broadway utilities reconstruction.

### Minutes

CDM Smith introduced the project, using a map to show the extent of the sewer and water work. We indicated most of that work would be done in 2018-19. We also focused attention on the Post Office Intersection area, explaining that road and water main construction in this area would be initiated very soon.

We provided a plan of the monitoring well locations, and a table of the laboratory analytical results to date. Jeff pointed out that there were no exceedances of either GLSD's Section 2.4 discharge regulations or of the Massachusetts DEP's GW-3 groundwater standards, with one exception. The exception was the lead level in monitoring well B-3, but that level met the GLSD standard, which supersedes the GW-3 standard. GLSD uses the GW-3 standards for guidance in cases where there is no GLSD standard.

CDM Smith and GLSD agreed that there were additional parameters that needed to be analyzed in samples from the monitoring wells to verify compliance with all GLSD standards. The parameters had been focused on NHDES discharge standards. The additional parameters are pH and four metals – copper, molybdenum, nickel and zinc. CDM Smith will make arrangements for those analyses and another round of water level readings.

Dan requested that the table of analytical results also include the GLSD Section 2.4 discharge standards. These have been added to the version of the table attached to this memorandum.

CDM Smith brought up the possibility of discharging the water into the sewer along with a similar discharge being planned for this summer and fall at the Town's former wastewater treatment facility. At that location, treatment facilities will be in place to remove volatile organic compounds

before discharge to the sewer. This would address the dry cleaning solvent (tetrachloroethylene) that will be encountered in B-11's water. CDM Smith asked if any treatment was needed, given that there were no exceedances. GLSD confirmed that no treatment was needed, and that the water could be discharged into any manhole, provided the upcoming sampling results are favorable.

GLSD asked if we knew what the flow rate from the trench would be. We indicated that we had not prepared an estimate, but that the work would be done during a low-groundwater season. We said that the June 29 reading from B-11, located in the Post Office Intersection area, was 4.8 feet below the road surface. This corresponds to the water table being only about 1.7 feet above the bottom of the trench. It likely will be even lower during construction, thereby limiting the amount of trench dewatering needed. GLSD noted that, whatever the actual flow may prove to be, it will be measured by the existing meter for payment purposes.

We asked GLSD what sampling would be required for the water being discharged to the sewer. After a discussion of options, GLSD determined the following:

- Monthly sampling will be required.
- Analyses should be performed for:
  - Total petroleum hydrocarbons (TPH).
  - The ten metals in GLSD's list of pollutant limitations (arsenic, cadmium, chromium, copper, lead, mercury, molybdenum, nickel, silver, zinc). Analyses for total metals, not dissolved metals, should be conducted.
  - Volatile organic chemicals (VOCs). Of particular interest are total BTEX, benzene, MTBE, and tetrachloroethylene. All of these parameters are addressed in the typical comprehensive VOC analysis.

GLSD requested advance notice from Salem regarding when the discharge will begin. This is for the purpose of allowing their pretreatment inspectors to visit the site of the discharge. This request applies to both the upcoming Post Office Intersection work and to the larger program in 2018-19.

With respect to the larger program in 2018-19, GLSD requested to be sent the relevant specification sections that address the discharge. They will review and comment on the spec before the project bidding begins. GLSD requested that the specifications require the contractor to have a frac tank on site in case unforeseen contamination is encountered.

We asked if there was a formal permit application form that we should fill out, for the upcoming Post Office Intersection work. GLSD indicated that it would be satisfactory for Salem to submit the remaining analytical results by email, and that GLSD would reply to that email. No further formal documentation is needed.

cc: Roy Sorenson/Town of Salem  
Cheri Cousens, Colleen Spero/GLSD

PROVISIONAL GROUNDWATER QUALITY DATA

DRAFT

TOWN OF SALEM, NEW HAMPSHIRE  
SOUTH BROADWAY WATER & SEWER REHABILITATION PROGRAM

LOCATION										B-2		B-3		B-4		B-11		B-15	
SAMPLING DATE										6/8/2017		6/8/2017		6/8/2017		6/8/2017		6/8/2017	
LAB SAMPLE ID										L1719160-01		L1719160-02		L1719160-03		L1719160-04		L1719160-05	
	CasNum	NH-AQS	NHGW-1	NHGW-2	MAGW-3	GLSD	Units	Results	Qual	Results	Qual	Results	Qual	Results	Qual	Results	Qual		
PCB by GC																			
Aroclor 1016	12674-11-2	0.5	0.5				ug/l	0.25	U	0.25	U	0.25	U	0.25	U	0.25	U		
Aroclor 1221	11104-28-2	0.5	0.5				ug/l	0.25	U	0.25	U	0.25	U	0.25	U	0.25	U		
Aroclor 1232	11141-16-5	0.5	0.5				ug/l	0.25	U	0.25	U	0.25	U	0.25	U	0.25	U		
Aroclor 1242	53469-21-9	0.5	0.5				ug/l	0.25	U	0.25	U	0.25	U	0.25	U	0.25	U		
Aroclor 1248	12672-29-6	0.5	0.5				ug/l	0.25	U	0.25	U	0.25	U	0.25	U	0.25	U		
Aroclor 1254	11097-69-1	0.5	0.5				ug/l	0.25	U	0.25	U	0.25	U	0.25	U	0.25	U		
Aroclor 1260	11096-82-5	0.5	0.5				ug/l	0.25	U	0.25	U	0.25	U	0.25	U	0.25	U		
Aroclor 1262	37324-23-5	0.5	0.5				ug/l	0.25	U	0.25	U	0.25	U	0.25	U	0.25	U		
Aroclor 1268	11100-14-4	0.5	0.5				ug/l	0.25	U	0.25	U	0.25	U	0.25	U	0.25	U		
PCBs, Total	1336-36-3	0.5	0.5		10		ug/l	0.25	U	0.25	U	0.25	U	0.25	U	0.25	U		
Misc.																			
pH	---					5.5-10.5	pH units												
Petroleum Hydrocarbon Quantitation																			
TPH	NONE				5000		ug/l	200	U	200	U	200	U	200	U	200	U		
Semivolatile Organics																			
Acenaphthene	83-32-9	420	420		10000		ug/l	0.1	U	0.1	U	0.1	U	0.1	U	0.1	U		
Benidine	92-87-5	0.8	0.8				ug/l	20	U	20	U	20	U	20	U	20	U		
1,2,4-Trichlorobenzene	120-82-1	70	70	150			ug/l	5	U	5	U	5	U	5	U	5	U		
Hexachlorobenzene	118-74-1	1	1		6000		ug/l	0.8	U	0.8	U	0.8	U	0.8	U	0.8	U		
Bis(2-chloroethyl)ether	111-44-4	10	10		50000		ug/l	2	U	2	U	2	U	2	U	2	U		
2-Chloronaphthalene	91-58-7						ug/l	0.2	U	0.2	U	0.2	U	0.2	U	0.2	U		
1,2-Dichlorobenzene	95-50-1	600	600	14000			ug/l	2	U	2	U	2	U	2	U	2	U		
1,3-Dichlorobenzene	541-73-1	600	600				ug/l	2	U	2	U	2	U	2	U	2	U		
1,4-Dichlorobenzene	106-46-7	75	75	80			ug/l	2	U	2	U	2	U	2	U	2	U		
3,3'-Dichlorobenzidine	91-94-1	1.3	1.3		2000		ug/l	5	U	5	U	5	U	5	U	5	U		
2,4-Dinitrotoluene	121-14-2	10	10		50000		ug/l	5	U	5	U	5	U	5	U	5	U		
2,6-Dinitrotoluene	606-20-2						ug/l	5	U	5	U	5	U	5	U	5	U		
Azobenzene	103-33-3	10	10				ug/l	2	U	2	U	2	U	2	U	2	U		
Fluoranthene	206-44-0	280	280		200		ug/l	0.1	U	0.1	U	0.1	U	0.1	U	0.1	U		
4-Chlorophenyl phenyl ether	7005-72-3						ug/l	2	U	2	U	2	U	2	U	2	U		
4-Bromophenyl phenyl ether	101-55-3						ug/l	2	U	2	U	2	U	2	U	2	U		
Bis(2-chloroisopropyl)ether	108-60-1	300	300		50000		ug/l	2	U	2	U	2	U	2	U	2	U		
Bis(2-chloroethoxy)methane	111-91-1						ug/l	5	U	5	U	5	U	5	U	5	U		
Hexachlorobutadiene	87-68-3	0.5	0.5		3000		ug/l	0.5	U	0.5	U	0.5	U	0.5	U	0.5	U		
Hexachlorocyclopentadiene	77-47-4	50	50				ug/l	20	U	20	U	20	U	20	U	20	U		
Hexachloroethane	67-72-1	1	1		50000		ug/l	0.8	U	0.8	U	0.8	U	0.8	U	0.8	U		
Isophorene	78-59-1	100	100				ug/l	5	U	5	U	5	U	5	U	5	U		
Naphthalene	91-20-3	20	20	1700	20000		ug/l	0.1	U	0.1	U	0.1	U	0.1	U	0.1	U		
Nitrobenzene	98-95-3						ug/l	2	U	2	U	2	U	2	U	2	U		
NDPA/DPA	86-30-6						ug/l	2	U	2	U	2	U	2	U	2	U		
n-Nitrosodi-n-propylamine	621-64-7						ug/l	5	U	5	U	5	U	5	U	5	U		
Bis(2-ethylhexyl)phthalate	117-81-7	6	6		50000		ug/l	3	U	3	U	3	U	3	U	3	U		
Butyl benzyl phthalate	85-68-7						ug/l	5	U	5	U	5	U	5	U	5	U		
Di-n-butylphthalate	84-74-2						ug/l	5	U	5	U	5	U	5	U	5	U		
Di-n-octylphthalate	117-84-0						ug/l	5	U	5	U	5	U	5	U	5	U		
Diethyl phthalate	84-66-2				9000		ug/l	5	U	5	U	5	U	5	U	5	U		
Dimethyl phthalate	131-11-3	50000	50000		50000		ug/l	5	U	5	U	5	U	5	U	5	U		
Benzo(a)anthracene	56-55-3	0.1	0.1		1000		ug/l	0.1	U	0.1	U	0.1	U	0.1	U	0.1	U		
Benzo(a)pyrene	50-32-8	0.2	0.2		500		ug/l	0.1	U	0.1	U	0.1	U	0.1	U	0.1	U		
Benzo(b)fluoranthene	205-99-2	0.1	0.1		400		ug/l	0.1	U	0.1	U	0.1	U	0.1	U	0.1	U		
Benzo(k)fluoranthene	207-08-9	0.5	0.5		100		ug/l	0.1	U	0.1	U	0.1	U	0.1	U	0.1	U		
Chrysene	218-01-9	5	5		70		ug/l	0.1	U	0.1	U	0.1	U	0.1	U	0.1	U		
Acenaphthylene	208-96-8	420	420		40		ug/l	0.1	U	0.1	U	0.1	U	0.1	U	0.1	U		
Anthracene	120-12-7	2100	2100		30		ug/l	0.1	U	0.1	U	0.1	U	0.1	U	0.1	U		
Benzo(g,h,i)perylene	191-24-2	210	210		20		ug/l	0.1	U	0.1	U	0.1	U	0.1	U	0.1	U		
Fluorene	86-73-7	280	280		40		ug/l	0.1	U	0.1	U	0.1	U	0.1	U	0.1	U		
Phenanthrene	85-01-8	210	210		10000		ug/l	0.1	U	0.1	U	0.1	U	0.1	U	0.1	U		
Dibenzo(a,h)anthracene	53-70-3	0.1	0.1		40		ug/l	0.1	U	0.1	U	0.1	U	0.1	U	0.1	U		
Indeno(1,2,3-cd)pyrene	193-39-5	0.1	0.1		100		ug/l	0.1	U	0.1	U	0.1	U	0.1	U	0.1	U		
Pyrene	129-00-0	210	210		20		ug/l	0.1	U	0.1	U	0.1	U	0.1	U	0.1	U		
Biphenyl	92-52-4	350	350		50000		ug/l	2	U	2	U	2	U	2	U	2	U		
Aniline	62-53-3						ug/l	2	U	2	U	2	U	2	U	2	U		
4-Chloroaniline	106-47-8	28	28		300		ug/l	5	U	5	U	5	U	5	U	5	U		
1-Methylnaphthalene	90-12-0	160					ug/l	0.1	U	0.1	U	0.1	U	0.1	U	0.1	U		
2-Nitroaniline	88-74-4						ug/l	5	U	5	U	5	U	5	U	5	U		
3-Nitroaniline	99-09-2						ug/l	5	U	5	U	5	U	5	U	5	U		
4-Nitroaniline	100-01-6						ug/l	5	U	5	U	5	U	5	U	5	U		
Dibenzofuran	132-64-9						ug/l	2	U	2	U	2	U	2	U	2	U		
2-Methylnaphthalene	91-57-6	280	280		20000		ug/l	0.1	U	0.1	U	0.1	U	0.1	U	0.1	U		
n-Nitrosodimethylamine	62-75-9						ug/l	2	U	2	U	2	U	2	U	2	U		
2,4,6-Trichlorophenol	88-06-2	5	5		500		ug/l	5	U	5	U	5	U	5	U	5	U		
p-Chloro-m-cresol	59-50-7						ug/l	2	U	2	U	2	U	2	U	2	U		
2-Chlorophenol	95-67-8	35	35		7000		ug/l	2	U	2	U	2	U	2	U	2	U		
2,4-Dichlorophenol	120-83-2	21	21		2000		ug/l	5	U	5	U	5	U	5	U	5	U		
2,4-Dimethylphenol	105-67-9	140	140		50000		ug/l	5	U	5	U	5	U	5	U	5	U		
2-Nitrophenol	88-75-5						ug/l	10	U	10	U	10	U	10	U	10	U		
4-Nitrophenol	100-02-7						ug/l	10	U	10	U	10	U	10	U	10	U		
2,4-Dinitrophenol	51-28-5	14	14		20000		ug/l	20	U	20	U	20	U	20	U	20	U		
4,6-Dinitro-o-cresol	534-52-1						ug/l	10	U	10	U	10	U	10	U	10	U		
Pentachlorophenol	87-86-5	1	1		200		ug/l	0.8	U	0.8	U	0.8	U	0.8	U	0.8	U		
Phenol	108-95-2	4000	4000		2000		ug/l	5	U	5	U	5	U	5					